### Stormwater Pollution Prevention Plan

### **Prepared for:**

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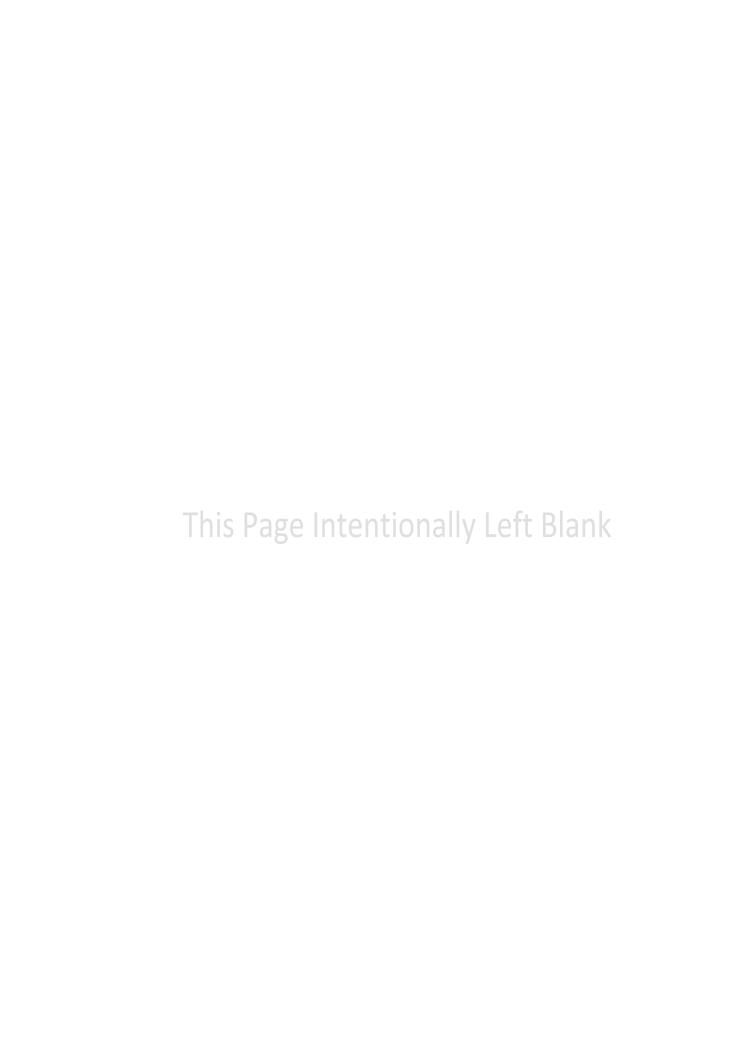
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Genesee 5 & Genesee 6 Solar Project Stormwater Pollution Prevention Plan

**Town of Stafford, Genesee County, New York** 

DATE: NOVEMBER 2023 LAST REVISED: JULY 2025 PROJECT NO. 2231877



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### 1.0 EXECUTIVE SUMMARY

This Stormwater Pollution Prevention Plan (SWPPP) has been prepared for major activities associated with construction of two solar projects on adjacent parcels with shared access in the Town of Stafford, Genesee County, NY. This SWPPP includes the elements necessary to comply with the national baseline general permit for construction activities enacted by the U.S. Environmental Protection Agency (EPA) under the National Pollutant Discharge Elimination System (NPDES) program and all local governing agency requirements. This SWPPP must be executed, and permit coverage must be obtained prior to the commencement of construction activity.

This SWPPP has been developed in accordance with the "New York State Department of Environmental Conservation (NYSDEC) State Pollutant Discharge Elimination System (SPDES) General Permit for Stormwater Discharges from Construction Activity," Permit No. GP-0-25-001, effective January 29, 2025 through January 28, 2030. The SWPPP and accompanying plans identify and detail stormwater management, pollution prevention, and erosion and sediment control measures necessary during and following completion of construction.

This SWPPP and the accompanying plans entitled "Genesee 5 Solar Array and Genesee 6 Solar Array" have been submitted as a set. These engineering drawings are considered an integral part of this SWPPP. Therefore, this SWPPP is not considered complete without them. References made herein to "the plans" or to a specific "sheet" refer to these drawings.

This report considers the impacts associated with the intended development with the purpose of:

- 1. Maintaining existing drainage patterns as much as possible while continuing the conveyance of upland watershed runoff;
- 2. Controlling increases in the rate of stormwater runoff resulting from the proposed development so as not to adversely alter downstream conditions; and
- 3. Mitigating potential stormwater quality impacts and preventing soil erosion and sedimentation resulting from stormwater runoff generated both during and after construction.

The analysis and design completed and documented in this report is intended to be part of the application made for an industrial development project completed on behalf of the Owner/Operator.

### 1.1 Project Description

NY CDG Genesee 5, LLC is proposing a development project on approximately 27 acres of land. The project is located on a 64.6-acre parcel at 8244 Batavia Stafford Townline Road, in the Town of Stafford, Genesee County, New York. Activities include the installation of a 5-MW ground mounted solar energy system, new electrical equipment, and a new gravel road. The total project area of disturbance proposed is 33 acres. The project will disturb one (1) or more acres and as such, preparation of this SWPPP is required under GP-0-25-001.

NY CDG Genesee 5, LLC is proposing a development project on approximately 27 acres of land. The project is located on a 127.5-acre parcel at 8244 Batavia Stafford Townline Road, in the Town of Stafford, Genesee County, New York. Activities include the installation of a 5-MW ground mounted solar energy system, new electrical equipment, and a new gravel road. The total project area of disturbance proposed is 32 acres. The project will disturb one (1) or more acres and as such, preparation of this SWPPP is required under GP-0-25-001.

A Site Location Map has been provided in Appendix A, as Figure A-1.

This type of project is included in Table 2 of Appendix B of GP-0-25-001; and the project site is not located in one of the watersheds listed in Appendix C of GP-0-25-001. Therefore, this SWPPP includes post-construction stormwater management practices, as well as erosion and sediment controls.

This project is not located within a regulated, traditional land use control Municipal Separate Stormwater Sewer System (MS4).

Runoff from the project site will discharge to the a tributary to Bigelow Creek, which is included in the list of Section 303(d) water bodies included in Appendix D of GP-0-25-001.

Project construction activities will consist primarily of site grading, and the installation of public utility infrastructure necessary to support the proposed development project. Construction phase pollutant sources anticipated at the site are disturbed (exposed) soil, vehicle fuels and lubricants, chemicals associated with building construction, and building materials. Without adequate control there is the potential for each type of pollutant to be transported by stormwater.

#### 1.2 Stormwater Pollution Controls

The stormwater pollution controls outlined herein have been designed and evaluated in accordance with the following standards and guidelines:

- New York State Stormwater Management Design Manual, dated July 31, 2024 (Design Manual).
- New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016 (SSESC).

Stormwater quality will be enhanced through the implementation of temporary and permanent erosion and sediment control measures, the proposed stormwater management practice(s), and other construction-phase pollution controls outlined herein.

The proposed stormwater management approach consisting of on-site stormwater management practices will adequately collect, treat, and convey the stormwater runoff.

Grass filter strips, two Bioretention Facilities, and one Dry Swale Facility will be used to manage and treat stormwater runoff generated by the proposed development project.

Pre- and post-development surface runoff rates have been evaluated for the 1-, 10-, and 100-year 24-hour storm events. Comparison of pre- and post-development watershed conditions demonstrates that the peak rate of runoff from the project site will not be increased.

The post-construction stormwater management practice(s) will be owned by the applicant. Policy and procedures will be in place, which ensure operation and maintenance of the practice(s) in accordance with the operation and maintenance plan.

### 2.0 SITE CHARACTERISTICS

### 2.1 State Environmental Quality Review

The construction activity is not subject to State Environmental Quality Review (SEQR), but is subject to the equivalent review from another New York State or federal agency. A copy of the documentation that project review, pursuant to a process equivalent to SEQR from another New York State or federal agency, in accordance with Part I.A.5. of GP-0-25-001, will be provided in Appendix A, as Figure A-5 upon receipt.

### 2.2 Land Use and Topography

The project site is located within the "Industrial Park" zoning district. Ground Mounted Solar Energy Systems are a permitted use, subject to a special use permit, within this district.

The overall site is moderately sloping, with slopes ranging from 0 to 25 percent. Site elevations range from approximately 780 feet above mean sea level (MSL) to 830 feet MSL.

The existing site is two parcels, a total of 192.1-acre former agricultural lot located at 8244 Batavia Stafford Townline Road in the Town of Stafford, Genesee County, NY.

The parcel is bound by Batavia-Stafford Townline Road to the west, Prole Road Extension to the north, and a railroad to the southeast. Agricultural land and sparse residential lots are adjacent to the parcel to both the northeast and south.

Two watersheds encompass the parcel and project site. One flows from the west to the northeast and the other flows from the west southeast. Both drain to an unnamed tributary to Bigelow Creek, which comprises a portion of the Genesee River watershed.

### 2.3 Soils and Groundwater

The US Department of Agriculture (USDA) Web Soil Survey (<a href="http://websoilsurvey.nrcs.usda.gov/app/">http://websoilsurvey.nrcs.usda.gov/app/</a>) was used to obtain surficial soil conditions for the study area, as follows:

Table 1: USDA Soil Data

Map Symbol & Description	Hydrologic Soil Group	Percent of Area
ApA - Appleton silt loam, 0 to 3 percent slopes	B/D	1.4
CeA - Cezenovia silt loam, 0 to 3 percent slopes	C/D	0.4
CeB - Cazenovia silt loam, 0 to 8 percent slopes	C/D	33.9
CeC - Cazenovia silt loam, 8 to 15 percent slopes	C/D	1.5
CgC3 - Cazenovia silty clay loam, 8 to 15 percent slopes, eroded	C/D	5.1
CgD3 - Cazenovia silty clay loam, 15 to 25 percent slopes, eroded	C/D	3.3
DuB - Dunkirk silt loam, 2 to 6 percent slopes	С	0.1
HIA - Hilton loam, 0 to 3 percent slopes	B/D	2.4

Map Symbol & Description	Hydrologic Soil Group	Percent of Area
LmA - Lima silt loam, 0 to 3 percent slopes	B/D	1.5
LmB - Lima silt loam, 3 to 8 percent slopes	B/D	1.0
OvA - Ovid silt loam, 0 to 3 percent slpoes	C/D	15.8
OvB - Ovid silt loam, 3 to 8 percent slopes	C/D	14.9
SeB - Schoharie silt loam, 1 to 6 percent slopes	D	8.9
ShC3 - Schoharie silty clay loam, 6 to 12 percent slopes	D	0.4
ShD3 - Schoharie silty clay loam, 12 to 20 percent slopes	D	2.0
Wy – Wayland soils complex, 0 to 3 percent slopes, frequently flooded	B/D	7.5

Upon review of the soil data presented in Table 1, the project site contains soils with a soil slope phase of D with a map unit name that inclusive of slopes greater than 25%, and does not contain soils with a soil slope phase of E or F.

The project site is composed of HSG B soils, HSG C soils, and HSG D soils, as shown in the table below. For the purposes of this report, HSG B/D and HSG C/D soils were modeled as HSG D soils to reflect the undrained condition.

Table 2: Project Site HSG Data

HSG A	HSG B	HSG C	HSG D
0%	0%	0.1%	99.9%

The Soil Conservation Service defines the hydrologic soil groups as follows:

- <u>Type C Soils</u>: Soils having a low infiltration rate when thoroughly wet and consisting chiefly of soils with a layer that impedes downward movement of water and soils with moderately fine-to-fine texture. These soils have a low rate of water transmission.
- <u>Type D Soils</u>: Soils having a very low infiltration rate and high runoff potential when thoroughly
  wet. These soils consist chiefly of clays that have high shrink-swell potential, soils that have a
  permanent high water table, soils that have a clay pan or clay layer at or near the surface, and
  soils that are shallow over nearly impervious material. These soils have a very low rate of water
  transmission.

The soils map for the study area is presented in Appendix A, as Figure A-2.

### 2.4 Watershed Designation

The project site is not located in a restricted watershed identified in Appendix C of GP-0-25-001.

### 2.5 Receiving Water Bodies

The nearest natural classified water course into which runoff from the project site will discharge is an unnamed tributary of Bigelow Creek. The unnamed tributary of Bigelow Creek on-site is classified by

NYSDEC as a Class C(T) stream. Bigelow Creek is classified by NYSDEC as a Class C stream, and is included in the Section 303(d) list of impaired waters found in Appendix D of GP-0-25-001.

### 2.6 Aquifer Designation

The project site is located over a Principal aquifer listed in the NYSDEC Technical and Operational Guidance Series (TOGS) 2.1.3 (1980).

### 2.7 Wetlands

The NYSDEC's Environmental Resource Mapper indicates no state-regulated wetlands or wetland buffers within the parcels that comprise the project area. The unnamed tributary of Bigelow Creek onsite is classified by NYSDEC as a Class C(T) stream. Refer to A-4 of Appendix A for the Environmental Resource map within the project area.

The U.S. Fish and Wildlife Service's National Wetlands Inventory indicates a federally-regulated stream running across the southern parcel from the southwest to the northeast. This mapping also shows a federally-regulated pond to the south and stream to the northeast of the project area. Refer to A-5 of Appendix A for the National Wetland Inventory map within the project area.

LaBella completed a Wetland and Stream Delineation, the results of which are documented in a set of Wetland Maps dated April 2023. One emergent wetland and one ephemeral ditch were identified on the northern parcel, both preliminarily identified as potentially non-jurisdictional. On the southern parcel, three streams, one pond, and one wetland were identified. One of these streams falls under NYSDEC and USACE jurisdiction along with the identified wetland. The other features are all identified as potentially non-jurisdictional. The proposed project layout avoids disturbing the jurisdictional features, save for a culvert in the jurisdictional stream to permit crossing.

### 2.8 Flood Plains

This project is not located in a FEMA floodzone.

#### 2.9 Listed, Endangered, or Threatened Species

A search was performed on the NYSDEC Environmental Resource Mapper on April, 2023, which determined that the project site has no known occurrences of threatened or endangered species, or critical habitat. An Environmental Resource Map has been provided in Appendix A, as Figure A-4.

### 2.10 Historic Places

SHPO has issued a No-Adverse-Effect letter on historic or archeological resources for both the Genesee 5 and Genesee 6 Solar Projects, letter date March 31, 2025.

### 2.11 Rainfall Data

Rainfall data utilized in the modeling and analysis was obtained from the Cornell University online Extreme Precipitation in New York & New England website (<a href="http://precip.eas.cornell.edu/">http://precip.eas.cornell.edu/</a>). The standard SCS/NRCS rainfall distributions were applied to evaluate the pre- and post-development

stormwater runoff characteristics. Rainfall data specific to the portion of Genesee County under consideration, for various 24-hour storm events, is presented in the following Table:

Table 3: Rainfall Data

Storm Event Return Period	24-Hour Rainfall (inches)
1-year	1.86
2-year	2.17
10-year	3.06
100-year	4.99

### 2.12 Pre-development Watershed Conditions

The pre-development project site contains two drainage areas that encompass the entire project site. The first drainage area flows from the west to the northeast and the other flows from the west southeast. Both drainage areas flow to an unnamed tributary to Bigelow Creek, which comprises a portion of the Genesee River watershed. Drainage area CM-E1 consists of the majority of the Genesee 5 project site. The site drains northeast to a roadside swale. This swale drains south to the unnamed tributary of Bigelow Creek. Drainage area CM-E2 comprises the remainder of the Genesee 5 project site and the Genesee 6 project set on the parcel to the south. Runoff in this drainage area flows to the unnamed tributary which flows through it.

The Pre-Development Watershed Delineation Map has been provided in Appendix A, as Figure A-5.

The results of the computer modeling used to analyze the overall watershed under pre-development conditions are presented in Appendix D. A summary of the pre-development watershed runoff rates at each Design Point is presented in Table 10.

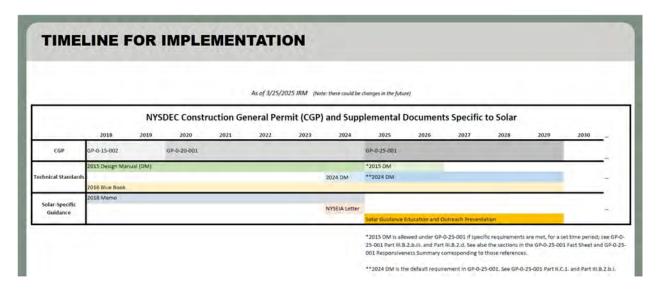
### 2.13 Post-development Watershed Conditions

The post-development project site has not been altered as compared to the pre-construction condition. The analysis of post-development conditions considered existing drainage patterns, soil types, ground cover to remain, planned site development, site grading, and stormwater management facilities proposed as part of site improvements. The Post-Development Watershed Delineation Map has been provided in Appendix A, as Figure A-6.

The results of the computer modeling used to analyze the overall watershed under post-development conditions are presented in Appendix D. A summary of the post-development watershed runoff rates at each Design Point is presented in Table 11.

The NYSDEC The Division of Water (DOW) issued "Solar Panel Construction Stormwater Permitting/SWPPP Guidance" on April 5, 2018. The 2018 Memo outlined two scenarios to comply with the SPDES General Permit for Stormwater Discharges from Construction Activity, GP-0-20-001, (CGP). However, the 2018 Memo did not adequately cover every solar installation scenario, and stormwater best practices and research relevant to the solar industry were evolving. Therefore, the Construction General Permit, GP-0-25-001 (CGP) was updated to replace the 2018 solar memo and provide clarification to achieve statewide consistency in the implementation of the CGP for solar energy construction sites.

Per the NYSDEC, please see the below timeline for implementation of solar sites per the Construction General Permit, GP-0-25-001 (CGP):



The NYSDEC offered the following guiding principles to be considered during the design of solar energy construction sites. Solar project sites are to be designed and constructed to promote sheet flow and infiltration underneath and between rows of solar panels. Compaction on solar sites is a critical factor to address; soil decompaction and restoration may be required after heavy equipment has been used to install solar panels. Solar project sites, like all other sites covered under the CGP, need to be stable, and discharges on solar project sites need to be non-erosive.

### 2.14 Description of Design Points

Design Point CM-E1 (92.8 acres) consists of the majority of the Genesee 5 project site. The site drains northeast to a roadside swale. Ultimately this swale drains south to the unnamed tributary of Bigelow Creek.

Design Point CM-E2 (118.9 acres) is comprised of the remainder of the Genesee 5 project site and the Genesee 6 project set on the parcel to the south. Runoff in this catchment drains to the unnamed tributary which flows through it.

#### 3.0 STORMWATER MANAGEMENT PLANNING

Chapter 3 of the Design Manual outlines a six-step planning process for site planning and selection of stormwater management practices that must be implemented for both new development and redevelopment projects. This process is intended to develop a design that maintains pre-construction hydrologic conditions through the application of environmentally sound development principles, as well as treatment and control of runoff discharges from the site. The following sections outline the step-by-step process and how it has been applied to this project.

The goals of this Stormwater Management Plan are to analyze the peak rate of runoff under pre- and post-development conditions, to maintain the pre-development rate of runoff in order to minimize impacts to adjacent or downstream properties, and to minimize the impact to the quality of runoff exiting the site.

The Design Manual provides both water quality and water quantity objectives to be met by projects requiring a "Full SWPPP". These objectives will be met by applying stormwater control practices to limit peak runoff rates and improve the quality of runoff leaving the developed site.

### 3.1 STEP 1 - Site Planning

During the Site Planning process, the project site is evaluated for implementation of the green infrastructure planning measures identified in Table 3.1 of the Design Manual, in order to preserve natural resources and reduce impervious cover. Appendix C provides a description of each green infrastructure planning measure, along with a project specific evaluation.

### 3.2 STEP 2 - Calculate Water Quality Treatment Volume (WQv)

Stormwater runoff from impervious surfaces is recognized as a significant contributor of pollution that can adversely affect the quality of receiving water bodies. Therefore, treatment of stormwater runoff is important since most runoff related water quality contaminants are transported from land, particularly the impervious surfaces, during the initial stages of storm events.

### 3.2.1 NYSDEC Requirements for Water Quality Volume

The Design Manual requires that water quality treatment be provided for the initial flush of runoff from every storm. The NYSDEC refers to the amount of runoff to be treated as the "Water Quality Volume" (WOv). Section 4.2 of the Design Manual defines the Water Quality Volume as follows:

$$WQv = \frac{[(P)(R_v)(A)]}{12}$$

Where: P = 90% Rainfall Event Number

 $R_v = 0.05 + 0.009 (I)$ 

I = Impervious Cover (Percent)
A = Contributing Area in Acres

This definition ensures that, all other things being equal, the Water Quality Volume will increase along with the impervious cover percentage.

### 3.2.2 Methodology for New Development

The Water Quality Volume equation has been applied to the drainage area tributary to each of the stormwater quality practices proposed for this project. The practices have been sized to accommodate the Water Quality Volume, as per the performance criteria presented in Chapter 5 and/or Chapter 6 of the Design Manual. Water quality volume calculations for each of the proposed practices are presented in Appendix C.

Table 4: Required WQv Summary

Required WQv		
5,545 cf	0.127 af	

# 3.3 STEP 3 – Apply RR Techniques and Standard SMPs with RRv Capacity to Reduce Total WQv

Land use change and development in the watershed increases the volume of runoff. As such, reductions in the amount of runoff from new development, accomplished through the implementation of a stormwater management plan for the site, will play an important role in the success or failure of the watershed-wide stormwater management plan. Runoff reduction techniques can be applied to manage, reduce, and treat stormwater, while maintaining and restoring natural hydrology through infiltration, evapo-transpiration, and the capture and reuse of stormwater. Volume reduction techniques by themselves typically are not sufficient to provide adequate attenuation of stormwater runoff, but they can decrease the size of the peak runoff rate reduction facilities.

### 3.3.1 NYSDEC Requirements for New Development

The Design Manual states that runoff reduction shall be achieved through infiltration, groundwater recharge, reuse, recycle, and/or evaporation/evapotranspiration of 100-percent of the post-development water quality volume to replicate pre-development hydrology. Runoff control techniques provide treatment in a distributed manner before runoff reaches the collection system, by maintaining pre-construction infiltration, peak runoff flow, discharge volume, as well as minimizing concentrated flow. This can be accomplished by applying a combination of Runoff Reduction Techniques, standard Stormwater Management Practices (SMPs) with RRv capacity, and good operation and maintenance.

### 3.3.2 Methodology

In order to reduce the required WQv and meet the RRv criteria, a site specific evaluation must be performed to determine the most practical means of reducing runoff volume by application of a combination of RR techniques and standard SMPs with RRv capacity.

### 3.3.3 Application of RR Techniques

The following Table demonstrates a summary of the RR techniques being applied for this project, and both the water quality and runoff reduction volumes they provide. The RR Techniques have been designed in accordance with Chapter 5 of the Design Manual. Refer to the contract drawings for practice dimensions, material specifications, and installation details. Practice specific calculations are presented in Appendix C.

Table 5: Summary of RR Techniques being Applied

RR Technique	NYSDEC Design Variant	RRv Capacity	WQv Required (CF)	WQv Reduced/RRv Provided (CF)
Runoff Reduct	duction)			
Sheet flow to Riparian Buffers/Filter Strips	RR-2	100%	3,634	3,634
RR Technique Totals			3,624	3,624

### 3.3.4 Application of Standard Stormwater Management Practices (SMPs) with RRv Capacity

The following Table demonstrates a summary of the standard SMP(s) with RRv capacity that have been incorporated into the stormwater management plan for this project. The standard SMP(s) with RRv capacity have been designed in accordance with Chapter 6 of the Design Manual. Refer to the contract drawings for practice dimensions, material specifications, and installation details. Practice specific calculations are presented in Appendix C.

Table 6: Summary of Standard SMPs with RRv Capacity being Applied

Standard SMP with RRv Capacity	Design Variant	Pretreatment Volume Required (% of WQv)	Pretreatment Volume Provided (CF)	RRv Capacity	WQv Required (CF)	WQv Reduced /RRv Provided (CF)	WQv Treated <sup>1</sup> (CF)	Total WQv Provided <sup>2</sup> (CF)
Filtration Bioretention	F-5	25	124	40%	980	980	0	980
Filtration Bioretention	F-5	25	91	40%	717	717	0	717
Dry Swale (HSG C & D)	0-1	10	166	20%	214	43	171	214
Standard SMP with RRv Capacity Totals				1,911	1,740	171	1,911	

#### Footnotes:

1WQv Treated = WQV Required - RRv Provided

2Total WOv Provided = WQV Treated + RRv Provided

### 3.3.5 RRv Performance Summary

A summary of the RRv provided is presented in the following table:

**Table 7: RRv Summary** 

WQv Required (CF)	RRv Provided	% RRv Provided/
(due to increase in impervious cover)	WQv Reduced (CF)	WQv Reduced
5,545	5,374	96.91%

As indicated in the above table, the RRv provided is not greater than or equal to the RRv required for the project site. A good faith effort has been made to reduce runoff to the greatest extent practical. However, the project site has shallow depth to bedrock, shallow depth to groundwater, soils with an infiltration rate less than 0.5 in/hr, which prevents reduction of the total WQv. As such, Appendix C provides a project specific evaluation for each RR technique and standard SMP with RRv capacity, demonstrating why these practices are infeasible.

### 3.4 STEP 4 - Calculate the Minimum RRv Required

Projects that cannot achieve 100% of the runoff reduction requirement due to site limitations, shall provide a minimum runoff reduction volume as calculated by the following equation:

$$\mathsf{RRV}_{\mathsf{min}} = \frac{\left[ (\mathsf{P}) (\mathsf{R}_{\mathsf{V}} *) (\mathsf{Aic}) (S) \right]}{12}$$

Where:

 $RRv_{min}$  = Runoff Reduction Volume (in acre-feet)

P = 90% Rainfall Event Number

Aic = Total area of new impervious cover (acres)  $Rv^*$  = 0.05+0.009(I), where I is 100% impervious

S = Hydrologic Soil Group (HSG) Specific

Reduction Factor where:

Based upon the soil survey data, the site consists of soils having a hydrologic soil type of D. As such, a specific reduction factor of 0.20 has been applied. Calculation of the required minimum RRv is presented in Appendix C.

**Table 8: Minimum RRv Summary** 

Minimum RRv Required (CF) (increase in impervious cover)	RRv Provided/ WQv Reduced (CF)	% of Minimum RRv Provided
1,002	5,545	100%

As indicated in the above table, the RRv provided is greater than the minimum RRv required for the project site. Therefore, the runoff reduction volume criteria has been met for the project and the design can proceed to Step 5.

### 3.5 STEP 5 - Apply Standard SMPs to Address Remaining Water Quality Volume

As previously discussed, 100% of the required WQv is being provided and reduced through RRv practices. As such, the water quality and runoff reduction volume criteria have been met and no other standard SMPs are required.

### 3.6 STEP 6 - Apply Volume and Peak Rate Control

This report presents the pre-development and post-development features and conditions associated with the rate of surface water runoff within the study area. For both cases, the drainage patterns, drainage structures, soil types, and ground cover types are considered in this study.

### 3.6.1 NYSDEC Requirements for New Development

Chapter 4 of the Design Manual requires that projects meet three separate stormwater quantity criteria:

- 1. The Channel Protection (CPv) requirement is designed to protect stream channels from erosion. This is accomplished by providing 24 hours of extended detention for the 1-year, 24-hour storm event. The Manual defines the CPv detention time as the center of mass detention time through each stormwater management practice.
- 2. The Overbank Flood Control (Qp) requirement is designed to prevent an increase in the frequency and magnitude of flow events that exceed the bank-full capacity of a channel, and

therefore must spill over into the floodplain. This is accomplished by providing detention storage to ensure that, at each Design Point, the post-development 10-year 24-hour peak discharge rate does not exceed the corresponding pre-development rate.

3. The Extreme Flood Control (Qf) requirement is designed to prevent the increased risk of flood damage from large storm events, to maintain the boundaries of the pre-development 100-year floodplain, and to protect the physical integrity of stormwater management practices. This is accomplished by providing detention storage to ensure that, at each Design Point, the post-development 100-year 24-hour peak discharge rate does not exceed the corresponding pre-development rate.

### 3.6.2 Methodology

In order to demonstrate that the NYSDEC detention requirements are being met, the Design Manual requires that a hydrologic and hydraulic analysis of the pre- and post-development conditions be performed using the Natural Resources Conservation Service Technical Release 20 (TR-20) and Technical Release 55 (TR-55) methodologies. HydroCAD, developed by HydroCAD Software Solutions LLC of Tamworth, New Hampshire, is a Computer-Aided-Design (CAD) program for analyzing the hydrologic and hydraulic characteristics of a given watershed and associated stormwater management facilities. HydroCAD uses the TR-20 algorithms and TR-55 methods to create and route runoff hydrographs.

HydroCAD has the capability of computing hydrographs (which represent discharge rates characteristic of specified watershed conditions, precipitation, and geologic factors) combining hydrographs and routing flows though pipes, streams and ponds. HydroCAD can also calculate the center of mass detention time for various hydraulic features. Documentation for HydroCAD can be found on their website: http://www.hydrocad.net/.

For this analysis, the watershed and drainage system was broken down into a network consisting of two types of components as described below:

- 1. Subcatchment: A relatively homogeneous area of land, which produces a volume and rate of runoff unique to that area.
- 2. Reach: Uniform streams, channels, or pipes that convey stormwater from one point to another.
- 3. Pond: Natural or man-made impoundment, which temporarily stores stormwater runoff and empties in a manner determined by its geometry and the hydraulic structure located at its outlets.
- 4. Link: A multi-purpose mechanism used to introduce a hydrograph from another file.

Subcatchments, reaches, and ponds and links are represented by hexagons, squares, and triangles, and broken boxes respectively, on the watershed routing diagrams provided with the computations included in Appendix D and Appendix E.

The analysis of hydrologic and hydraulic conditions and proposed stormwater management facilities, servicing the study area, was performed by dividing the tributary watershed into relatively homogeneous subcatchments. The separation of the watershed into subcatchments was dictated by watershed conditions, methods of collection, conveyance, and points of discharge. Watershed characteristics for each subcatchment were then assessed from United States Geological Service (USGS) 7.5-minute topographic maps, aerial photographs, a topographical survey, soil surveys, site investigations, and land use maps.

Proposed stormwater management practices were designed and evaluated in accordance with the Design Manual and local regulatory requirements. The hydrologic and hydraulic analysis considered the SCS Type II 24-hour storm events identified in the following Table.

Table 9: Design Events

Facility	24-hour Storm Event
Stormwater Management Practice(s)	1-year
	10-year
	100-year

### 3.6.3 Performance Summary

Per Section 4.6 of the Design Manual, the CPv requirement does not apply as the reduction of the entire CPv is achieved by application of runoff reduction techniques or infiltration systems.

A comparison of the pre- and post-development watershed conditions was performed for all Design Points and storm events evaluated herein. For all Design Points and design storms, this comparison demonstrates that the peak rate of runoff will not be increased. Therefore, the project will not have a significant adverse impact on the adjacent or downstream properties or receiving water courses.

The results of the computer modeling used to analyze the pre- and post-development watersheds are presented in Appendix  $\frac{D}{D}$  and Appendix  $\frac{E}{D}$ , respectively. The following Table summarizes the results of this analysis.

Table 10: Summary of Pre- and Post-Development Peak Discharge Rates

Pre- vs. Post-Development Discharge Rate (cfs)						
Design Point (DP)	10-year 24-hour storm event		100-year 24-hour storm event			
	Pre	Post	Pre	Post		
1	77.98	77.98	182.90	182.90		
2	158.40	158.40	374.10	374.10		
Total	200.72	200.72	478.90	478.90		

### 4.0 CONSTRUCTION SEQUENCE

This project has not received written approval from NYSDEC allowing the disturbance of more than five acres of land at any one time. Therefore, if the Contractor's construction sequence requires the disturbance of more than five acres at any one time, written approval must be obtained from written approval from NYSDEC prior to disturbing more than five acres at once.

- The SPDES permit identification number (Permit ID); and
- Full technical justification demonstrating why alternative methods of construction that would result in five acres of soil disturbance or less at any one time are not feasible; and
- The phasing plan for the project and sequencing plans for all phases from the SWPPP in accordance with Part III.B.1.d.; and
- Plans with locations and details of erosion and sediment control practices such that the heightened concern for erosion when disturbing greater than five acres at one time has been addressed; and
- Acknowledgement that the Owner/Operator will comply with the requirements in Part IV.C.2.b.;
   and
- Acknowledgement that the Owner/Operator will comply with the requirements in Part II.B.1.b.

The Owner/Operator must be in receipt of an Authorization Letter to Disturb Greater than Five Acres, which will include when the authorization begins and ends and indicate a maximum area (acres) of soil disturbance allowed at any one time from the NYSDEC. Should the request be denied, the contractor shall limit the area of disturbance to less than five acres of disturbance at any given time. The contractor shall prepare and submit to the Owner's/Operator's Engineer a sequencing plan that identifies the progression of construction through the site. This sequencing plan must be retained as part of the Site Log Book.

The "Erosion and Sediment Control Plan" and the "Erosion and Sediment Control Plan Prior to Construction" in the accompanying drawings and waiver request identifies the major construction activities that are the subject of this SWPPP. The order (or sequence) in which the major activities are expected to begin is presented on the accompanying drawings, though each activity will not necessarily be completed before the next begins. In addition, these activities could occur in a different order if necessary to maintain adequate erosion and sediment control. If this is the case, the contractor shall notify the Owner's/Operator's Engineer overseeing the implementation of the SWPPP.

The Contractor will be responsible for implementing the erosion and sediment control measures identified on the plans. The Contractor may designate these tasks to certain subcontractors as they see fit, but the ultimate responsibility for implementing these controls and ensuring their proper function remains with the Contractor.

In accordance with Part III.B.c.iv. a phasing plan for the project and sequencing plans for all phases have been provided. The plans address clearing and grubbing, excavation and grading, utility and infrastructure installation, final stabilization, and any other construction activity at the site that will result in soil disturbance. Refer to the plans for further information on the project phasing and sequencing.

### 5.0 CONSTRUCTION-PHASE POLLUTION CONTROL

The SWPPP and accompanying plans identify the temporary and permanent erosion and sediment control measures that have been incorporated into the design of this project. These measures will be implemented during construction, to minimize soil erosion and control sediment transport off-site, and after construction, to control the quality and quantity of stormwater runoff from the developed site.

Erosion control measures, designed to minimize soil loss, and sediment control measures, intended to retain eroded soil and prevent it from reaching water bodies or adjoining properties, have been developed in accordance with the following documents:

- NYSDEC SPDES General Permit for Stormwater Discharges From Construction Activity, Permit No. GP-0-25-001 (effective January 29, 2025 through January 28, 2030)
- New York State Standards and Specifications for Erosion and Sediment Control, NYSDEC (November 2016)
- Genesee County Stormwater and Erosion and Sediment Control Manual
- Town of Stafford Local Ordinance

The SWPPP and accompanying plans outline the construction scheduling for implementing the erosion and sediment control measures. These documents include limitations on the duration of soil exposure, criteria and specifications for placement and installation of the erosion and sediment control measures, a maintenance schedule, and specifications for the implementation of erosion and sediment control practices and procedures.

Temporary and permanent erosion and sediment control measures that shall be applied during construction generally include:

- 1. Minimizing soil erosion and sedimentation by stabilization of disturbed areas and by removing sediment from construction site discharges.
- 2. Preservation of existing vegetation to the greatest extent practical. Following the completion of construction activities in any portion of the site, permanent vegetation shall be established on all exposed soils.
- 3. Site preparation activities to minimize the area and duration of soil disruption.
- 4. Establishment of permanent traffic corridors to ensure that "routes of convenience" are avoided.

### 5.1 Temporary Erosion and Sediment Control Measures

The temporary erosion and sediment control measures described in the following sections are included as part of the construction documents.

### 5.1.1 Stabilized Construction Access

Prior to construction, stabilized construction access(es) will be installed, per accompanying plans, to reduce the tracking of sediment onto public roadways.

Construction traffic must enter and exit the site at the stabilized construction access(es). The intent is to trap dust and mud that would otherwise be carried off-site by construction traffic.

The access(es) shall be maintained in a condition, which will control tracking of sediment onto public rights-of-way or streets. When necessary, additional aggregate will be placed atop the filter fabric to assure the minimum thickness is maintained. All sediment and/or soil spilled, dropped, or washed onto public rights-of-way must be removed immediately. Periodic inspection and needed maintenance shall be provided after each substantial rainfall event.

### 5.1.2 Temporary Soil Stockpile

Materials, such as topsoil, will be temporarily stockpiled (if necessary) on the site during the construction process. Stockpiles shall be located in an area away from storm drainage, water bodies and/or courses, and will be properly protected from erosion by a surrounding silt fence barrier.

### 5.1.3 Silt Fencing

Prior to the initiation of and during construction activities, a geotextile filter fabric (or silt fence) will be established downgradient of all disturbed areas. These barriers may extend into non-impact areas to provide adequate protection of adjacent lands.

Clearing and grubbing will be performed only as necessary for the installation of the sediment control barrier. To facilitate effectiveness of the silt fencing, daily inspections and inspections immediately after significant storm events will be performed by the Contractor(s). Maintenance of the fence will be performed as needed.

### 5.1.4 Temporary Seeding

The project is authorized to disturb greater than five acres in accordance with Part I.E.5.a.viii. As such, temporary soil stabilization measures must be initiated by the end of the next business day and completed within seven (7) days from the date the soil disturbance activity has temporarily ceased.

#### 5.1.5 Erosion Control Blanket

Erosion control blankets shall be installed in accordance with manufacturer's requirements on all slopes exceeding 3:1. Erosion control blankets provide temporary erosion protection, rapid vegetative establishment, and long-term erosion resistance to shear stresses generated by high runoff flow velocities associated with steep slopes.

### 5.1.6 Dewatering Operations

Dewatering will be used to intercept sediment-laden stormwater or pumped groundwater and allow it to settle out of the pumped discharge prior to being discharged from the site. Water from dewatering operations shall be treated to eliminate the discharge of sediment and other pollutants. Water resulting from dewatering operations shall be directed to temporary sediment traps or dewatering devices. Temporary sediment traps and dewatering bags will be provided, installed, and maintained at downgradient locations to control sediment deposits to downstream surfaces.

### 5.2 Permanent Erosion and Sediment Control Measures

The permanent erosion and sediment control measures described in the following sections are included as part of the construction documents.

### 5.2.1 Establishment of Permanent Vegetation

Disturbed areas that will be vegetated must be seeded in accordance with the contract documents. The type of seed, mulch, and maintenance measures as described in the contract documents shall also be followed.

The project is authorized to disturb greater than five acres in accordance with Part I.E.5.a.viii. As such, permanent soil stabilization measures must be initiated by the end of the next business day and completed within seven (7) days from the date the soil disturbance activity has permanently ceased.

Final site stabilization is achieved when all soil-disturbing activities at the site have been completed and a uniform, perennial vegetative cover with a density of 80 percent has been established or equivalent stabilization measures (such as the use of mulches or geotextiles) have been employed on all unpaved areas and areas not covered by permanent structures.

### 5.3 Other Pollutant Controls

Part I.C.1 of GP-0-25-001 prohibits discharges from construction material wastewater, pollutants used in vehicle and equipment operation and maintenance, vehicle and equipment washing and toxic or hazardous substances.

The following table identifies materials and/or chemicals commonly used and/or stored on construction sites and should be addressed in the site-specific spill prevention and response plan:

**Table 11: Common Construction Pollutants** 

Material/Chemical	Physical Description	Stormwater Pollutants	Location*
Pesticides	Various colored to	Chlorinated	Herbicides used for
(insecticides,	colorless liquid,	hydrocarbons,	noxious weed control
fungicides,	powder, pellets, or	organophosphates,	
herbicides,	grains	carbamates, arsenic	
rodenticides)			
Fertilizer	Liquid or solid grains	Nitrogen, phosphorous	Newly seeded areas
Cleaning solvents	Colorless, blue, or	Perchloroethylene,	No equipment cleaning
	yellow-green liquid	methylene chloride,	allowed in project limits
		trichloroethylene,	
	<b>5.</b>	petroleum distillates	
Asphalt	Black solid	Oil, petroleum distillates	Streets and roofing
Concrete	White solid/grey liquid	Limestone, sand, pH,	Curb and gutter, building
		chromium	construction
Curing compounds	Creamy white liquid	Naphtha	Curb and gutter
Hydraulic oil/fluids	Brown oily petroleum	Mineral oil	Leaks or broken hoses
Gasoline	hydrocarbon	Danzana ethyl banzana	from equipment
Gasonne	Colorless, pale brown or pink petroleum	Benzene, ethyl benzene, toluene, xylene, MTBE	Secondary containment /
	hydrocarbon	toluerie, xylerie, WTBE	staging area
Diesel Fuel	Clear, blue-green to	Petroleum distillate, oil &	Secondary containment /
Diesei i dei	yellow liquid	grease, naphthalene,	staging area
	yellow liquid	xylenes	Staging area
Kerosene	Pale yellow liquid	Coal oil, petroleum	Secondary containment /
	petroleum	distillates	staging area
	hydrocarbon		
Antifreeze/coolant	Clear green/yellow	Ethylene glycol, propylene	Leaks or broken hoses
·	liquid	glycol, heavy metals	from equipment
		(copper, lead, zinc)	
Sanitary toilets	Various colored liquid	Bacteria, parasites, and	Staging area
		viruses	
Construction			
materials			
Granular fill	Various colored solids	Sediment	Stockpile / fill areas
Subbase course	Gray/brown solid	Sediment, dust	Stockpile
Topsoil	Brown solid	Sediment	Stockpile
Mulch	Various colored solid	Sediment, debris	Staging area
Seed	Brown/yellow solid	Nutrients, debris	Staging area
HDPE Storm Pipe	Black solid		Staging area
SDR-35, SDR-21	Various colored solid		Staging area
PVC Pipe  Metals Frames and	Gray solid		Stading area
Grates	Gray Sullu		Staging area
Joint Sealant	Light gray viscous	Polyurethane	Staging area
Joint Gealant	solid	1 diyarethane	Staging area
(Aroa whore material /ek		<u> </u>	l

<sup>\*(</sup>Area where material/chemical is used on-site)

### 5.4 Construction Housekeeping Practices

During the construction phase, the Contractor(s) will implement the following measures:

### 5.4.1 Sediment Sweeping/Vacuuming

Any sediment that is tracked by construction vehicles or erosion onto adjacent public or private impervious surfaces must be swept or vacuumed, utilizing self-propelled and/or walk-behind equipment, and removed on a daily basis. Kick brooms and sweeper attachments are not an acceptable means of sweeping. Sweeping or vacuuming should not take place while tracked sediment is wet. If tracked sediment is compacted, the sediment must be scraped loose prior to sweeping or vacuuming.

### 5.4.2 Material Stockpiles

Material resulting from clearing and grubbing operations that will be stockpiled on-site, must be adequately protected with downgradient erosion and sediment controls.

### 5.4.3 Equipment Cleaning and Maintenance

The Contractor(s) will designate areas for equipment cleaning, maintenance, and repair. The Contractor(s) and subcontractor(s) will utilize those areas. The areas will be protected by a temporary perimeter berm.

### 5.4.4 Detergents

The use of detergents for large-scale washing is prohibited (i.e., vehicles, buildings, pavement surfaces, etc.)

### 5.4.5 Spill Prevention and Response

A Spill Prevention and Response Plan shall be developed, for the pollutants identified in Section 5.3, for the site by the Contractor(s) that addresses the following:

- 1. Reducing chance of spills
- 2. Stopping the source of spills
- 3. Containing and cleaning up spills
- 4. Disposing of materials contaminated by spills
- 5. Training personnel responsible for spill prevention/response
- 6. Material handling procedures
- 7. Material storage requirements

The plan shall detail the steps required in the event of an accidental spill and shall identify contact names and phone numbers of people and agencies that must be notified.

The plan shall include Safety Data Sheets (SDS) for all materials to be stored on-site. All workers on-site will be required to be trained on safe handling and spill prevention procedures for all materials used during construction. Regular tailgate safety meetings shall be held and all workers that are expected on the site during the week shall be required to attend.

### 5.4.6 Concrete Washout Areas

A temporary concrete washout area shall be provided for every project where concrete will be poured or otherwise formed on-site and shall consist of an excavated or above-ground lined construction pit

where concrete trucks or equipment can be washed out after their loads have been discharged. Waste generated from concrete wash water that shall not be allowed to flow into drainage ways, inlets, receiving waters, highway right-of-way, or any location other than the designated concrete washout area(s). Proper signage shall be placed adjacent to the facility to designate the "Concrete Washout Area". Locate the facility a minimum of 100-feet from drainage swales, storm drain inlets, wetlands, streams, and other surface waters. Prevent surface water from entering the washout area.

The hardened residue from the concrete wash areas will be disposed of in the same manner as other non-hazardous construction waste materials. Maintenance of the washout area shall include removal of hardened material when 75% of the storage capacity is filled, and a minimum freeboard of 12 inches shall be maintained. The Contractor will be responsible for seeing that these procedures are followed. The project may require the use of multiple concrete washout areas based on the frequency of concrete pours.

### 5.4.7 Material Storage

Construction materials shall be stored in a dedicated staging area. The staging area shall be located in an area that prevents negative impacts of construction materials on stormwater quality.

Chemicals, paints, solvents, fertilizers, and other toxic material must be stored in waterproof containers. Except during application, the contents must be kept in trucks or within storage facilities. Runoff containing such material must be collected, removed from the site, treated, and disposed of at an approved solid waste or chemical disposal facility.

### 6.0 INSPECTIONS, MAINTENANCE, AND REPORTING

### 6.1 Inspection and Maintenance Requirements

### 6.1.1 Pre-Construction Inspection and Certification

Prior to the commencement of construction, the Qualified Inspector/Qualified Professional shall conduct an assessment of the site and certify that the appropriate erosion and sediment control measures have been adequately installed and implemented. The Contractor shall contact the Qualified Inspector/Qualified Professional once the erosion and sediment control measures have been installed.

### 6.1.2 Construction Phase Inspections and Maintenance

A Qualified Inspector/Qualified Professional, as defined in Appendix A of the General Permit GP-0-25-001, shall conduct regular site inspections between the time this SWPPP is implemented and final site stabilization. Site inspections shall occur at an interval of at least once every seven (7) calendar days.

The purpose of site inspections is to assess performance of pollutant controls. Based on these inspections, the Qualified Inspector/Qualified Professional will decide whether it is necessary to modify this SWPPP, add or relocate sediment barriers, or whatever else may be needed in order to prevent pollutants from leaving the site via stormwater runoff. The general contractor has the duty to cause pollutant control measures to be repaired, modified, maintained, supplemented, or whatever else is necessary in order to achieve effective pollutant control.

Examples of particular items to evaluate during site inspections are listed below. This list is not intended to be comprehensive. During each inspection the inspector must evaluate overall pollutant

control system performance as well as particular details of individual system components. Additional factors should be considered as appropriate to the circumstances.

- 1. Locations where vehicles enter and exit the site must be inspected for evidence of off-site sediment tracking. A stabilized construction access will be constructed where vehicles enter and exit. This access will be maintained or supplemented as necessary to prevent sediment from leaving the site on vehicles.
- 2. Sediment barriers must be inspected and, if necessary, they must be enlarged or cleaned in order to provide additional capacity. All material from behind sediment barriers will be stockpiled on the up slope side. Additional sediment barriers must be constructed as needed.
- 3. Inspections will evaluate disturbed areas and areas used for storing materials that are exposed to rainfall for evidence of, or the potential for, pollutants entering the drainage system. If necessary, the materials must be covered or original covers must be repaired or supplemented. Also, protective berms must be constructed, if needed, in order to contain runoff from material storage areas.
- 4. Grassed areas will be inspected to confirm that a healthy stand of grass is maintained. The site has achieved final stabilization once all areas are covered with building foundation or pavement, or have a stand of grass with at least 80 percent density. The density of 80 percent or greater must be maintained to be considered as stabilized. Areas must be watered, fertilized, and reseeded as needed to achieve this goal.
- 5. All discharge points must be inspected to determine whether erosion control measures are effective in preventing significant impacts to receiving waters.

The inspection reports must be completed entirely and additional remarks should be included if needed to fully describe a situation. An important aspect of the inspection report is the description of additional measures that need to be taken to enhance plan effectiveness. The inspection report must identify whether the site was in compliance with the SWPPP at the time of inspection and specifically identify all incidents of non-compliance.

Within one (1) business day of the completion of an inspection, the *Qualified Inspector/Qualified Professional* shall notify the Owner/Operator and appropriate contractor or subcontractor of any corrective actions that need to be taken. For corrective actions not requiring engineering design, the contractor must begin implementing corrective actions within one business day and complete the corrective actions within five business days. For corrective actions requiring engineering design, the engineering design process must begin within five business days and the contractor must complete the corrective action in a reasonable time frame but no later than 60 calendar days.

In addition to the inspections performed by the *Qualified Inspector/Qualified Professional*, the Contractor shall perform routine inspections that include a visual check of all erosion and sediment control measures. All inspections and maintenance shall be performed in accordance with the inspection and maintenance schedule provided on the accompanying plans. Sediment removed from erosion and sediment control measures will be exported from the site, stockpiled for later use, or used immediately for general non-structural fill.

It is the responsibility of the general contractor to assure the adequacy of site pollutant discharge controls. Actual physical site conditions or contractor practices could make it necessary to install more structural controls than are shown on the accompanying plans. (For example, localized concentrations of runoff could make it necessary to install additional sediment barriers, sediment traps, etc.) Assessing the need for additional controls and implementing them or adjusting existing controls will be a continuing aspect of this SWPPP until the site achieves final stabilization.

### 6.1.3 Temporary Suspension of Construction Activities

For construction sites where soil disturbance activities have been temporarily suspended (e.g. Winter shutdown) and temporary stabilization measures have been applied to all disturbed areas, the frequency of Qualified Inspector/Qualified Professional inspections can be reduced to once every 30 calendar days. Prior to reducing the frequency of inspections, the Owner/Operator shall notify the NYSDEC Region 8 stormwater contact person and the Town of Stafford in writing.

### 6.1.4 Partial Project Completion

For construction sites where soil disturbance activities have been shut down with partial project completion, all areas disturbed as of the project shutdown date have achieved final stabilization, and all post-construction stormwater management practices required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational, the inspections by the Qualified Inspector/Qualified Professional can stop. Prior to the shutdown, the Owner/Operator shall notify the NYSDEC Region 8 stormwater contact person and the Town of Stafford in writing.

If soil disturbance activities have not resumed within two years from the date of shutdown, a Notice of Termination (NOT) shall be properly completed and submitted to the NYSDEC.

### 6.1.5 Post-Construction Inspections and Maintenance

Inspections and maintenance of final stabilization measures and post-construction stormwater management practices shall be performed in accordance with Appendix G, once all disturbed areas are stabilized and all stormwater management systems are in place and operable.

### 6.2 Reporting Requirements

#### 6.2.1 Inspection Reports

Pursuant to Part IV.C of GP-0-25-001, inspection reports shall be prepared for the duration of construction, as outlined herein, and shall be signed by the *Qualified Inspector* or *Qualified Professional*. A sample inspection form is provided in Appendix F.

At a minimum, each inspection report shall record the following information:

- 1. Permit identification number; and
- 2. Date and time of inspection; and
- 3. Name and title of person(s) performing inspection; and
- 4. A description of the weather and soil conditions (e.g. dry, wet, saturated) at the time of the inspection, including the temperature at the time of the inspection; and
- A description of the condition of the runoff at all points of discharge from the construction site.
   This must include identification of any discharges of sediment from the construction site.
   Include discharges from conveyance systems (i.e. pipes, culverts, ditches, etc.) and overland flow; and
- 6. A description of the condition of all surface waters of the State located within, or immediately adjacent to, the property boundaries of the construction site which receive runoff from disturbed areas. This must include identification of any discharges of sediment to the surface waters of the State: and

- 7. Identification of all erosion and sediment control practices and pollution prevention measures that need repair or maintenance; and
- 8. Identification of all erosion and sediment control practices and pollution prevention measures that were not installed properly or are not functioning as designed and need to be reinstalled or replaced; and
- 9. Description and sketch (map) of areas with active soil disturbance activity, areas that have been disturbed but are inactive at the time of the inspection, and areas that have been stabilized (temporary and/or final) since the last inspection; and
- 10. Estimates, in square feet or acres, of the following areas:
  - a. Total area with active soil disturbance (not requiring either temporary stabilization or final stabilization); and
  - b. Total area with inactive soil disturbance (requiring either temporary stabilization or final stabilization); and
  - c. Total area that has achieved temporary stabilization; and
  - d. Total area that has achieved final stabilization; and
- 11. Current stage of construction of all SMPs and identification of all construction activity on site that is not in conformance with the SWPPP and technical standards; and
- 12. Corrective action(s) that must be taken to install, repair, replace or maintain erosion and sediment control practices and pollution prevention measures; and to correct deficiencies identified with the construction of the SMP(s); and
- 13. Identification and status of all corrective actions that were required by previous inspection; and
- 14. Digital photographs, with date stamp, that clearly show the condition of all practices that have been identified as needing corrective actions. The qualified inspector must attach color copies of the digital photographs to the inspection report being maintained onsite within seven (7) calendar days of the date of the inspection. The qualified inspector must also take digital photographs, with date stamp, that clearly show the condition of the practice(s) after the corrective action has been completed. The qualified inspector must attach paper color copies of the digital photographs to the inspection report that documents the completion of the corrective action work within seven (7) calendar days of that inspection.

### 6.2.2 Site Log Book

Pursuant to Part I.E.3 of GP-0-25-001, the Owner/Operator shall retain a copy of the General Permit, NOI, NOI Acknowledgment Letter, MS4 SWPPP Acceptance Form (if applicable), inspection reports, contractor and subcontractor certification forms, and all documentation necessary to demonstrate eligibility under the permit, at the construction site from commencement of construction activity until the date that all areas of disturbance have achieved final stabilization and the Notice of Termination has been submitted to the NYSDEC.

The Site Log Book shall be maintained on-site in a secure location (i.e. job trailer, on-site construction office, or mailbox with lock) and must be accessible during normal business hours to an individual performing a compliance inspection.

### 6.2.3 Post Construction Records and Archiving

Following construction, the Owner/Operator shall retain copies of the SWPPP, the complete construction Site Log Book, and records of all data used to complete the NOI to be covered by this permit, for a period of at least five years from the date that the site is finally stabilized. This period may be extended by the NYSDEC, at its sole discretion, at any time upon written notification.

Records shall be maintained of all post construction inspections and maintenance work performed in accordance with the requirements outlined in Appendix G.

### 7.0 SWPPP IMPLEMENTATION RESPONSIBILITIES

A summary of the responsibilities and obligations of all parties involved with compliance with the NYSDEC SPDES General Permit GP-0-25-001 conditions is outlined in the subsequent sections. For a complete listing of the definitions, responsibilities, and obligations, refer to the SPDES General Permit GP-0-25-001 presented in Appendix J.

### 7.1 Owner's/Operator's Responsibilities

- 1. Ensure that control measures are selected, designed, installed, implemented and maintained to minimize the discharge of pollutants and prevent a violation of the water quality standards, meeting the non-numeric effluent limitations in Part II.B.1.(a)-(e) of the SPDES General Permit and in accordance with the New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016.
- 2. Ensure that practices are selected, designed, installed, and maintained to meet the performance criteria in the Design Manual. Practices must be designed to meet the applicable sizing criteria in Part II.C.2.a., b., c. or d. of GP-0-25-001.
- 3. Retain the services of a "Qualified Inspector" or "Qualified Professional" as defined under GP-0-25-001, to provide the services outlined in Section 7.5 "Qualified Inspector's/Qualified Professional's Responsibilities."
- 4. Retain the services of a "Qualified Professional," as defined under GP-0-25-001, to provide the services outlined in Section 2.3 "Owner's/Operator's Engineers Responsibilities."
- 5. Have an authorized corporate officer sign the Owner/Operator Certification Form to accompany the eNOI. A copy of the completed NOI is included in Appendix B.
- 6. Submit the electronic version of the NOI (eNOI) along with the MS4 No Jurisdiction Form using the NYSDEC's website (<a href="http://www.dec.ny.gov/chemical/43133.html">http://www.dec.ny.gov/chemical/43133.html</a>).
- 7. Pay the required initial and annual fees upon receipt of invoices from NYSDEC. These invoices are generally issued in the fall of each year. The initial fee is calculated as \$110.00 per acre disturbed plus \$675.00 per acre of net increase in impervious cover, and the annual fee is \$110.00.
- 8. Prior to the commencement of construction activity, identify the contractor(s) and subcontractor(s) that will be responsible for installing, constructing, repairing, replacing, inspecting, and maintaining the erosion control practices included in the SWPPP and the

- contractor(s) and subcontractor(s) that will be responsible for constructing the SMPs included in the SWPPP. Each of the contractors and subcontractors must identify at least one person from their company to be the trained contractor that will be responsible for implementation of the SWPPP. Ensure that at least one trained contractor is on site daily when soil disturbance activities are being performed.
- 9. Schedule a pre-construction meeting which shall include the Town of Stafford representative, Owner's/Operator's Engineer, Qualified Inspector, Contractor, and their sub-contractors to discuss responsibilities as they relate to the implementation of this SWPPP.
- 10. Require the Contractor to fully implement the SWPPP prepared for the site by the Owner/Operator's Engineer to ensure that the provisions of the SWPPP are implemented from the commencement of construction activity until all areas of disturbance have achieved final stabilization and the Notice of Termination (NOT) has been submitted to the NYSDEC.
- 11. The Owner/Operator is authorized to commence construction activity as of the authorization date indicated in the Letter of Authorization (LOA), which is sent by NYSDEC after a complete eNOI is submitted.
- 12. Forward a copy of the LOA received from DEC to the Owner's/Operator's Engineer for project records, and to the Contractor for display at the construction site.
- 13. As of the date the LOA is received, the Owner/Operator must make the eNOI, SWPPP and LOA available for review and copying in accordance with the requirements in Part VII.H. of GP-0-25-001. When applicable, as of the date an updated LOA is received, the Owner/Operator must make the updated LOA available for review and copying in accordance with the requirements in Part VII.H.
- 14. The Owner/Operator must ensure compliance with all requirements of GP-0-25-001 and that the provisions of the SWPPP, including any changes made to the SWPPP in accordance with Part III.A.5., are properly implemented and maintained from the commencement of construction activity until all area of disturbance have achieved final stabilization; and the Owner/Operator's coverage under the permit is terminated in accordance with Part V.A.5.a.
- 15. As of the date of the commencement of construction activities until Part I.E.2.a. and b. have been met, the Owner/Operator must maintain at the construction site, a copy of all documentation necessary to demonstrate eligibility with GP-0-25-001, a copy of GP-0-25-001, the SWPPP, the signed SWPPP Preparer Certification Form, the signed MS4 SWPPP Acceptance Form, NYCDEP SWPPP Acceptance/Approval Form, MS4 No Jurisdiction Form, signed Owner/Operator Certification Form, eNOI, and LOA, and LOA transmittal to the MS4 Operator in accordance with Part I.D.3.c.
- 16. The Owner/Operator must maintain at the construction site, until Part I.E.2.a and b. have been met, as of the date the documents become final or are received, a copy of the responsible contractor's or subcontractor's certification statement(s) in accordance with Part III.A.7, and inspection reports in accordance with Part IV.C.4. and 6., and Request to Disturb Greater than Five Acres and the Authorization Letter to Disturb Greater than Five Acres in accordance with Part I.E.6, Request to Continue Coverage and the Letter of Continued Coverage (LOCC) in accordance with Part I.F.2. and 4., and the updated LOA(s) in accordance with Part I.E.9.
- 17. The Owner/Operator must maintain the documents in a secure location, such as a job trailer, on-site construction office, or mailbox with lock. The secure location must be accessible during

normal business hours to an individual performing a compliance inspection. The documents must be paper documents unless electronic documents are accessible to the inspector during an inspection to the same extent as a paper copy stored at the site would be. If electronic documents are kept on site, the Owner/Operator must maintain functional equipment on site available to an inspector during normal hours of operation such that an inspector may view the electronic documents in a format that can be red in a similar manner as a paper record and in a legally dependable format with no less evidentiary value than their paper equivalent.

- 18. Upon finding a significant non-compliance with the practices described in the SWPPP or violation of GP-0-25-001, NYSDEC may order an immediate stop to all construction activity at the site until the non-compliance is remedied. The stop work order must be in writing, describe the non-compliance in detail, and be sent to the Owner/Operator. Forward a copy of any stop work order received immediately to the Owner's/Operator's Engineer and to the Contractor.
- 19. If any human remains or archaeological remains are encountered during excavation, the Owner/Operator must immediately cease, or cause to cease, all construction activity in the area of the remains and notify the appropriate Regional Water Engineer (RWE). Construction activity shall not resume until written permission to do so has been received from the RWE.
- 20. To be authorized to implement modifications to the information previously submitted in the eNOI, the Owner/Operator must notify NYSDEC via email at <a href="Stormwater info@dec.ny.gov">Stormwater info@dec.ny.gov</a> requesting access to update the eNOI, update the eNOI to reflect the modifications and resubmit the eNOI in accordance with Part I.D., and receive an updated LOA.
- 21. The eNOI, SWPPP, LOA, updated LOAs, and inspection reports required by GP-0-25-001 are public documents that the Owner/Operator must make available for review and copying by any person within five (5) business days of the Owner/Operator receiving a written request by any such person to review these documents. Copying of documents will be done at the requester's expense.
- 22. The Owner/Operator must terminate coverage when the project reaches total project completion, has a planned shutdown with partial project completion, is changing Owner/Operator or has obtained coverage under an alternative general SPDES permit or an individual SPDES permit.
- 23. Have a qualified inspector perform a final site inspection prior to submitting the eNOT.
- 24. Prior to submitting a Notice of Termination, ensure
  - for SMP(s) that are privately owned, the Owner/Operator has a mechanism in place that requires operation and maintenance of the practice(s) in accordance with the operation and maintenance plan, such as a deed covenant in the Owner/Operator's deed of record.
- 25. Submit a complete Notice of Termination form electronically using the NYSDEC eNOT. Coverage is terminated as of the termination date indicated in the Letter of Termination (LOT), which is sent by NYSDEC after a complete eNOT is submitted.
- 26. Request and receive all SWPPP records from the Owner's/Operator's Engineer and archive those records, along with the LOT, for a period of at least five (5) years from the date that NYSDEC accepts a complete NOT submitted.

- 27. Implement the Post-Construction Inspections and Maintenance procedures outlined in Appendix G.
- 28. The Owner/Operator must keep the SWPPP current so that it at all times accurately documents the erosion and sediment controls practices that are being used or will be used during construction, and all post-construction stormwater management practices that will be constructed on the site. At a minimum, the Owner/Operator shall amend the SWPPP, including construction drawings:
  - a) Whenever the current provisions prove to be ineffective in minimizing pollutants in stormwater discharges from the project site;
  - b) Whenever there is a change in design, construction, or operation at the construction site that has or could have an effect on the discharge of pollutants; and
  - c) To address issues or deficiencies identified during an inspection by the "Qualified Inspector," the Department, or other Regulatory Authority.
  - d) To document the final construction conditions.
- 29. When property ownership changes or when there is a change in operational control over the construction plans and specifications, the original owner or operator must notify the new owner or operator, in writing, of the requirement to obtain permit coverage by submitting a NOI with the Department, in conformance with Part I.G.. For construction activities subject to the requirements of a regulated, traditional land use control MS4, the original owner or operator must also notify the MS4, in writing, of the change in ownership at least 30 calendar days prior to the change in ownership.

### 7.2 Owner's/Operator's Engineer's Responsibilities

- 1. Prepare the SWPPP using good engineering practices, best management practices, and in compliance with all federal, state, and local regulatory requirements.
- 2. Prepare the electronic Notice of Intent (eNOI) (see Appendix B) and sign the "SWPPP Preparer Certification Form." Forward the Owner/Operator Certification Form to the Owner/Operator for signature.
- 3. Provide copies of the SWPPP to the Town of Stafford once all signatures and attachments are complete.
- 4. Enter Contractor's information in Section 7.5 "SWPPP Participants" once a Contractor is selected by the Owner/Operator.
- 5. Participate in a pre-construction meeting which shall include the Choose an item. of MuniName representative, Owner/Operator, Qualified Inspector, Contractor, and all subcontractors to discuss responsibilities as they relate to the implementation of this SWPPP.
- 6. Update the SWPPP each time there is a significant modification to the pollution prevention measures or a change of the principal Contractor working on the project who may disturb site soil.

### 7.3 Contractor's Responsibilities

- 1. Sign the SWPPP Contractor's Certification Form contained within Appendix B and forward to the Owner's/Operator's Engineer for inclusion in the Site Log Book.
- 2. Identify at least one Trained Contractor that will be responsible for implementation of this SWPPP. Ensure that at least one Trained Contractor is on site on a daily basis when soil disturbance activities are being performed. The Trained Contractor shall inspect the erosion and sediment control practices and pollution prevention measures being implemented within the active work area daily to ensure that they are being maintained in effective operating conditions at all times. For corrective actions not requiring engineering design, the contractor must begin implementing corrective actions within one business day and complete the corrective actions within five business days. For corrective actions requiring engineering design, the engineering design process must begin within five business days and the contractor must complete the corrective action in a reasonable time frame but no later than 60 calendar days.
- 3. Provide the names and addresses of all subcontractors working on the project site. Require all subcontractors who will be involved with construction activities that will result in soil disturbance to identify at least one Trained Contractor that will be on site on a daily basis when soil disturbance activities are being performed; and to sign a copy of the Subcontractor's Certification Form contained within Appendix B, then forward to the Owner's/Operator's Engineer for inclusion into the Site Log Book. This information must be retained as part of the Site Log Book.
- 4. Maintain a Spill Prevention and Response Plan in accordance with requirements outlined in Section 5 of this SWPPP. This plan shall be provided to the Owner's/Operator's Engineer for inclusion in the Site Log Book, prior to mobilization on-site.
- 5. Participate in a pre-construction meeting which shall include the Town of Stafford representative, Owner/Operator, Owner's/Operator's Engineer, Qualified Inspector, and all subcontractors to discuss responsibilities as they relate to the implementation of this SWPPP.
- 6. If Contractor plans on utilizing adjacent properties for material, waste, borrow, or equipment storage areas, or if Contractor plans to engage in industrial activity other than construction (such as operating asphalt and/or concrete plants) at the site, Contractor shall submit appropriate documentation to the Owner's/Operator's Engineer so that the SWPPP can be modified accordingly.
- 7. Implement site stabilization, erosion and sediment control measures, and other requirements of the SWPPP.
- 8. In accordance with the requirements in the most current version of the NYS Standards and Specifications for Erosion and Sediment Control, conduct inspections of erosion and sediment control measures installed at the site to ensure that they remain in effective operating condition at all times. Prepare and retain written documentation of inspections as well as of all repairs/maintenance activities performed. This information must be retained as part of the Site Log Book.
- 9. Begin implementing corrective actions within one (1) business day of receipt of notification by the Qualified Inspector/Qualified Professional of any corrective actions. For corrective actions not requiring engineering design, the contractor must begin implementing corrective actions

within one business day and complete the corrective actions within five business days. For corrective actions requiring engineering design, the engineering design process must begin within five business days and the contractor must complete the corrective action in a reasonable time frame but no later than 60 calendar days.

- 10. Maintain a record of the date(s) and location(s) that soil restoration is performed in accordance with the accompanying plans and NYSDEC Division of Water's publication "Deep-Ripping and Decompaction," dated April 2008. A copy of this publication is provided in Appendix H. The record that is to be maintained shall be a copy of the overall site grading plan delineating the area(s) and date(s) that the soil was restored.
- 11. Upon completion of all construction at the site, the contractor responsible for overall SWPPP Compliance shall sign the certification on their Contractor Certification Form indicating that:
  a.) all temporary erosion and sediment control measures have been removed from the site, b.) the on-site soils disturbed by construction activity have been restored in accordance with the SWPPP and the NYSDEC Division of Water's publication "Deep-Ripping and Decompaction," and c.) all permanent stormwater management practices required by the SWPPP have been installed in accordance with the contract documents.

### 7.4 Qualified Inspector's/Qualified Professional's Responsibilities

- 1. Participate in a pre-construction meeting with the Town of Stafford representative, Owner/Operator, Owner/Operator's Engineer, Contractor, and their subcontractors to discuss responsibilities as they relate to the implementation of this SWPPP.
- Conduct an initial assessment of the site prior to the commencement of construction and certify in an inspection report that the appropriate erosion and sediment control measures described within this SWPPP have been adequately installed and implemented to ensure overall preparedness of the site.
- 3. Provide on-site inspections to determine compliance with the SWPPP. Site inspections shall occur at an interval of at least once every seven calendar days. A written inspection report shall be provided to the Owner/Operator and general contractor within one business day of the completion of the inspection, with any deficiencies identified. A sample inspection form is provided in Appendix F.
- 4. Prepare an inspection report subsequent to each and every inspection that shall include/address the items listed in Part IV.C.4 of GP-0-25-001. Sign all inspection reports and maintain on site with the SWPPP.
- 5. Notify the owner/operator and appropriate contractor or subcontractor of any corrective actions that need to be taken.
- 6. Prepare a construction Site Log Book to be used as a record of all inspection reports generated throughout the duration of construction. Ensure that the construction Site Log Book is maintained and kept up-to-date throughout the duration of construction.
- 7. Review the Contractor's SWPPP records on a periodic basis to ensure compliance with the requirements for daily reports, soil restoration, inspections, and maintenance logs.

- 8. Prepare the Notice of Termination (NOT). The Qualified Professional shall sign the NOT Certifications VI (Final Stabilization) and VII (Post-construction Stormwater Management Practices) and forward the NOT to the Owner/Operator for signature on Certification VIII (Owner/Operator Certification).
- 9. The owner's or operator's coverage is terminated as of the termination date indicated in the Letter of Termination (LOT), which is sent by NYSDEC after a complete eNOT is submitted.
- 10. Transfer the SWPPP documents, along with all NOI's, LOA, permit certificates, NOT's, LOT, construction Site Log Book, and written records required by the General Permit to the Owner/Operator for archiving.

### 7.5 SWPPP Participants

1	Owners/Operator's Engin	LaBella 300 St Roches	eenier, PE a Associates, DPC ate Street, Suite 201 ster, NY 14614 (585) 454-6110
2	. Owner/Operator <sup>3</sup> :	NY CD0 Janet V 800 G6 Housto	G Genesee 5, LLC G Genesee 6, LLC Vard essner Rd, Suite 700 In, TX 77024 303-991-5883
3	. Contractor <sup>4,6</sup> :	Name and Title:	
		Company Name:	
		Mailing Address:	
		Phone:	
		Fax:	

<sup>&</sup>lt;sup>2</sup> Refer to Appendix B for the SWPPP Preparer Certification Form.

<sup>&</sup>lt;sup>3</sup> Refer to Appendix B for the Owner/Operator Certification Form.

 $<sup>^{\</sup>rm 5}\,\rm Refer$  to Appendix B for Contractor and Subcontractor Certification Form.

 $<sup>^{6}</sup>$  Contractor's information to be entered once the Contractor has been selected.



## APPENDIX A: FIGURES

A-1: Site Location Map
A-2: Soils Map
A-3: Historic Places Screening Map
A-4: Environmental Resource Map
A-5: FEMA Firm Map



Source: Esri, Maxar, GeoEye, Earthstar Geographics, USDA, USGS, AeroGRID, IGN, and the GIS User Commi

1:18,056

November 10, 2021

.aBella

300 STATE STREET ROCHESTER, NY 14614 P: (585) 454-6110 F: (585) 454-3066 www.labellapc.com

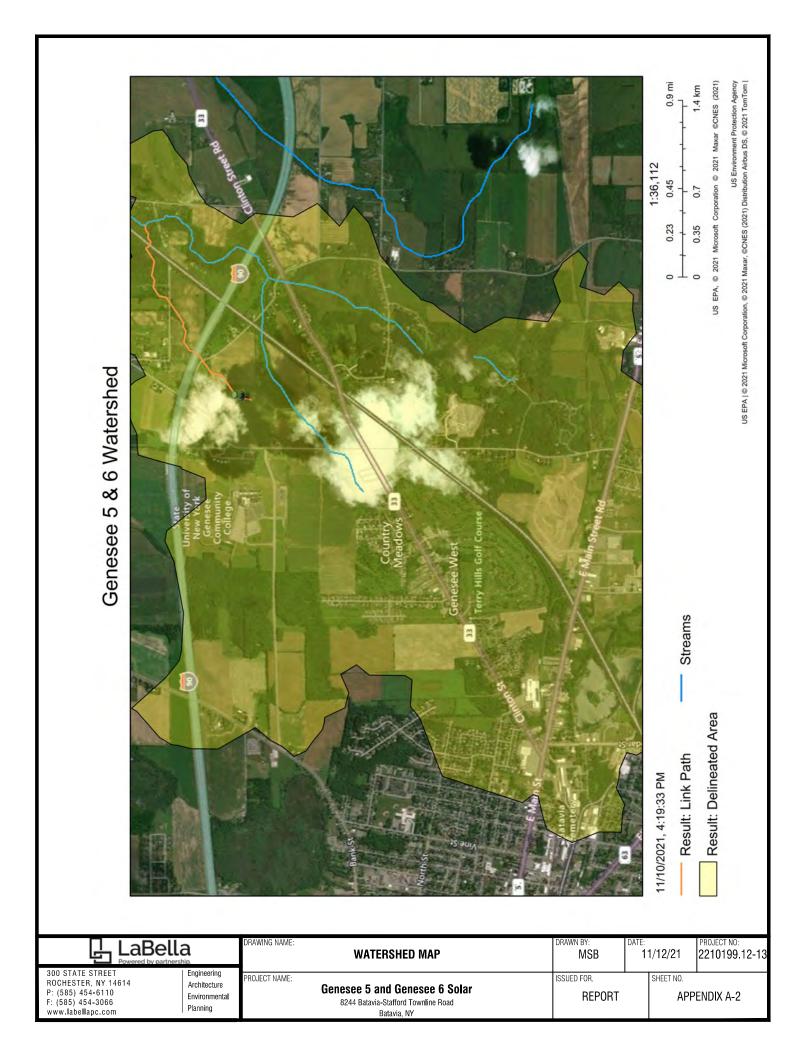
Genesee 5 & 6 Solar

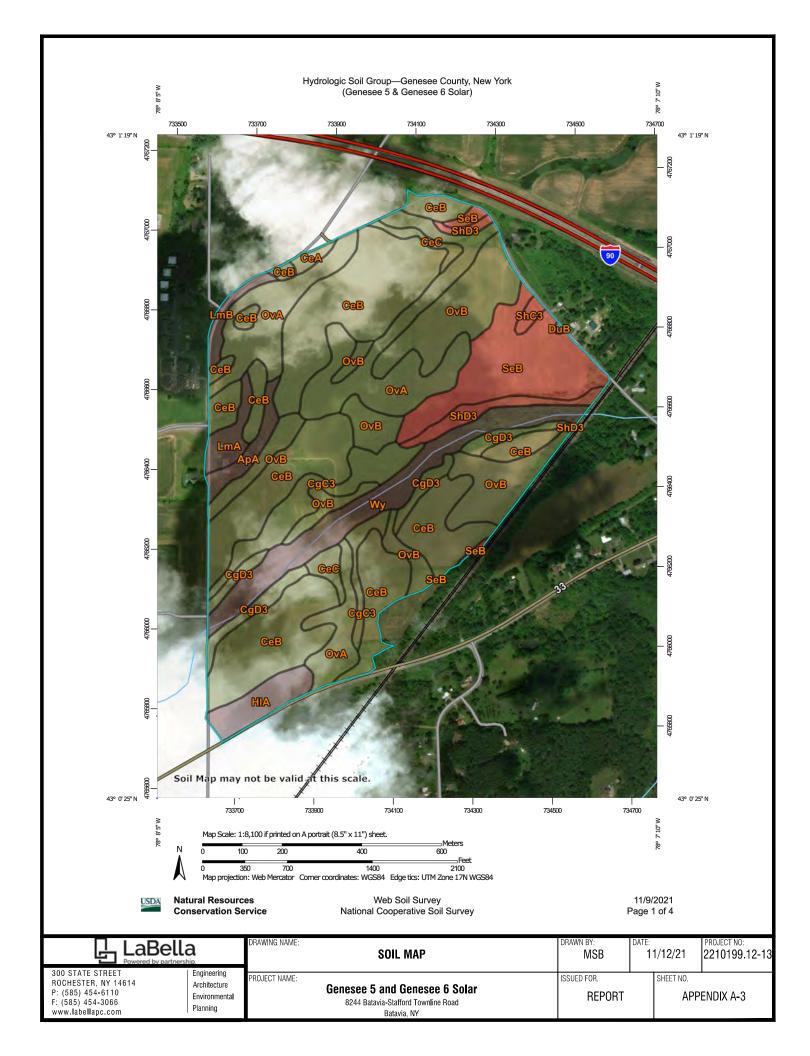
Engineering Architecture Environmental Planning

MSB

PROJECT NO: 2210199.12-13

**NYSDEC'S STORMWATER INTERACTIVE MAP** 11/12/21 PROJECT NAME: ISSUED FOR. SHEET NO. Genesee 5 and Genesee 6 Solar **REPORT** APPENDIX A-1 8244 Batavia-Stafford Townline Road Batavia, NY





1:18,056
0.1
0.2
0.4 mi
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0.35
0.17 ki
Source: Esri, Maxar, GeoEye, Earthslar Geographics, CNES/Airbus C USDA, USGS, AeroGRID, IGN, and the GIS User Community, Esri, HEE

November 10, 2021

DRAWING NAME:

ENVIRONMENTAL RESOURCE MAPPER

PROJECT NAME:

PROJECT NAME:

Genesee 5 and Genesee 6 Solar 8244 Batavia-Stafford Townline Road Batavia, NY ISSUED FOR.

REPORT

MSB

E: PROJECT NO: 2210199.12-13

APPENDIX A-4

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Engineering Architecture Environmental Planning

## 6 Solar య Genesee 5

# National Wetlands Inventory U.S. Fish and Wildlife Service



**Estuarine and Marine Deepwater** 

**Estuarine and Marine Wetland** 

Freshwater Forested/Shrub Wetland Freshwater Emergent Wetland

Freshwater Pond

National Wetlands Inventory (NWI) This page was produced by the NWI mapper

Riverine Other Lake

Wetlands

PROJECT NO: 2210199.12-13 11/12/21

300 STATE STREET ROCHESTER, NY 14614 (585) 454-6110 F: (585) 454-3066 www.labellapc.com

Engineering Architecture Environmental Planning

.aBella

PROJECT NAME:

RAWING NAME

Genesee 5 and Genesee 6 Solar 8244 Batavia-Stafford Townline Road

**FEDERAL WETLANDS** 

Batavia, NY

ISSUED FOR. **REPORT** 

DRAWN BY:

MSB

APPENDIX A-5

SHEET NO.

Commissioner Pro Tempore



March 31, 2025

Kristina Garenani 300 State Street Suite 201 Rochester, NY 14614

Re: USACE

Genesee 5 Solar and Genesee 6 Solar/5MW 62.3 Acres of a Combined 127 Acre Parcels

8244 Batavia Stafford Townline Rd (Parcels 01-02-117.1 and 01-02-115),

Stafford, Genesee County

21PR07343

Dear Kristina Garenani:

Thank you for your requesting the comments of the New York State Historic Preservation Office (SHPO). We have reviewed the provided documentation in accordance with Section 106 of the National Historic Preservation Act of 1966. These comments are those of the SHPO and relate only to Historic/Cultural resources. They do not include other environmental impacts to New York State Parkland that may be involved in or near your project.

We note that 5549 Clinton Street Road is eligible for listing in the State and National Registers of Historic Places. We have reviewed the submission received on March 4, 2025, including the letter dated March 4, 2025. Based on that review, it is the SHPO's opinion that the project, as described, will have No Adverse Effect on historic or archaeological resources.

If you have any questions, you can call or e-mail me at the contact information below.

Sincerely,

Robyn Sedgwick

Historic Site Restoration Coordinator e-mail: robyn.sedgwick@parks.ny.gov



## APPENDIX B: FORMS

Notice of Intent (NOI)
SWPPP Preparer Certification Form
Owner/Operator Certification Form
Contractor and Subcontractor Certification Forms
Notice of Termination (NOT)



## Construction General Permit (CGP) Electronic Notice of Intent (eNOI) GP-0-25-001

version 1.11

(Submission #: HQF-1GEZ-1JY77, version 1)

#### **Details**

Originally Started By Mark Potter

Alternate Identifier Gensee 5 & 6 Solar Projects—Region 8

Submission ID HQF-1GEZ-1JY77

Status Draft

#### **Form Input**

#### **Eligibility**

#### **Disturbance Threshold**

1. Will the construction activity involve soil disturbances listed in Part I.A.1 of GP-0-25-001?

Yes

- 1.a. Will any runoff from the site enter a sewer system classified as a combined sewer? No
- 1.b. Is this a remediation project being done under a Department approved work plan (i.e. CERCLA, RCRA, Voluntary Cleanup Agreement, etc.) with a SWPPP which meets the substantive requirements of GP-0-25-001?
  Yes
- 1.b.i. Is this eNOI submission only for the remediation work?

1.c. Is the construction activity related to a stormwater discharge that does not require
permit as described in 40 CFR 122.3(e), e.g. non-point source agriculture or silviculture
activities?

No

#### **Other SPDES Permits**

2. Will the discharge from the construction activity meet all conditions listed in Part I.A.2 of GP-0-25-001?

Yes

**Threatened and Endangered Species** 

3. Will the construction activity potentially adversely affect a species that is endangered or threatened per Part I.A.3.?

No

**State Historic Preservation Act (SHPA)** 

4. Is the construction activity designated by the Commissioner of the Office of Parks, Recreation and Historic Preservation (OPRHP), pursuant to 9 NYCRR §§428.12 or 428.13 as exempt from the SHPA review (see Attachment 2 of the Letter of Resolution between NYSDEC and OPRHP, dated January 9, 2015)?

Yes

**State Environmental Quality Review (SEQR)** 

- 5. Is the construction activity subject to SEQR (Part I.A.5.), or the equivalent environmental review from another NYS or federal agency (Part I.A.6.)? Yes
- 5.a. Has the owner/operator obtained documentation that the project review pursuant to SEQR, or the equivalent, has been satisfied per Part I.A.5. or I.A.6. of GP-0-25-001? Yes

**Uniform Procedures Act (UPA) Permits** 

6. Has the owner/operator obtained all necessary UPA permits from NYSDEC, or the equivalent from another NYS or federal agency per Part I.A.7.a. of GP-0-25-001? Yes

Steep Slope

7. Is the construction activity within the watershed of surface waters of the State classified as AA or AA-S identified utilizing the Stormwater Interactive Map on NYSDEC's website?

No

#### **Owner/Operator Information**

#### 8. Owner/Operator Name

NY CDG Genesee 5, LLC & NY CDG Genesee 6, LLC

9. Owner/Operator Contact Person Information

First and Last Name	Phone	E-mail
Janet Ward	303-991-5883	janet.ward@catalyze.com

#### 10. Owner/Operator Mailing Address

800 Gessner Rd, Suite 700

Houston, TX 77024

**USA** 

11. Is the billing contact different from the Owner/Operator Contact?

No

12. What type of organization is the owner/operator?

Private Owner

#### Site Information

#### 13. Project/Site Name

Gensee 5 & 6 Solar Projects

#### 14. Site Address

8244 Batavia Stafford Townline Rd

Batavia, NY 14020

Genesee

#### **DEC Region**

8

#### 15. Site Latitude & Longitude

43.015803189220605,-78.12992296134644

#### **Project Details**

#### 16. This eNOI submission is for:

A construction activity not part of a common plan of development or sale in accordance with Part I.D.1.a.

17. Does the project type fall under Table 1 or Table 2 of Appendix B of GP-0-25-001? If any portion of the construction activity falls under Table 2, regardless of the size of the disturbance, select "Table 2".

Table 2

18. Consistent with Part III.B.1.c.i. of GP-0-25-001, provide a concise overview of the project. Describe existing and proposed conditions, and include any other relevant information.

Installation of two 5MWAC ground mounted solar arrays. The Solar Projects are called Genesee 5 Solar Project and Genesee 6 Solar Project. The two solar projects share a property boundary and will be permitted and constructed simultaneously.

Enter the total project site acreage, the acreage to be disturbed, and the future impervious area (acreage) within the disturbed area, rounded to the nearest tenth of an acre.

#### 19. Total Site Area (acres)

54.0

20. Total Area to be Disturbed (acres)

65.0

21. Existing Impervious Area to be Disturbed (acres)

0.6

22. Future Impervious Area Within Disturbed Area (acres)

1.4

The total area to be disturbed is greater than the site area. Please review the responses to Questions 19 and 20 to ensure the correct values are provided.

#### Nature of the project:

Redevelopment with increase in impervious area

23. Do you plan to disturb more than 5 acres of soil at any one time?

No

24. Indicate the percentage (%) of each Hydrologic Soil Group(HSG) at the site.

<b>A</b> 0	(%)
<b>B</b> 0	(%)
<b>C</b> 0	(%)
<b>D</b> 10	<b>(%)</b> )0

25. Enter the planned start and end dates of the disturbance activities.

#### **Start Date**

11/01/2025

#### **End Date**

07/01/2026

26. Identify the nearest surface waterbody(ies) to which construction site runoff will discharge.

unnamed tributary to Bigelow Creek

27. Type of waterbody identified in question 26?

Stream/Creek On Site

28. Has the surface waterbody in question 26 been identified as a 303(d) segment in Appendix D of GP-0-25-001?

Yes

29. Is this project located in one of the Watersheds identified in Appendix C of GP-0-25-001?

No

30. Will the project disturb soils within a State regulated wetland or the protected 100 foot adjacent area?

No

31. Does the site runoff enter a separate storm sewer system (including roadside drains, swales, ditches, culverts, etc)?

No

32. Will future use of this site be an agricultural property as defined by the NYS Agriculture and Markets Law?

Yes

33. Is this property owned by a state authority, state agency, federal government or local government?

No

#### Required SWPPP Components

#### **General SWPPP Requirements**

34. Has a SWPPP been developed in conformance with the requirements in Part III. of GP-0-25-001?

Yes

35. Does the SWPPP demonstrate consideration of the future physical risks due to climate change pursuant to the CRRA, 6 NYCRR Part 490, and associated guidance per Part III.A.2. of GP-0-25-001?

Yes

36. Has the required Erosion and Sediment Control component of the SWPPP been developed in conformance with the current NYS Standards and Specifications for Erosion and Sediment Control (aka Blue Book)?

Yes

37. Has the post-construction stormwater management practice component of the SWPPP been developed in conformance with the NYS Stormwater Management Design Manual?

Yes

37.a. Which version of the NYS Stormwater Management Design Manual was used to develop the SWPPP?

2024

#### **SWPPP Preparer**

39. The Stormwater Pollution Prevention Plan (SWPPP) was prepared by:

Professional Engineer (P.E.)

40. Name of the person who prepared the SWPPP

Bob Steehler, PE

#### 41. SWPPP Preparer Organization Name

LaBella Associates

42. SWPPP Preparer Contact Information

First and Last Name	Phone	E-mail
Bob Steehler	(585) 298-9340	mtshimanga@labellapc.com

#### 43. SWPPP Preparer Address

300 State Street

Rochester, NY 14614

#### **Download SWPPP Preparer Certification Form**

Please take the following steps to prepare and upload your preparer certification form:

- 1) Click on the link below to download a blank certification form
- 2) The certified SWPPP preparer should sign this form
- 3) Upload the completed form

Download SWPPP Preparer Certification Form

#### 44. Please upload the SWPPP Preparer Certification

NONE PROVIDED

#### Comment

NONE PROVIDED

## 44.a. Has the SWPPP Preparer Certification Form been signed by the SWPPP preparer in accordance with Part VII.J of GP-0-25-001? Yes

#### **Erosion & Sediment Control Criteria**

## 45. Has a construction sequence schedule for the planned management practices been prepared?

Yes

#### Post-Construction Criteria

#### Site Planning and Soil Restoration

## 46. Identify all site planning practices that were used to prepare the final site plan/layout for the project.

Preservation of Undisturbed Area

Preservation of Buffers

Reduction of Clearing and Grading

Roadway Reduction

Locating Development in Less Sensitive Areas

**Driveway Reduction** 

**Building Footprint Reduction** 

## 47. Indicate which of the following soil restoration criteria was used to address the requirements in Section 5.1.6 ("Soil Restoration") of the Design Manual.

All disturbed areas will be restored in accordance with the Soil Restoration requirements in Table 5.3 of the Design Manual (see page 5-22).

#### Water Quality Criteria

#### 49. Water Quality Sizing Criteria

Total WQv required (acre- feet)	Total RRv provided (acre- feet)	Minimum RRv (acre-feet)	Total WQv provided (acre- feet)	Sum of RRv and WQv provided
.127	.123	.023	.004	0.127

#### Water Quantity Criteria

50. Per Section 9.2.1.C.VI and VII of the 2024 Design Manual, is there 0% change to hydrology that increases the discharge rate and volume from the project site? Yes

#### **Operation and Maintenance**

53. Has a long-term Operation and Maintenance Plan for the post-construction stormwater management practice(s) been developed?
Yes

**53.a.** Identify the entity responsible for the long-term Operation and Maintenance. Owner

#### **Post-Construction SMP Identification**

54. Post-Construction RR Techniques and Standard SMPs

RR Techniques and SMPs	Contributing Impervious Area (acres)	Total Contributing Area (acres)
Sheet Flow to Riparian Buffers or Filter Strips (RR-2)	0.970	2.560
Filtration Bioretention (F-5)	0.430	
Dry Swale (O-1)	0.060	

#### 55. Alternative SMPs

Type of Alternative SMP	Manufacturer of the Alternative SMP	Name of the Alternative SMP	Contributing Impervious Area (acres)
NONE PROVIDED	NONE PROVIDED	NONE PROVIDED	NONE PROVIDED

#### Other Permits

**56.** Identify other permits, existing and new, that are required for this project/facility. USACE Wetland Permit

56.a. Indicate the Size of Impact from the USACE Wetland Permit, in acres, to the nearest tenth.

0.027

57. Is this NOI for a change in owner/operator per Part I.G.?

#### MS4 SWPPP Acceptance

59. Will the construction activities be within the municipal boundary(ies) of Traditional Land Use Control MS4 Operator(s) and discharge to the MS4(s)?

No

#### **Owner/Operator Certification**

#### Owner/Operator Certification Form Download

Download the Owner/Operator Certification Form by clicking the link below. Owner/Operator Certification Form

#### 61. Upload Owner/Operator Certification Form

NONE PROVIDED

#### Comment

NONE PROVIDED

61.a. Has the Owner/Operator Certification Form from Appendix J been signed by the owner/operator, or a representative of the owner/operator in accordance with Part VII.J of GP-0-25-001 and uploaded to the eNOI?

Yes

#### **Additional Project Information**

62. Enter any additional pertinent project information in the text box below.

NONE PROVIDED

9 of 9



## **SWPPP Preparer Certification Form**

## SPDES General Permit for Stormwater Discharges from Construction Activity, GP-0-25-001 (CGP)

(In accordance with CGP Part I.D.2.b., the completed form must be attached to the eNOI and submitted to NYSDEC electronically.)

Project/Site Name:	Genesee 5 & Genesee 6 Solar Projects			
eNOI Submission ID:	HQF-1GEZ-1JY77			
Owner/Operator Name:	NY CDG Genesee 5 & NY CDG Genesee 6			
prepared in accordance with of law that the SWPPP and supervision in accordance properly gather and evaluates person or persons who magathering the information, than delief, true, accurate, a	rmwater Pollut th the requirem I all attachmen with a system te the informat nage the syste he information and complete. se information,	tion Prevention Plan (SWPPP) has been nents of GP-0-25-001. I certify under penalty its were prepared under my direction or designed to assure that qualified personnel tion submitted. Based on my inquiry of the em, or those persons directly responsible for submitted is, to the best of my knowledge I am aware that there are significant including the possibility of fine and		
Bob		Steehler		
SWPPP Preparer First Nar	me MI	SWPPP Preparer Last Name		
AA1ts		8/22/2025		

Date

Revised: January 2025

Signature

Revised: January 2025



## Owner/Operator Certification Form

SPDES General Permit for Stormwater Discharges from Construction Activity, GP-0-25-001 (CGP)

(In accordance with CGP Part I.D.2.b. or Part I.F.2. and 3., the completed form must be attached to the eNOI or the Request to Continue Coverage, and submitted to NYSDEC electronically.

Project/Site Name:	Genesee 5	& Gene	see 6 So	olar Pro	ojects
eNOI Submission ID:	HQF-1GEZ	Z-1JY7	7		<del></del>
eNOI Submitted by:	Owner/Ope	rator	SWPPP P	reparer	Other
Certification Statem	nent - Owner/Op	erator			
I hereby certify that I read authorization to discharge Letter of Authorization (L Department of Environme I am aware that there are fine and imprisonment fo	e under the permit for OA) or a Letter of Content of Conservation (expension significant penalties	or the project ontinued Cov NYSDEC) in s for submitti	/site named ab /erage (LOCC) accordance wi	ove is deper from the Ne th CGP Part	ndent on receipt of a w York State I.D.3.b. or Part I.F.4.
Owner/Operator First N	lame	MI	Owner/Opera	ator Last Na	ame
NYECTTO COLERAN SECTION NYECTT	C and NY CDG Gene	esee 6 LLC, b	y: LamPhung N	Igo-Burns, C	Chief Financial Officer
Signature					
8/22/2025					
Date					

#### Stormwater Pollution Prevention Plan Contractor Certification Statement (Responsible for overall SWPPP Compliance)

Project Name Project Address, Town of X, X County, New York

This is to certify that the following contracting firm will be responsible for installing, constructing, repairing, inspecting and/or maintaining the erosion and sediment control practices and post-construction stormwater management control practices required by the SWPPP.

Contracting Firm Info	rmation
Name:	<u> </u>
Address:	
Telephone & Fax:	
Trained Contractor(s)	Responsible for SWPPP Implementation (Provide name, title, and date of last training)
Prior to commenceme	ent of construction activity, the following certification shall be issued:
implement any correc or operator must cor Elimination System (S that it is unlawful for a	penalty of law that I understand and agree to comply with the requirements of the SWPPP and agree to tive actions identified by the qualified inspector during a site inspection. I also understand that the owner inply with the requirements of the most current version of the New York State Pollutant Discharge PDES) Construction General Permit (CGP) for Stormwater Discharges from Construction Activities and any person to cause or contribute to a violation of water quality standards. Furthermore, I am aware that enalties for submitting false information, that I do not believe to be true, including the possibility of fine knowing violations
Printed Name:	
Title/Position:	
Signature: _	Date:
Upon completion of c	construction activities, the following certification shall be issued, prior to issuance of the NOT:
accordance with the c been removed from the	at all permanent stormwater management practices required by the SWPPP have been installed in ontract documents. I further certify that all temporary erosion and sediment control measures have ne site, and that the on-site soils disturbed by construction activity have been restored in accordance the NYSDEC Division of Water's publication "Deep-Ripping and Decompaction".
Printed Name:	
Title/Position:	
Signature:	Date:

<sup>&</sup>lt;sup>1</sup> "Trained Contractor" means an employee from a contracting (construction) company that has received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity. After receiving the initial training, the "trained contractor" shall receive four (4) hours of training every three (3) years. It can also mean an employee from the contracting (construction) company that meets the "qualified inspector" qualifications (e.g. licensed Professional Engineer, Certified Professional in Erosion and Sediment Control (CPESC), Registered Landscape Architect, New York State Erosion and Sediment Control Certificate Program holder, or someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided they have received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity). The "Trained Contractor" will be responsible for the day to day implementation of the SWPPP.

#### Stormwater Pollution Prevention Plan Subcontractor Certification Statement (whose work involves soil disturbance)

Project Name Project Address, Town of X, X County, New York

Each Subcontractor whose work will involve soil disturbance of any kind is required to complete and sign this Certification Statement before commencing any construction activity at the site. This completed Certification Statement(s) shall be maintained at the construction site in the Site Log Book.

Subcontracting Firm	Information
Name:	
Address:	
Telephone & Fax:	
Trained Contractor(s)	<sup>2</sup> Responsible for SWPPP Implementation (Provide name, title, and date of last training)
Prior to commencem	ent of construction activities, the following certification shall be issued:
implement any correct or operator must con Elimination System (S that it is unlawful for a	penalty of law that I understand and agree to comply with the requirements of the SWPPP and agree to tive actions identified by the qualified inspector during a site inspection. I also understand that the owner mply with the requirements of the most current version of the New York State Pollutant Discharge (PDES) Construction General Permit (CGP) for Stormwater Discharges from Construction Activities and any person to cause or contribute to a violation of water quality standards. Furthermore, I am aware that enalties for submitting false information, that I do not believe to be true, including the possibility of fine knowing violations
Printed Name:	
Title/Position:	<u> </u>
Signature:	Date:

<sup>&</sup>lt;sup>2</sup> "Trained Contractor" means an employee from a contracting (construction) company that has received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity. After receiving the initial training, the "trained contractor" shall receive four (4) hours of training every three (3) years. It can also mean an employee from the contracting (construction) company that meets the "qualified inspector" qualifications (e.g. licensed Professional Engineer, Certified Professional in Erosion and Sediment Control (CPESC), Registered Landscape Architect, New York State Erosion and Sediment Control Certificate Program holder, or someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided they have received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity). The "Trained Contractor" will be responsible for the day to day implementation of the SWPPP.



#### **eNOT Owner or Operator Certification**

for construction activities seeking termination from the

## SPDES General Permit for Stormwater Discharges from Construction Activity, GP-0-25-001 (CGP)

(The completed form must be attached to the eNOT, which must be submitted to NYSDEC electronically in accordance with CGP Part V.A.5.)

#### I. Project Owner/Operator Information

- a. Owner/Operator Name:
- b. Contact Person:
- c. Street Address:
- d. City/State/Zip:

#### II. Project Site Information

- a. Project/Site Name:
- b. Street Address:
- c. City/State/Zip:
- d. CGP SPDES Permit ID:

#### III. Certification Statement

I certify that I have met the requirements of CGP Part V.A.1., 2., 3., and 4. I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

- a. Printed name of the Owner or Operator:
- b. Title/Position:
- c. Signature:
- d. Date:



#### eNOT Qualified Inspector Certification – Final Stabilization

for construction activities seeking termination from the

## SPDES General Permit for Stormwater Discharges from Construction Activity, GP-0-25-001 (CGP)

(The completed form must be attached to the eNOT, which must be submitted to NYSDEC electronically in accordance with CGP Part V.A.5.)

#### I. Project Owner/Operator Information

- a. Owner/Operator Name:
- b. Contact Person:
- c. Street Address:
- d. City/State/Zip:

#### II. Project Site Information

- a. Project/Site Name:
- b. Street Address:
- c. City/State/Zip:
- d. CGP SPDES Permit ID:

#### III. Certification Statement

I hereby certify that all the requirements in CGP Part V.A.1.a.i., ii., and iii. or CGP Part V.A.1.b.i., ii., and iii. have been achieved. I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

- a. Printed name of the Qualified Inspector:
- b. Title/Position:
- c. Signature:
- d. Date:



#### **eNOT Qualified Inspector Certification – SMPs**

for construction activities seeking termination from the

## SPDES General Permit for Stormwater Discharges from Construction Activity, GP-0-25-001 (CGP)

(The completed form must be attached to the eNOT, which must be submitted to NYSDEC electronically in accordance with CGP Part V.A.5.)

I. Project	t Owner/O	perator In	formation
------------	-----------	------------	-----------

- a. Owner/Operator Name:
- b. Contact Person:
- c. Street Address:
- d. City/State/Zip:

#### II. Project Site Information

- a. Project/Site Name:
- b. Street Address:
- c. City/State/Zip:
- d. CGP SPDES Permit ID:

#### III. Certification Statement

I hereby certify that all the requirements in CGP Part V.A.1.a.iv. or CGP Part V.A.1.b.iv. have been achieved. I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

- a. Printed name of the Qualified Inspector:
- b. Title/Position:
- c. Signature:
- d. Date:



## APPENDIX C: PROJECT EVALUATION



#### **NOI QUESTIONS**

#	NOI Question	Reported Value		
28	Total Water Quality Volume (WQv) Required	5545	0.127	
30	Total RRV Provided	5374	0.123	
31	Is RRv Provided ≥WQv Required?	No		
32	Minimum RRv	1002	0.023	
32a	Is RRv Provided ≥ Minimum RRv Required?  Yes			
33a	Total WQv Treated	171	0.004	
34	Sum of Volume Reduced & Treated	5545	0.127	
35	Is Sum RRv Provided and WQv Provided ≥WQv Required? Yes			

#### **NOI QUESTIONS**

96.91%

## Step 1 - Site Planning

	Practice	Description	Applicable	Project Specific Evaluation
Preservation of Natural Features and Conservation Design	Preservation of Undisturbed Areas	Delineate and protect undisturbed forests, native vegetated areas, riparian corridors, water bodies, wetlands, and natural terrain.	Yes	EXAMPLES - Approximately XX+/- Acres of land will remain undisturbed, in its natural state, which accounts for XX% of the total project parcel. OR The project does not propose permanent conservation of this area at this time.
	Preservation of Buffers	Delineate and protect naturally vegetated buffers along perennial streams, rivers, shorelines, and wetlands.	Yes	EXAMPLES - There is a stream/river/shorline/wetland located on the project site. For the area adjacent to this water body, a XX-ft naturally vegetated buffer will be applied. No disturbance will occur within this buffer. OR The only disturbance that will occur within this buffer will include XX. This disturbance is necessary because XX. OR There are no water bodies located on or adjacent to the project site.
	Reduction of Clearing and Grading	Limit clearing and grading to the minimum amount needed for roads, driveways, foundations, utilities and stormwater management facilities.	Yes	EXAMPLES - Clearing and grading will be limited to the area of disturbance and will be minimized to the greatest extent practical. The limits of all proposed clearing will be demarcated in the field with orange construction fencing, prior to construction, to prevent unnecessary removal of trees.
	Locating Development in Less Sensitive Areas	Avoid sensitive resource areas such as floodplains, steep slopes, erodible soils, wetlands, mature forests and critical habitats by locating development to fit the terrain in areas that will create the least impact.	Yes	EXAMPLES - The site has been designed to avoid sensitive resource areas to the greatest extent practical. Include project specific information for the various sensitive resources.  OR There are no floodplains, steep slopes, erodible soils, wetlands, mature forests or critical habitats located on the project site.
	Open Space Design	Use clustering, conservation design or open space design to reduce impervious cover, preserve more open space and protect water resources.	N/A	EXAMPLES - The site layout has been designed to maximize open space. Impervious surfaces have been minimized to the greatest extent practical and approximately XX+/- Acres will be maintained as vegetated open space.
	Soil Restoration	Restore the original properties and porosity of the soil by deep till and amendment with compost to reduce the generation of runoff and enhance the runoff reduction performance of post construction practices.	Yes	EXAMPLES - All disturbed areas will be restored in accordance with the Soil Restoration requirements in Table 5.3 of the Design Manual.  OR Compacted areas were considered as impervious cover when calculating the WQv Required and the compacted areas were assigned a post-construction HSG designation that is one level less permeable than existing conditions for the hydrology analysis.

## Step 1 - Site Planning

	Roadway Reduction	Minimize roadway widths and lengths, below local requirements, to reduce site impervious area	Yes	EXAMPLES - Proposed roadways will be constructed with a porous XX surface, in order to minimize the overall impervious surface. <b>OR</b> The project proposes to reduce the roadway width to XX-ft, from the required XX-ft. <b>OR</b> Reducing the roadway width is not feasible for the project's intended use. <b>OR</b> No new roadways are proposed as part of this project.
- 1	Sidewalk Reduction	Minimize sidewalk lengths and widths, below local requirements, to reduce site impervious area	N/A	EXAMPLES - All pedestrian walkways will be constructed with a porous XX surface. OR Sidewalk widths have been minimized to X-ft and quantity of sidewalk has been reduced to that required to ensure safe pedestrian access throughout the site. OR Reducing the sidewalk width/length is not feasible for the project's intended use. OR No new sidewalks are proposed as part of this project.
- 1	Driveway Reduction	Minimize driveway lengths and widths, below local requirements, to reduce site impervious area	Yes	EXAMPLES - All driveways on-site will be constructed with a permeable XX surface, where feasible, in order to minimize the overall impervious surface. <b>OR</b> The project proposes to reduce the driveway width to XX-ft, instead of the required XX-ft. <b>OR</b> Reducing the driveway width is not feasible for the intended use. <b>OR</b> No new driveways are proposed as part of this project.
- 1	Cul-de-sac Reduction	Minimize the number of cul-desacs and incorporate landscaped areas to reduce their impervious cover.	N/A	EXAMPLES - Proposed cul-de-sacs will utilize a landscaped island to reduce impervious cover.  OR No cul-de-sacs are proposed as part of this project.
- 1	Building Footprint Reduction	Reduce the impervious footprint of residences and commercial buildings by using alternate or taller buildings while maintaining the same floor to area ratio.	Yes	EXAMPLES - All new building area has been allocated to efficiently implement the intended use. <b>OR</b> No new buildings are proposed as part of this project. <b>OR</b> The current proposal involves the renovation and reuse of XX existing buildings on-site.
	Parking Reduction	Reduce imperviousness on parking lots by eliminating unneeded spaces, providing compact car spaces and efficient parking lanes, reducing stall dimensions below local requirements, using porous pavement surfaces in overflow parking areas, and using multistoried parking decks where appropriate.	N/A	EXAMPLES - All parking lots will be constructed of a porous XX surface, to reduce overall impervious surfaces. <b>OR</b> The project proposes to reduce the parking stall dimensions to X-ft wide by XX-ft long, instead of the required XX-ft by XX-ft stall. <b>OR</b> The project proposes to reduce the number of parking stalls proposed to XX, instead of the required XX, resulting in an approximate XX% reduction in impervious. <b>OR</b> Reducing the parking stall dimensions or number of stalls is not feasible for the intended use. <b>OR</b> No new parking stalls are proposed as part of this project.

Reduction of Impervious Cover

## Step 2 - Calculate Water Quality Volume

Is this project subject to Section 4.3 of the NYS Design Manual for Enhanced Phosphorus

Removal?

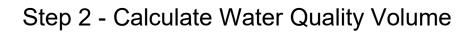
What is the nature of this construction project?

New Construction

Design Point: Gen 5&6

Enter 90% Rainfall Event as P

P=						
	Calculate Required WQv					
Drainage Area Number	Contributing Area (Acres)	Impervious Area (Acres)	Percent Impervious %	Rv	WQv (cf)	SMP Description
1	0.90	0.25	28	0.30	980	Filtration Bioretention
2	2.56	0.97	38	0.39	3,634	Sheet Flow to Grass Filter Strip
3	0.10	0.06	60	0.59	214	Dry Swale
4	0.71	0.18	25	0.28	717	Filtration Bioretention
5						
6						
7						
8						
9						
10						
11						
12						
13						
14						
15						
16						
17						
18						
19						
20						
21						
22						
23						
24						
25						
26						
27						
28						
29						
30						
Total	4.27	1.46	34	0.36	5545	Required WQv



## Steps 3 and 5 - Apply RR Techniques and Standard SMPs

	Runoff Reduction Volume and Treated Volumes					
	Runoff Reduction Techiques/Standard SMPs		Total Contributing Area	Total Contributing Impervious Area	WQv Reduced (RRv)	WQv Treated
			(acres)	(acres)	(cf)	(cf)
	Conservation of Natural Areas	RR-1	0.00		0	
	Sheet Flow to Riparian Buffer/Filter Strip	RR-2	2.56	0.97	3,634	-
ω (	Tree Planting/Tree Pit/Tree Trench	RR-3	0.00	0.00	0	-
RR Techniques	Disconnection of Rooftop Runoff	RR-4	2.22	0.00	0	
pini	Vegetated Swale	RR-5	0.00	0.00	0	
ech	Rain Garden	RR-6	0.00	0.00	0	-
<del> </del>	Stormwater Planter	RR-7	0.00	0.00	0	-
_ ₹	Rainwater Harvesting Systems  Porous Pavement	RR-8	0.00	0.00	0	-
		RR-9 RR-10	0.00	0.00	0	-
	Green Roof (Extensive & Intensive)	RR-10	0.00	0.00	U	
	Stream Daylighting Infiltration Trench	I-1	0.00	0.00	0	0
``*	Infiltration Basin	I-1	0.00	0.00	0	0
Ps ity	Dry Well	I-3	0.00	0.00	0	0
SM	Underground Infiltration System	I-4	0.00	0.00	0	0
5 S	Infiltration Bioretention	F-4	0.00	0.00	0	0
andard SMPs RRv Capacity	Filtration Bioretention	F-5	1.61	0.43	1,697	0
Standard SMPs w/ RRv Capacity	Bioslope	F-6	0.00	0.00	0	0
-	Dry swale	O-1 P-1	0.10	0.06	43	171
	Micropool Extended Detention		0.00	0.00		0
	Wet Pond	P-2	0.00	0.00		0
	Wet Extended Detention	P-3	0.00	0.00		0
ω	Multiple Pond System	P-4	0.00	0.00		0
SMPs	Shallow Wetland	W-1	0.00	0.00		0
	Extended Detention Shallow Wetland	W-2	0.00	0.00		0
Standard	Pond/Wetland System	W-3	0.00	0.00		0
anc	Pocket Wetland	W-4	0.00	0.00		0
\ \text{S}	Gravel Wetland	W-5	0.00	0.00		0
	Surface Sand Filter	F-1	0.00	0.00		0
	Underground Sand Filter	F-2	0.00	0.00		0
	Perimeter Sand Filter	F-3	0.00	0.00		0
	Wet Swale	0-2	0.00	0.00		0
Alt. SMPs	Flow Based Alternative Practice	-	0.00	0.00		0
, <sub>R</sub>	Volume Based Alternative Practice -					
	Totals by RR Technique $ ightarrow$			0.97	3,634	
	Totals by Standard SMP w/RRV $ ightarrow$			0.49	1,740	171
	Totals by Standard SMP	$\rightarrow$	0.00	0.00		0
	Totals by Alternative SMP	<b>→</b>	0.00	0.00		0
	Totals ( RR Techniques + all SMPs)	$\rightarrow$	4.27	1.46	5,374	171

Step 3 - Evaluation of RR Techniques and Standard SMPs with RRv Capacity

	Practice	Description	Applicable	Project Specific Evaluation
RR Techniques	Conservation of Natural Areas (RR-1)	Retain the pre-development hydrologic and water quality characteristics of undisturbed natural areas by permanently conserving these areas on a site. Undisturbed natural areas include: forest retention areas; reforestation areas; stream and river corridors; shorelines; wetlands, vernal pools, and associated vegetated buffers; and undisturbed open space.		EXAMPLES - The project site does not contain any significant natural resources. <b>OR</b> As a Redevelopment Project, the proposed site layout has been designed to limit land disturbance to the greatest extent practical. <b>OR</b> Approximately XX+/-Acres will be placed into permanent conservation, which accounts for XX% of the total property. <b>OR</b> The project does not propose permanent conservation of these areas at this time.
	Sheet Flow to Riparian Buffer/Filter Strip (RR-2)	Undisturbed natural areas such as forested conservation areas and stream buffers or vegetated filter strips and riparian buffers can be used to treat and control stormwater runoff from some areas of a development project.		EXAMPLES - The project site does not contain any riparian buffers. <b>OR</b> Sheet flow to riparian buffers or grass filter strips is not proposed at this time. The project proposes treatment by RR techniques, Standard SMPs with RRv capacity, or Standard SMPs without RRv capacity. <b>OR</b> The project proposes sheet flow to riparian buffers to reduce runoff from XX. Calculations have been provided in the SWPPP. <b>OR</b> The project proposes sheet flow to grass filter strips to reduce runoff from XX. Calculations have been provided in the SWPPP.
	Tree Planting/Tree Pit/Tree Trench (RR-3)	Plant or conserve trees to reduce stormwater runoff, increase nutrient uptake, and provide bank stabilization. Trees can be used for applications such as landscaping, stormwater management practice areas, conservation areas and erosion and sediment control.		EXAMPLES - The project proposes the preservation of existing mature trees, as well as the planting of numerous trees throughout the site, in order to reduce stormwater runoff, increase nutrient uptake, and provide bank stabilization. OR The project proposes new tree plantings/tree pits/tree trenches, adjacent to impervious surfaces, Calculations have been provided in the SWPPP. OR Tree plantings/tree pits/tree trenches are proposed on site. However, credit for these trees will not be taken toward area reduction/volume reduction in the RRv calculations.
	Disconnection of Rooftop Runoff (RR-4)	Direct runoff from rooftop areas and upland overland runoff flow to designated pervious areas to reduce runoff volumes and rates.		EXAMPLES - The building roof(s) will be directed to downspouts with splash blocks and discharge over a grassed filter. Calculations have been provided in the SWPPP. <b>OR</b> The building roof(s) will be directed to downspouts with splash blocks and vegetated areas, which will promote sheet flow and filtering. However, credit for rooftop disconnect will not be taken toward an impervious area reduction in the RRv calculations. <b>OR</b> Due to the size/use/layout of the proposed building(s), rooftop disconnect is not proposed at this time.

Step 3 - Evaluation of RR Techniques and Standard SMPs with RRv Capacity

Vegetated Swale (RR-5)	The natural drainage paths, or properly designed vegetated channels, can be used instead of constructing underground storm sewers or concrete open channels to increase time of concentration, reduce the peak discharge, and provide infiltration.	EXAMPLES - Due to flat topography/steep topography/etc., vegetated swales are not feasible for use on the project site. <b>OR</b> Vegetated swale(s) will be used to treat XX. Calculations have been provided in the SWPPP.
Rain Garden (RR-6)	Manage and treat small volumes of stormwater runoff using a conditioned planting soil bed and planting materials to filter runoff stored within a shallow depression.	EXAMPLES - Due to the size of contributing area/tributary driveway/tributary roadway, a bioretention facility will be implemented instead of rain gardens. <b>OR</b> Due to XXX,rain gardens are not feasible for use on the project site. <b>OR</b> Rain gardens will be used to treat roof runoff at the source for the building(s). Calculations have been provided in the SWPPP.
Stormwater Planter (RR-7)	Small landscaped stormwater treatment devices that can be designed as infiltration or filtering practices. Stormwater planters use soil infiltration and biogeochemical processes to decrease stormwater quantity and improve water quality.	EXAMPLES - Due to XXX, a bioretention facility will be implemented instead of stormwater planter(s). <b>OR</b> Due to XXX,stormwater planters are not feasible for use on the project site. <b>OR</b> Stormwater planters will be used to treat roof runoff at the source for the building(s). Calculations have been provided in the SWPPP.
Rainwater Harvesting System (RR-8)	Capture and store stormwater runoff to be used for irrigation systems or filtered and reused for non-contact activities.	EXAMPLES - Rainwater harvesting is not proposed on-site due to XXX. <b>OR</b> A below-grade cistern will be implemented for reuse as XXX. The system has been sized to provide adequate storage capacity for the entire WQv & RRv calculated for the tributary area. Calculations have been provided in the SWPPP. An above-grade rain barrel/cistern will be implemented for reuse as XXX. The system has been sized to provide adequate storage capacity for the entire WQv & RRv calculated for the tributary area. Calculations have been provided in the SWPPP.
Porous Pavement (RR-9)	Pervious types of pavements that provide an alternative to conventional paved surfaces, designed to infiltrate rainfall through the surface, thereby reducing stormwater runoff from a site and providing some pollutant uptake in the underlying soils.	EXAMPLES - Due to XXX, porous pavement is not feasible for use on the project site. <b>OR</b> Porous asphalt will be used, in place of XX, for the proposed XX. Calculations have been provided in the SWPPP. <b>OR</b> Porous concrete will be used, in place of XX, for the proposed XX. Calculations have been provided in the SWPPP. <b>OR</b> Porous pavers will be used, in place of XX, for the proposed XX. Calculations have been provided in the SWPPP.

RR Techniques

Step 3 - Evaluation of RR Techniques and Standard SMPs with RRv Capacity

	Green Roof (RR-10)	Capture runoff by a layer of vegetation and soil installed on top of a conventional flat or sloped roof. The rooftop vegetation allows evaporation and evapotranspiration processes to reduce volume and discharge rate of runoff entering conveyance system.	f∈	EXAMPLES - Due to XX, a green roof is not feasible for use on the project site. <b>OR</b> An extensive/intensive green roof is proposed on-site. Calculations have been provided in the SWPPP.
	Stream Daylighting (RR-11)	Stream Daylight previously- culverted/piped streams to restore natural habitats, better attenuate runoff by increasing the storage size, promoting infiltration, and help reduce pollutant loads.	a   c   s   T	EXAMPLES - No stream daylighting opportunities are present on the site. <b>OR</b> Although stream daylighting opportunities are present, due to XX, stream daylighting is not proposed on this site. <b>OR</b> The project proposes stream daylighting for an XX-t length of stream.
	Infiltration Trench (I-1)	An infiltration practice that stores the water quality volume in the void spaces of a gravel trench before it is infiltrated into the ground.	r    	EXAMPLES - Due to XX, infiltration trenches are not feasible for use on the project site. <b>OR</b> infiltration trench(es) are proposed on-site to treat XX. Calculations have been provided in the SWPPP.
	Infiltration Basin (I-2)	An infiltration practice that stores the water quality volume in a shallow depression, before it is infiltrated it into the ground.	fo ii	EXAMPLES - Due to XX, infiltration basins are not feasible for use on the project site. <b>OR</b> An infiltration basin(s) is proposed on-site to treat XX. Calculations have been provided in the SWPPP.
	Dry Well (I-3)	An infiltration practice similar in design to the infiltration trench, and best suited for treatment of rooftop runoff.	f <sub>0</sub>	EXAMPLES - Due to XX, dry wells are not feasible for use on the project site. <b>OR</b> Dry well(s) are proposed on-site to treat XX. Calculations have been provided in the SWPPP.
RV Capacity	Underground Infiltration System (I-4)	An infiltration practice below grade that stores the water quality volume in premanufactured pipes, vaults or other modular structures, before it is infiltrated into the ground.	s (	EXAMPLES - Due to XX, underground infiltration systems are not feasible for use on the project site.  OR An underground infiltration system(s) is proposed on-site to treat XX. Calculations have been provided in the SWPPP.
Standard SMPs with RRv	Infiltration Bioretention (F- 4)	A shallow depression that treats stormwater as it flows through a soil matrix, before it is infiltrated into the ground.	v b fi b	EXAMPLES - Due to XXX, a filtration bioretention will be implemented, instead of an infiltration bioretention. <b>OR</b> Due to XXX, bioretention is not leasible for use on the project site. <b>OR</b> Infiltration bioretention will be used to treat XX. Calculations have been provided in the SWPPP.
	Filtration Bioretention (F- 5)	A shallow depression that treats stormwater as it flows through a soil matrix and is returned to the storm drain system.	t f is F	EXAMPLES - Due to XXX, an infiltration bioretention will be implemented, instead of a litration bioretention. <b>OR</b> Due to XXX, bioretention is not feasible for use on the project site. <b>OR</b> Filtration bioretention will be used to treat XX. Calculations have been provided in the SWPPP.

## Step 3 - Evaluation of RR Techniques and Standard SMPs with RRv Capacity

Bioslope (F-6)	Permeable engineered soil media that is installed along embankments or other slopes, designed to capture and treat stormwater runoff from adjacent paved areas.	EXAMPLES - Due to XXX, bioslopes are not feasible for use on the project site. <b>OR</b> Bioslope(s) will be used to treat XX. Calculations have been provided in the SWPPP.
Dry Swale (O- 1)	An open drainage channel or depression explicitly designed to detain and promote the filtration of stormwater runoff into the soil media.	EXAMPLES - Due to XXX, dry swales are not feasible for use on the project site. <b>OR</b> A dry swale(s) will be used to treat XX. Calculations have been provided in the SWPPP.

## Step 4 - Calcuate Minimum RRv Required

Enter the Soils Da	Enter the Soils Data for the site			
Hydrologic Soil Group	Acres	S		
Α	0.00	55%		
В	0.00	40%		
С	0.00	30%		
D	1.46	20%		
Total Area	1.46			

Calculate the Minimum RRv					
S =	0.20				
Impervious =	1.46	acres			
Precipitation	1.00	inches			
Rv	0.95				
Minimum RRv	0.023	af			
	1002	cf			

# Sheet Flow to Grass Filter Strip (RR-3)

Design Point:	Gen 5&6							
		Enter Site D	ata For Drain	age Area to b	e Reduc	ed		
Drainage Area Number	Contributing Area (Acres)	Impervious Area (Acres)	Percent Impervious %	Rv	WQv (cf)	Precipitation (in)	Description	
2	2.56	0.97	38	0.39	3,634	1.00	Sheet Flow to Grass Filter Strip	
			Design (					
Is the riparian buffer delineated and permanently protected through establishment of a legal conservation easement?								
Is the contributin	ng area a desigr	nated hotspot?	?			No		
Is a pretreatmenthe buffer?	nt pea gravel dia	phragm propo	osed along the	e upgradient e	dge of	Yes		
Is runoff entering upgradient of the	•	overland shee	t flow or a flow	v spreader pro	posed	Yes		
Enter the total le	ength of contribu	iting flow path	(ft)			20		
Enter the length	of contributing	flow path from	ı impervious s	urfaces (ft)		20		
Enter the slope of	of contributing f	low path (%)				2		
Minimum buffer length based on contriuting flow path slope (ft) 35								
Enter the slope f	for the first 10 ft	of the buffer	(%)			2		
			Sizing (	Criteria				
				Value	Э	Units	Notes	
Enter Trav	el Time through	n Buffer	Т	6		min		
Enter 2-y	r 24-hr Rainfall	Depth	Р	2.17	•	inch		
Enter C	Overall Buffer Sl	ope	S	0.02	!	ft/ft		
Enter Mannir	ng's Coefficient	for Buffer	n	0.24				
Calculated M	/linimum Length	of Buffer	L	27		ft		
Minimu	ım Length of Bu	ıffer	L	35		ft		
Is the buffer within HSG C or D soils?								
Require	Required Length of Buffer L 40 ft							
Enter Provided Length of Buffer L 40 ft								
				off Reduction	1			
	<b>RRv Provided</b>	3,634	cf					

# Sheet Flow to Grass Filter Strip (RR-3)

Total Area	2.56	acres
Total Impervious	1	acres
Total RRv Provided	3,634	cf

# Filtration Bioretention (F-5)

Design Point:	Gen 5&6							
Enter Site Data For Drainage Area to be Treated by Practice								
Drainage Area Number	Contributing Area (Acres)	Impervious Area (Acres)	Percent Impervious %	Rv	WQv (cf)	Precipitation (in)	Description	
1	0.90	0.25	28	0.30	980	1.00	Filtration Bioretention	
			Design Cri	teria				
Enter underlying geotechnical tes	g soil infiltration r sting, refer to Ap		0.2	Underd	rains requi	red		
Is the contributir stormwater hots	pot?		No					
Is the practice the of a Level 1 (Infi	Itration Restricte	ed) hotspot?	No					
contributing area			No					
Enter depth to s		ter table (ft)	5					
Enter depth to b	` '		10					
Is pretreatment provided, in conformance with Section 6.4.3.1			Yes					
Enter average height of ponding (ft)			0.5					
Enter depth of surface layer (inches)			6					
Enter depth of fi	` ' /		2.5					
Enter depth of d	<u> </u>		12					
Enter slope of m		` '	10					
Enter width of m	aintenance acce	ess (ft)	20					
			Sizing Crit	teria				
				V	'alue	Units	Notes	
Perr	neability Flow Ra	ate	k		1	ft/day		
	Filter Time		tf		2	days		
Re	quired Filter Are	 a	Af	4	408	sf		
Enter Provided Filter Area			Af	1	844	sf		
Recalculated Water Quality Volume (based on provided filter area)			WQv calc	44	125.6	cf		
		Cald	culate Runoff	Reduct	ion			
RRv Provided		980	cf					
WQv Treated 0			cf	This is t	•	of the WQv that	is not reduced in	

# Filtration Bioretention (F-5)

Design Point:	Gen 5&6						
	Enter	Site Data For	Drainage Are	a to be	Treated by	Practice	
Drainage Area Number	Contributing Area (Acres)	Impervious Area (Acres)	Percent Impervious %	Rv	WQv (cf)	Precipitation (in)	Description
4	0.71	0.18	25	0.28	717	1.00	Filtration Bioretention
			Design Cri	teria			
Enter underlying geotechnical tes			0.2	Underd	rains requir	red	
Is the contributin stormwater hots	pot?		No				
Is the practice th of a Level 1 (Infil			No				
Is contributing area greater than max. contributing area?			No				
Enter depth to se	easonal high wa	ter table (ft)	5				
Enter depth to be	edrock (ft)		10				
Is pretreatment provided, in conformance with Section 6.4.3.1			Yes				
Enter average he			0.5				
Enter depth of su		hes)	6				
Enter depth of fil	. ,		2.5				
Enter depth of di			12				
Enter slope of m		, ,	10				
Enter width of m	aintenance acce	ess (ft)	20				
			Sizing Crit				
				V	alue	Units	Notes
Perm	neability Flow Ra	ate	k		1	ft/day	
	Filter Time		tf		2	days	
Red	quired Filter Are	a	Af		299	sf	
Enter	Provided Filter A	∖rea	Af	9	961	sf	
Recalculated Water Quality Volume (based on provided filter area)			WQv calc	23	306.4	cf	
		Cald	ulate Runoff	Reduct	ion		
RRv Provided		717	cf				
WQv Treated		0	cf	This is t	•	of the WQv that	is not reduced in

# Filtration Bioretention (F-5)

Total Area	1.61	acres
Total Impervious	0	acres
Total RRv Provided	1,697	cf
Total WQv Provided	0	cf

## Dry Swale (O-1)

Design Point:	Gen 5&6						
Enter Site Data For Drainage Area to be Treated by Practice							
Drainage Area Number	Contributing Area (Acres)	Impervious Area (Acres)	Percent Impervious %	Rv	<b>WQv</b> (ft <sup>3</sup> )	Precipitation (in)	Description
3	0.10	0.06	60	0.59	214	1.00	Dry Swale
			Design Crite	eria			
Select HSG			D				
Is the contributing designated hotsp	g area to the pract oot?	ice a	No				
Level 1 (Infiltration	e first in series for on Restricted) hots	pot?	No				
	ea greater than 5 a		No				
	easonal high water	table (ft)	5				
Enter depth to be		1.(.6)	10				
	ent volume provide	a (ct)	200 30				
	ter media (inches) ainage layer (inche	201					
Is an underdrain	<u> </u>	<del>2</del> 8)	12 Yes				
			Sizing Crite				
		•	Value	Units		Notes	
	tom Width	b	2.00	ft			
	de Slopes ım Flow Depth	X:1 d	3.00 0.50	:1 ft			
	Top Width	Wwqv	5.00	ft			
	r op widti	Awqv	1.75	sf			
	annel Length	Lr	123.00	ft			
	Channel Length	Lp	166.00	ft			
	ume Provided	Vc	290.50	cf			
•	Dam Height	Ch	0.50	ft			
	m Spacing	Cs		ft			
	k Dams Required	C	13.00				
	Flow Depth	d2	2.17	ft			
	ow Top Width	W2	15.02	ft			
	r 24-hr Flow	A2	18.47	sf			
	d Perimeter	Pw2	24.44	ft			
	nings Coef.	n	0.20				
	tudinal Slope	S	0.04	ft/ft			
2-yr V	elocity/	V	1.24	fps			
Enter 10-y	r Freeboard		0.50	ft			
		Deterr	mine Runoff	Reducti	on		
RRv Provided			43	cf			
WQv Treated			171	cf	This is the reduced/i	e portion of the W nfiltrated	Qv that is not

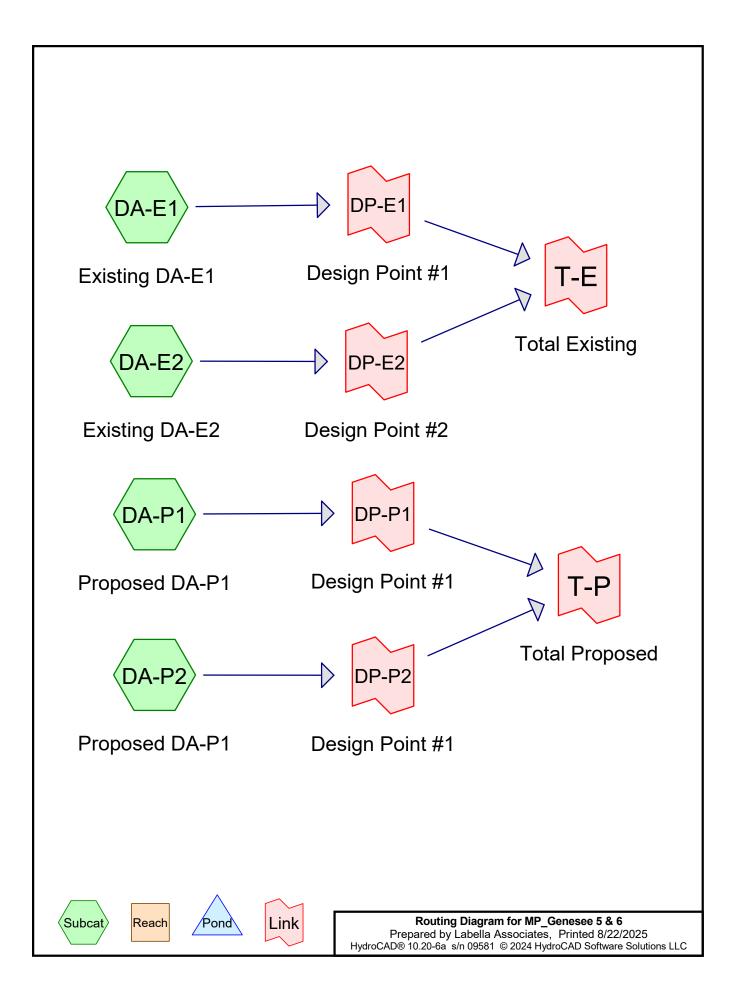
## Dry Swale (O-1)

Total Area	0.10	acres
Total Impervious	0	acres
Total RRv Provided	43	cf
Total WQv Provided	171	cf



# APPENDIX D: PRE- & POST-CONSTRUCTION STORMWATER MODELING





#### MP\_Genesee 5 & 6

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#### **Rainfall Events Listing**

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC	P2
	Name				(hours)		(inches)		(inches)
1	1-YR	Type II 24-hr		Default	24.00	1	1.86	2	2.17
2	10-YR	Type II 24-hr		Default	24.00	1	3.06	2	2.17
3	10-0YR	Type II 24-hr		Default	24.00	1	4.99	2	2.17

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#### **Area Listing (all nodes)**

Area	CN	Description
(acres)		(subcatchment-numbers)
11.347	98	Existing Impervious (DA-E1, DA-E2, DA-P1, DA-P2)
0.200	58	Meadow, non-grazed, HSG B (DA-E1, DA-P1)
341.660	78	Meadow, non-grazed, HSG D (DA-E1, DA-E2, DA-P1, DA-P2)
1.040	98	Proposed Impverious (DA-P2)
0.420	98	Propsoed Impervious (DA-P1)
2.580	98	Water Surface, HSG D (DA-E1, DA-E2, DA-P1, DA-P2)
66.293	77	Woods, Good, HSG D (DA-E1, DA-E2, DA-P1, DA-P2)
423.540	79	TOTAL AREA

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#### Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.200	HSG B	DA-E1, DA-P1
0.000	HSG C	
410.533	HSG D	DA-E1, DA-E2, DA-P1, DA-P2
12.807	Other	DA-E1, DA-E2, DA-P1, DA-P2
423.540		TOTAL AREA

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#### **Ground Covers (all nodes)**

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
0.000	0.000	0.000	0.000	11.347	11.347	Existing Impervious	DA-E1,
							DA-E2,
							DA-P1,
							DA-P2
0.000	0.200	0.000	341.660	0.000	341.860	Meadow, non-grazed	DA-E1,
							DA-E2,
							DA-P1,
							DA-P2
0.000	0.000	0.000	0.000	1.040	1.040	Proposed Impverious	DA-P2
0.000	0.000	0.000	0.000	0.420	0.420	Propsoed Impervious	DA-P1
0.000	0.000	0.000	2.580	0.000	2.580	Water Surface	DA-E1,
							DA-E2,
							DA-P1,
							DA-P2
0.000	0.000	0.000	66.293	0.000	66.293	Woods, Good	DA-E1,
							DA-E2,
							DA-P1,
							DA-P2
0.000	0.200	0.000	410.533	12.807	423.540	TOTAL AREA	

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Time span=5.00-36.00 hrs, dt=0.05 hrs, 621 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment DA-E1: Existing DA-E1 Runoff Area=92.840 ac 4.35% Impervious Runoff Depth=0.44" Flow Length=3,681' Tc=40.6 min CN=79 Runoff=24.22 cfs 3.424 af

Subcatchment DA-E2: Existing DA-E2

Runoff Area=118.930 ac 2.71% Impervious Runoff Depth=0.41"

Flow Length=1,836' Tc=18.3 min CN=78 Runoff=48.02 cfs 4.043 af

**Subcatchment DA-P1: Proposed DA-P1** Runoff Area=92.840 ac 4.36% Impervious Runoff Depth=0.44" Flow Length=3,681' Tc=40.6 min CN=79 Runoff=24.22 cfs 3.424 af

Subcatchment DA-P2: Proposed DA-P1 Runoff Area=118.930 ac 3.43% Impervious Runoff Depth=0.41" Flow Length=1,836' Tc=18.3 min CN=78 Runoff=48.02 cfs 4.043 af

Link DP-E1: Design Point #1 Inflow=24.22 cfs 3.424 af Primary=24.22 cfs 3.424 af

Link DP-E2: Design Point #2 Inflow=48.02 cfs 4.043 af

Primary=48.02 cfs 4.043 af

Link DP-P1: Design Point #1 Inflow=24.22 cfs 3.424 af Primary=24.22 cfs 3.424 af

Link DP-P2: Design Point #1 Inflow=48.02 cfs 4.043 af Primary=48.02 cfs 4.043 af

Link T-E: Total Existing

Inflow=59.66 cfs 7.468 af
Primary=59.66 cfs 7.468 af

Link T-P: Total Proposed Inflow=59.66 cfs 7.468 af Primary=59.66 cfs 7.468 af

Total Runoff Area = 423.540 ac Runoff Volume = 14.935 af Average Runoff Depth = 0.42" 96.37% Pervious = 408.153 ac 3.63% Impervious = 15.387 ac Prepared by Labella Associates

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#### Summary for Subcatchment DA-E1: Existing DA-E1

Runoff = 24.22 cfs @ 12.42 hrs, Volume= 3.424 af, Depth= 0.44"

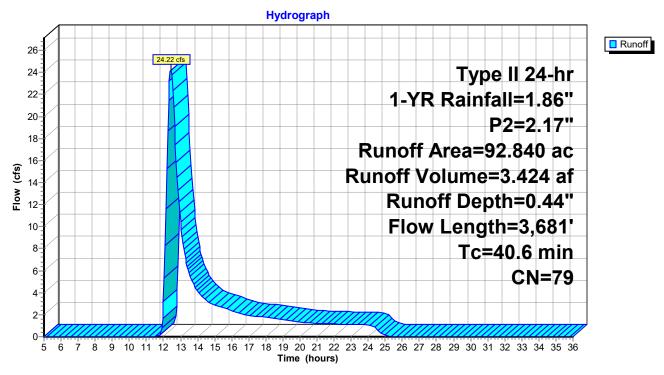
Routed to Link DP-E1 : Design Point #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs Type II 24-hr 1-YR Rainfall=1.86", P2=2.17"

	Area	(ac)	CN	Desc	cription		
	5.	520	77	Woo	ds, Good,	HSG D	
*		710	98		ting Imper		
	_	330	98		er Surface		
		180	78			grazed, HS	G D
		100	58			grazed, HS	
_		840	79		ghted Aver	· · · · · · · · · · · · · · · · · · ·	<del> </del>
		800	7.5	•	5% Pervio	•	
		040			% Impervi		
	٦.	0+0		7.00	70 IIIIpei vii	ous Alea	
	Тс	Length		Slope	Velocity	Capacity	Description
	(min)	(feet		(ft/ft)	(ft/sec)	(cfs)	Becomplient
	3.0	33		1818	0.19	(===)	Sheet Flow,
	0.0	00	<i>)</i> 0.	1010	0.10		Grass: Dense n= 0.240 P2= 2.17"
	9.8	67	7 0	0373	0.11		Sheet Flow,
	0.0	O i	0.	0070	0.11		Grass: Dense n= 0.240 P2= 2.17"
	23.6	2,596	3 0	0130	1.84		Shallow Concentrated Flow,
	20.0	2,000	0.	0100	1.04		Unpaved Kv= 16.1 fps
	4.2	985	5 0	0160	3.93	29.44	Channel Flow,
	7.2	300	0.	0100	0.00	23.44	Area= 7.5 sf Perim= 12.0' r= 0.63'
							n= 0.035 Earth, dense weeds
_	40.6	2 604	т.				11- 0.000 Lattif, defise weeds
	40.6	3,68	1 10	otal			

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#### **Subcatchment DA-E1: Existing DA-E1**



Runoff

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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#### Hydrograph for Subcatchment DA-E1: Existing DA-E1

(hours) (inches) (inches)

1.86

1.86

1.86

1.86

1.86

1.86

1.86

1.86

1.86

1.86

1.86

Time

31.00

31.50

32.00

32.50

33.00

33.50

34.00

34.50

35.00

35.50

36.00

Precip. Excess

0.44

0.44

0.44

0.44

0.44

0.44

0.44

0.44

0.44

0.44

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.12	0.00	0.00
5.50	0.13	0.00	0.00
6.00	0.15	0.00	0.00
6.50	0.17	0.00	0.00
7.00	0.18 0.20	0.00	0.00 0.00
7.50 8.00	0.20	0.00	0.00
8.50	0.25	0.00	0.00
9.00	0.27	0.00	0.00
9.50	0.30	0.00	0.00
10.00	0.34	0.00	0.00
10.50	0.38	0.00	0.00
11.00 11.50	0.44 0.53	0.00	0.00
12.00	1.23	0.00	0.00 <b>3.19</b>
12.50	1.37	0.20	23.53
13.00	1.44	0.23	10.45
13.50	1.49	0.25	5.84
14.00	1.53	0.27	4.17
14.50	1.56	0.29	3.29
15.00 15.50	1.59 1.61	0.30 0.31	2.89 2.61
16.00	1.64	0.31	2.35
16.50	1.66	0.34	2.10
17.00	1.68	0.35	1.95
17.50	1.70	0.35	1.85
18.00	1.71	0.36	1.76
18.50 19.00	1.73 1.74	0.37 0.38	1.66 1.56
19.50	1.74	0.38	1.45
20.00	1.77	0.39	1.35
20.50	1.78	0.40	1.25
21.00	1.79	0.41	1.20
21.50	1.81	0.41	1.18
22.00	1.82	0.42	1.16
22.50 23.00	1.83 1.84	0.43 0.43	1.15 1.13
23.50	1.85	0.43	1.11
24.00	1.86	0.44	1.09
24.50	1.86	0.44	0.67
25.00	1.86	0.44	0.11
25.50	1.86	0.44	0.02
26.00 26.50	1.86 1.86	0.44 0.44	0.00
27.00	1.86	0.44	0.00
27.50	1.86	0.44	0.00
28.00	1.86	0.44	0.00
28.50	1.86	0.44	0.00
29.00	1.86	0.44	0.00
29.50 30.00	1.86 1.86	0.44 0.44	0.00
30.50	1.86	0.44	0.00

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#### Summary for Subcatchment DA-E2: Existing DA-E2

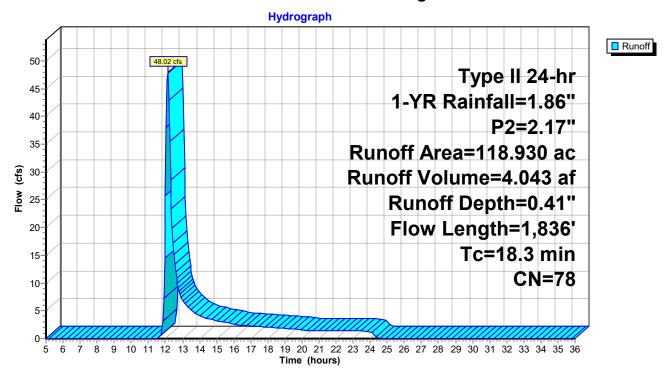
Runoff = 48.02 cfs @ 12.14 hrs, Volume= 4.043 af, Depth= 0.41"

Routed to Link DP-E2: Design Point #2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs Type II 24-hr 1-YR Rainfall=1.86", P2=2.17"

	Area	(ac)	CN	Desc	cription		
	29.	160	77	Woo	ds, Good,	HSG D	
*	2.	260	98	Exist	ting Imper	vious	
	86.	550	78	Mea	dow, non-g	grazed, HS	G D
*	0.	960	98	Wate	er Surface	, HSG D	
	118.	930	78	Weig	ghted Aver	age	
	115.	710		97.2	9% Pervio	us Area	
	3.	220		2.71	% Impervi	ous Area	
	_			01			
	Tc	Lengt		Slope	Velocity	Capacity	Description
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	11.6	10	0 0	.0550	0.14		Sheet Flow,
							Grass: Dense n= 0.240 P2= 2.17"
	6.7	1,73	6 0	.0730	4.35		Shallow Concentrated Flow,
							Unpaved Kv= 16.1 fps
	18.3	1,83	6 T	otal			

#### **Subcatchment DA-E2: Existing DA-E2**



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#### Hydrograph for Subcatchment DA-E2: Existing DA-E2

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00 5.50	0.12 0.13	0.00	0.00 0.00
6.00	0.15	0.00	0.00
6.50	0.17	0.00	0.00
7.00	0.18	0.00	0.00
7.50	0.20	0.00	0.00
8.00	0.22	0.00	0.00
8.50 9.00	0.25 0.27	0.00	0.00 0.00
9.50	0.27	0.00	0.00
10.00	0.34	0.00	0.00
10.50	0.38	0.00	0.00
11.00	0.44	0.00	0.00
11.50	0.53	0.00	0.00
12.00 12.50	1.23 1.37	0.13 0.18	25.67 15.12
13.00	1.44	0.10	6.95
13.50	1.49	0.23	5.20
14.00	1.53	0.24	4.17
14.50	1.56	0.26	3.55
15.00	1.59	0.27	3.26
15.50 16.00	1.61 1.64	0.28 0.30	2.95 2.63
16.50	1.66	0.31	2.40
17.00	1.68	0.32	2.28
17.50	1.70	0.32	2.17
18.00	1.71	0.33	2.06
18.50 19.00	1.73 1.74	0.34 0.35	1.94 1.81
19.50	1.74	0.36	1.68
20.00	1.77	0.36	1.55
20.50	1.78	0.37	1.47
21.00	1.79	0.37	1.45
21.50	1.81	0.38	1.43
22.00 22.50	1.82 1.83	0.39 0.39	1.41 1.38
23.00	1.84	0.40	1.36
23.50	1.85	0.40	1.34
24.00	1.86	0.41	1.32
24.50	1.86	0.41	0.09
25.00 25.50	1.86 1.86	0.41 0.41	0.00 0.00
26.00	1.86	0.41	0.00
26.50	1.86	0.41	0.00
27.00	1.86	0.41	0.00
27.50	1.86	0.41	0.00
28.00	1.86	0.41 0.41	0.00
28.50 29.00	1.86 1.86	0.41	0.00 0.00
29.50	1.86	0.41	0.00
30.00	1.86	0.41	0.00
30.50	1.86	0.41	0.00

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
31.00	1.86	0.41	0.00
31.50	1.86	0.41	0.00
32.00	1.86	0.41	0.00
32.50	1.86	0.41	0.00
33.00	1.86	0.41	0.00
33.50	1.86	0.41	0.00
34.00	1.86	0.41	0.00
34.50	1.86	0.41	0.00
35.00	1.86	0.41	0.00
35.50	1.86	0.41	0.00
36.00	1.86	0.41	0.00

Prepared by Labella Associates

Printed 8/22/2025

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#### Summary for Subcatchment DA-P1: Proposed DA-P1

Runoff = 24.22 cfs @ 12.42 hrs, Volume= 3.424 af, Depth= 0.44"

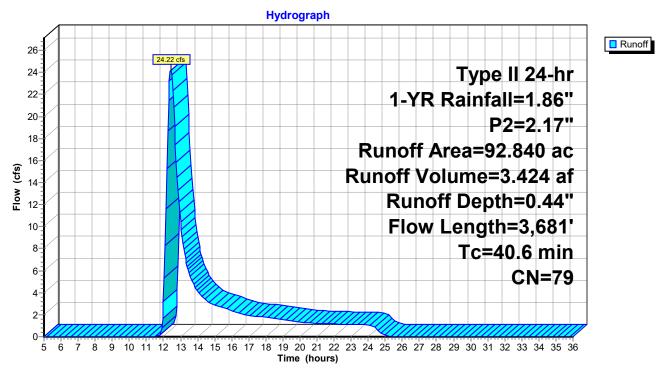
Routed to Link DP-P1 : Design Point #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs Type II 24-hr 1-YR Rainfall=1.86", P2=2.17"

	Area	(ac)	CN Des	cription					
	5.	520	77 Woo	ds, Good,	HSG D				
*	3.	297		ting Imper					
*	0.	420		soed Impe					
	0.	330		er Surface					
	83.	173			, grazed, HS	GD			
	0.	100			grazed, HS				
	92.	840	79 Wei	ghted Avei	age				
	88.	793		4% Pervio	•				
	4.	047	4.36	4.36% Impervious Area					
				•					
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	•	(ft/sec)	(cfs)	•			
_	3.0	33	0.1818	0.19	, ,	Sheet Flow,			
				• • • • • • • • • • • • • • • • • • • •		Grass: Dense n= 0.240 P2= 2.17"			
	9.8	67	0.0373	0.11		Sheet Flow,			
						Grass: Dense n= 0.240 P2= 2.17"			
	23.6	2,596	0.0130	1.84		Shallow Concentrated Flow,			
		,				Unpaved Kv= 16.1 fps			
	4.2	985	0.0160	3.93	29.44	Channel Flow,			
						Area= 7.5 sf Perim= 12.0' r= 0.63' n= 0.035			
	40.6	3,681	Total	_					

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#### Subcatchment DA-P1: Proposed DA-P1



Runoff

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

 $0.00\\0.00$ 

0.00

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#### Hydrograph for Subcatchment DA-P1: Proposed DA-P1

(hours) (inches) (inches)

1.86

1.86

1.86

1.86

1.86

1.86

1.86

1.86

1.86

1.86

1.86

Time

31.00

31.50

32.00

32.50

33.00

33.50

34.00

34.50

35.00

35.50

36.00

Precip. Excess

0.44

0.44

0.44

0.44

0.44

0.44

0.44

0.44

0.44

0.44

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00 5.50	0.12 0.13	0.00	0.00 0.00
6.00	0.15	0.00	0.00
6.50	0.17	0.00	0.00
7.00 7.50	0.18 0.20	0.00	0.00 0.00
8.00	0.20	0.00	0.00
8.50	0.25	0.00	0.00
9.00 9.50	0.27 0.30	0.00	0.00 0.00
10.00	0.34	0.00	0.00
10.50	0.38	0.00	0.00
11.00 11.50	0.44 0.53	0.00	0.00 0.00
12.00	1.23	0.00	3.19
12.50	1.37	0.20	23.53
13.00 13.50	1.44 1.49	0.23 0.25	10.45 5.84
14.00	1.53	0.27	4.17
14.50	1.56	0.29	3.29
15.00 15.50	1.59 1.61	0.30 0.31	2.89 2.61
16.00	1.64	0.32	2.35
16.50 17.00	1.66 1.68	0.34 0.35	2.10 1.95
17.50	1.70	0.35	1.85
18.00	1.71	0.36	1.76
18.50 19.00	1.73 1.74	0.37 0.38	1.66 1.56
19.50	1.74	0.39	1.45
20.00	1.77	0.39	1.35
20.50 21.00	1.78 1.79	0.40 0.41	1.25 1.20
21.50	1.81	0.41	1.18
22.00	1.82	0.42	1.16
22.50 23.00	1.83 1.84	0.43 0.43	1.15 1.13
23.50	1.85	0.44	1.11
24.00 24.50	1.86	0.44	1.09 0.67
25.00	1.86 1.86	0.44 0.44	0.07
25.50	1.86	0.44	0.02
26.00 26.50	1.86 1.86	0.44 0.44	0.00
27.00	1.86	0.44	0.00
27.50	1.86	0.44	0.00
28.00 28.50	1.86 1.86	0.44 0.44	0.00
29.00	1.86	0.44	0.00
29.50	1.86	0.44	0.00
30.00 30.50	1.86 1.86	0.44 0.44	0.00 0.00

#### Summary for Subcatchment DA-P2: Proposed DA-P1

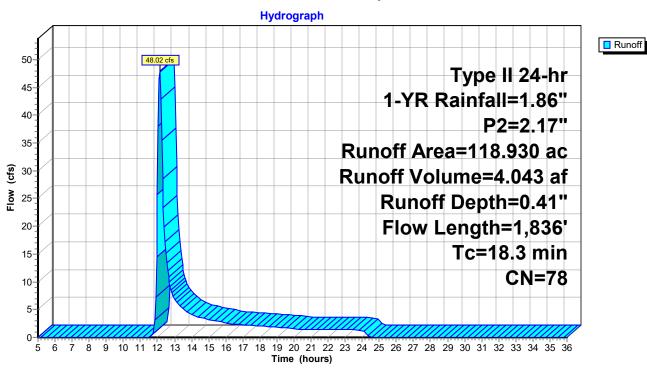
Runoff = 48.02 cfs @ 12.14 hrs, Volume= 4.043 af, Depth= 0.41"

Routed to Link DP-P2 : Design Point #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs Type II 24-hr 1-YR Rainfall=1.86", P2=2.17"

	Area	(ac)	CN	Desc	cription		
	26.	093	77	Woo	ds, Good,	HSG D	
*	2.	080	98	Exist	ting Imper	vious	
*	1.	040	98	Prop	osed Impy	erious/	
*	0.	960	98	Wate	er Surface	, HSG D	
	88.	757	78	Mea	dow, non-ເ	grazed, HS	G D
	118.	930	78	Weig	hted Aver	age	
	114.	850		96.5	7% Pervio	us Area	
	4.	080		3.43	% Impervi	ous Area	
					-		
	Тс	Lengt	:h	Slope	Velocity	Capacity	Description
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	11.6	10	0 (	0.0550	0.14		Sheet Flow,
							Grass: Dense n= 0.240 P2= 2.17"
	6.7	1,73	6	0.0730	4.35		Shallow Concentrated Flow,
		-					Unpaved Kv= 16.1 fps
	18.3	1,83	6	Total			

#### Subcatchment DA-P2: Proposed DA-P1



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#### Hydrograph for Subcatchment DA-P2: Proposed DA-P1

Time	Precip.	Excess	Runoff
(hours) 5.00	(inches) 0.12	(inches) 0.00	(cfs) 0.00
5.50	0.13	0.00	0.00
6.00 6.50	0.15 0.17	0.00	0.00 0.00
7.00	0.18	0.00	0.00
7.50 8.00	0.20 0.22	0.00	0.00 0.00
8.50	0.25	0.00	0.00
9.00	0.27	0.00	0.00
9.50 10.00	0.30 0.34	0.00	0.00
10.50	0.38	0.00	0.00
11.00 11.50	0.44 0.53	0.00	0.00 0.00
12.00	1.23	0.13	25.67
12.50 13.00	1.37 1.44	0.18 0.21	<b>15.12</b> 6.95
13.50	1.49	0.23	5.20
14.00 14.50	1.53 1.56	0.24 0.26	4.17 3.55
15.00	1.59	0.20	3.26
15.50	1.61	0.28	2.95
16.00 16.50	1.64 1.66	0.30 0.31	2.63 2.40
17.00	1.68	0.32	2.28
17.50 18.00	1.70 1.71	0.32 0.33	2.17 2.06
18.50	1.73	0.34	1.94
19.00 19.50	1.74 1.76	0.35 0.36	1.81 1.68
20.00	1.77	0.36	1.55
20.50 21.00	1.78 1.79	0.37 0.37	1.47 1.45
21.50	1.79	0.38	1.43
22.00	1.82	0.39 0.39	1.41
22.50 23.00	1.83 1.84	0.39	1.38 1.36
23.50	1.85	0.40	1.34
24.00 24.50	<b>1.86</b> 1.86	<b>0.41</b> 0.41	1.32 0.09
25.00	1.86	0.41	0.00
25.50 26.00	1.86 1.86	0.41 0.41	0.00 0.00
26.50	1.86	0.41	0.00
27.00 27.50	1.86 1.86	0.41 0.41	0.00 0.00
28.00	1.86	0.41	0.00
28.50 29.00	1.86 1.86	0.41 0.41	0.00 0.00
29.50	1.86	0.41	0.00
30.00 30.50	1.86	0.41 0.41	0.00 0.00
30.50	1.86	0.41	0.00

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
31.00	1.86	0.41	0.00
31.50	1.86	0.41	0.00
32.00	1.86	0.41	0.00
32.50	1.86	0.41	0.00
33.00	1.86	0.41	0.00
33.50	1.86	0.41	0.00
34.00	1.86	0.41	0.00
34.50	1.86	0.41	0.00
35.00	1.86	0.41	0.00
35.50	1.86	0.41	0.00
36.00	1.86	0.41	0.00

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#### Summary for Link DP-E1: Design Point #1

Inflow Area = 92.840 ac, 4.35% Impervious, Inflow Depth = 0.44" for 1-YR event

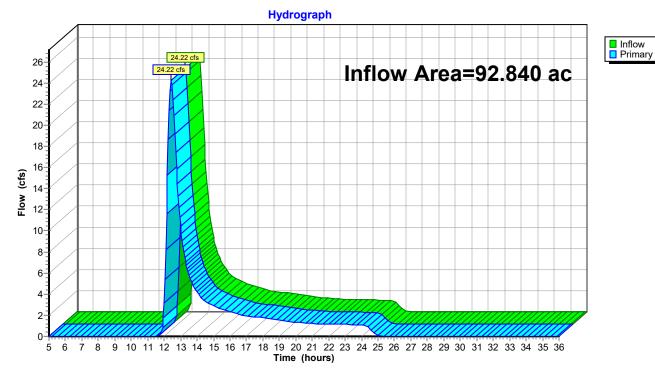
Inflow = 24.22 cfs @ 12.42 hrs, Volume= 3.424 af

Primary = 24.22 cfs @ 12.42 hrs, Volume= 3.424 af, Atten= 0%, Lag= 0.0 min

Routed to Link T-E: Total Existing

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

#### Link DP-E1: Design Point #1



Primary

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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#### Hydrograph for Link DP-E1: Design Point #1

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

			_
Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00	0.00	0.00	0.00
5.50	0.00	0.00	0.00
6.00	0.00	0.00	0.00
6.50	0.00	0.00	0.00
7.00	0.00	0.00	0.00
7.50 8.00	0.00	0.00 0.00	0.00
8.50	0.00	0.00	0.00
9.00	0.00	0.00	0.00
9.50	0.00	0.00	0.00
10.00	0.00	0.00	0.00
10.50	0.00	0.00	0.00
11.00	0.00	0.00	0.00
11.50	0.00	0.00	0.00
12.00	3.19	0.00	3.19
12.50	23.53	0.00	23.53
13.00	10.45	0.00	10.45
13.50	5.84	0.00	5.84
14.00	4.17	0.00	4.17
14.50	3.29	0.00	3.29
15.00 15.50	2.89 2.61	0.00 0.00	2.89 2.61
16.00	2.35	0.00	2.35
16.50	2.10	0.00	2.10
17.00	1.95	0.00	1.95
17.50	1.85	0.00	1.85
18.00	1.76	0.00	1.76
18.50	1.66	0.00	1.66
19.00	1.56	0.00	1.56
19.50	1.45	0.00	1.45
20.00	1.35	0.00	1.35
20.50	1.25	0.00	1.25
21.00	1.20	0.00	1.20
21.50	1.18	0.00	1.18
22.00 22.50	1.16 1.15	0.00 0.00	1.16 1.15
23.00	1.13	0.00	1.13
23.50	1.13	0.00	1.11
24.00	1.09	0.00	1.09
24.50	0.67	0.00	0.67
25.00	0.11	0.00	0.11
25.50	0.02	0.00	0.02
26.00	0.00	0.00	0.00
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50	0.00	0.00	0.00
28.00	0.00	0.00	0.00
28.50	0.00	0.00	0.00
29.00 29.50	0.00 0.00	0.00 0.00	0.00
30.00	0.00	0.00	0.00
30.50	0.00	0.00	0.00
22.30	3.00	3.00	3.33
			· ·

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#### Summary for Link DP-E2: Design Point #2

Inflow Area = 118.930 ac, 2.71% Impervious, Inflow Depth = 0.41" for 1-YR event

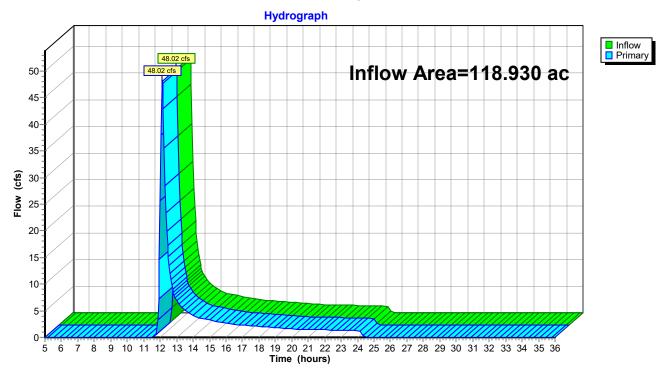
Inflow = 48.02 cfs @ 12.14 hrs, Volume= 4.043 af

Primary = 48.02 cfs @ 12.14 hrs, Volume= 4.043 af, Atten= 0%, Lag= 0.0 min

Routed to Link T-E: Total Existing

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

#### Link DP-E2: Design Point #2



Primary

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

 $0.00 \\ 0.00$ 

0.00

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#### Hydrograph for Link DP-E2: Design Point #2

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00	0.00	0.00	0.00
5.50	0.00	0.00	0.00
6.00 6.50	0.00	0.00 0.00	0.00 0.00
7.00	0.00	0.00	0.00
7.50	0.00	0.00	0.00
8.00	0.00	0.00	0.00
8.50	0.00	0.00	0.00
9.00 9.50	0.00	0.00 0.00	0.00 0.00
10.00	0.00	0.00	0.00
10.50	0.00	0.00	0.00
11.00	0.00	0.00	0.00
11.50	0.00	0.00	0.00
12.00	25.67 15.12	0.00	25.67
12.50 13.00	6.95	0.00 0.00	<b>15.12</b> 6.95
13.50	5.20	0.00	5.20
14.00	4.17	0.00	4.17
14.50	3.55	0.00	3.55
15.00	3.26	0.00	3.26
15.50 16.00	2.95 2.63	0.00 0.00	2.95 2.63
16.50	2.03	0.00	2.40
17.00	2.28	0.00	2.28
17.50	2.17	0.00	2.17
18.00	2.06	0.00	2.06
18.50	1.94 1.81	0.00	1.94
19.00 19.50	1.68	0.00 0.00	1.81 1.68
20.00	1.55	0.00	1.55
20.50	1.47	0.00	1.47
21.00	1.45	0.00	1.45
21.50	1.43	0.00	1.43
22.00 22.50	1.41 1.38	0.00 0.00	1.41 1.38
23.00	1.36	0.00	1.36
23.50	1.34	0.00	1.34
24.00	1.32	0.00	1.32
24.50	0.09	0.00	0.09
25.00 25.50	0.00	0.00 0.00	0.00 0.00
26.00	0.00	0.00	0.00
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50	0.00	0.00	0.00
28.00 28.50	0.00	0.00 0.00	0.00 0.00
28.50 29.00	0.00	0.00	0.00
29.50	0.00	0.00	0.00
30.00	0.00	0.00	0.00
30.50	0.00	0.00	0.00
			I

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#### Summary for Link DP-P1: Design Point #1

Inflow Area = 92.840 ac, 4.36% Impervious, Inflow Depth = 0.44" for 1-YR event

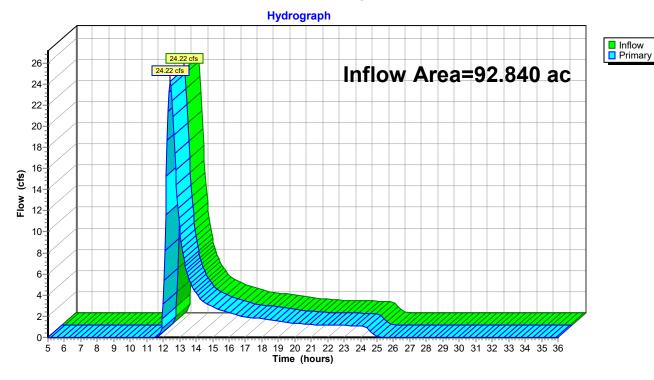
Inflow = 24.22 cfs @ 12.42 hrs, Volume= 3.424 af

Primary = 24.22 cfs @ 12.42 hrs, Volume= 3.424 af, Atten= 0%, Lag= 0.0 min

Routed to Link T-P: Total Proposed

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

#### Link DP-P1: Design Point #1



Primary

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

 $0.00 \\ 0.00$ 

0.00

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#### Hydrograph for Link DP-P1: Design Point #1

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00	0.00	0.00	0.00
5.50 6.00	0.00	0.00 0.00	0.00 0.00
6.50	0.00	0.00	0.00
7.00	0.00	0.00	0.00
7.50	0.00	0.00	0.00
8.00	0.00	0.00	0.00
8.50	0.00	0.00	0.00 0.00
9.00 9.50	0.00 0.00	0.00 0.00	0.00
10.00	0.00	0.00	0.00
10.50	0.00	0.00	0.00
11.00	0.00	0.00	0.00
11.50	0.00 <b>3.19</b>	0.00	0.00 <b>3.19</b>
12.00 12.50	23.53	0.00 0.00	23.53
13.00	10.45	0.00	10.45
13.50	5.84	0.00	5.84
14.00	4.17	0.00	4.17
14.50	3.29	0.00	3.29
15.00 15.50	2.89 2.61	0.00 0.00	2.89 2.61
16.00	2.35	0.00	2.35
16.50	2.10	0.00	2.10
17.00	1.95	0.00	1.95
17.50	1.85	0.00	1.85
18.00 18.50	1.76 1.66	0.00 0.00	1.76 1.66
19.00	1.56	0.00	1.56
19.50	1.45	0.00	1.45
20.00	1.35	0.00	1.35
20.50 21.00	1.25 1.20	0.00	1.25 1.20
21.50	1.20	0.00 0.00	1.18
22.00	1.16	0.00	1.16
22.50	1.15	0.00	1.15
23.00	1.13	0.00	1.13
23.50 24.00	1.11	0.00	1.11
24.50	1.09 0.67	0.00 0.00	1.09 0.67
25.00	0.11	0.00	0.11
25.50	0.02	0.00	0.02
26.00	0.00	0.00	0.00
26.50 27.00	0.00	0.00	0.00
27.00 27.50	0.00 0.00	0.00 0.00	0.00 0.00
28.00	0.00	0.00	0.00
28.50	0.00	0.00	0.00
29.00	0.00	0.00	0.00
29.50 30.00	0.00 0.00	0.00 0.00	0.00 0.00
30.50	0.00	0.00	0.00

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#### Summary for Link DP-P2: Design Point #1

Inflow Area = 118.930 ac, 3.43% Impervious, Inflow Depth = 0.41" for 1-YR event

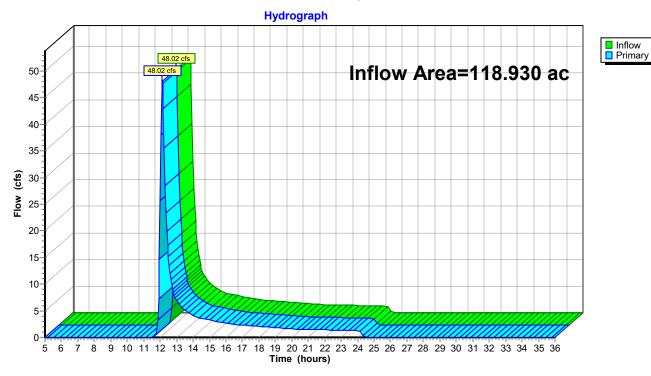
Inflow = 48.02 cfs @ 12.14 hrs, Volume= 4.043 af

Primary = 48.02 cfs @ 12.14 hrs, Volume= 4.043 af, Atten= 0%, Lag= 0.0 min

Routed to Link T-P: Total Proposed

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

#### Link DP-P2: Design Point #1



Primary

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

 $0.00 \\ 0.00$ 

0.00

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#### Hydrograph for Link DP-P2: Design Point #1

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00	0.00	0.00	0.00
5.50 6.00	0.00	0.00 0.00	0.00 0.00
6.50	0.00	0.00	0.00
7.00	0.00	0.00	0.00
7.50	0.00	0.00	0.00
8.00	0.00	0.00	0.00
8.50 9.00	0.00	0.00 0.00	0.00 0.00
9.50	0.00	0.00	0.00
10.00	0.00	0.00	0.00
10.50	0.00	0.00	0.00
11.00	0.00	0.00	0.00
11.50 12.00	0.00 <b>25.67</b>	0.00 0.00	0.00 <b>25.67</b>
12.50	15.12	0.00	15.12
13.00	6.95	0.00	6.95
13.50	5.20	0.00	5.20
14.00	4.17 3.55	0.00	4.17
14.50 15.00	3.33	0.00 0.00	3.55 3.26
15.50	2.95	0.00	2.95
16.00	2.63	0.00	2.63
16.50	2.40	0.00	2.40
17.00 17.50	2.28 2.17	0.00 0.00	2.28 2.17
18.00	2.06	0.00	2.06
18.50	1.94	0.00	1.94
19.00	1.81	0.00	1.81
19.50 20.00	1.68 1.55	0.00 0.00	1.68 1.55
20.50	1.47	0.00	1.47
21.00	1.45	0.00	1.45
21.50	1.43	0.00	1.43
22.00 22.50	1.41 1.38	0.00 0.00	1.41 1.38
23.00	1.36	0.00	1.36
23.50	1.34	0.00	1.34
24.00	1.32	0.00	1.32
24.50	0.09	0.00	0.09
25.00 25.50	0.00	0.00 0.00	0.00 0.00
26.00	0.00	0.00	0.00
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50 28.00	0.00	0.00 0.00	0.00 0.00
28.50	0.00	0.00	0.00
29.00	0.00	0.00	0.00
29.50	0.00	0.00	0.00
30.00 30.50	0.00 0.00	0.00 0.00	0.00 0.00
00.00	0.00	0.00	0.00

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#### **Summary for Link T-E: Total Existing**

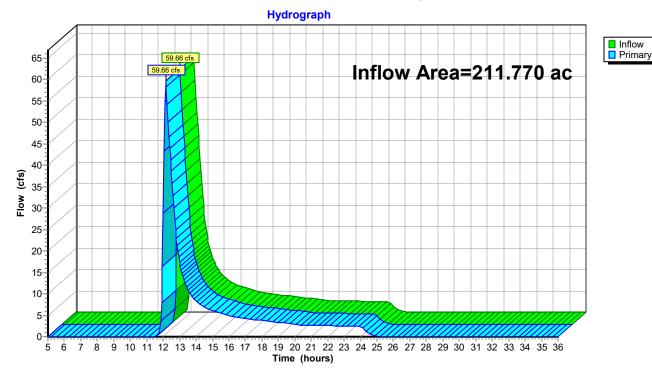
Inflow Area = 211.770 ac, 3.43% Impervious, Inflow Depth = 0.42" for 1-YR event

Inflow = 59.66 cfs @ 12.17 hrs, Volume= 7.468 af

Primary = 59.66 cfs @ 12.17 hrs, Volume= 7.468 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

#### **Link T-E: Total Existing**



Primary

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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#### Hydrograph for Link T-E: Total Existing

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00	0.00	0.00	0.00
5.50	0.00	0.00	0.00
6.00	0.00	0.00	0.00
6.50	0.00	0.00	0.00
7.00	0.00	0.00	0.00
7.50	0.00	0.00	0.00
8.00	0.00	0.00	0.00
8.50 9.00	0.00 0.00	0.00 0.00	0.00 0.00
9.50	0.00	0.00	0.00
10.00	0.00	0.00	0.00
10.50	0.00	0.00	0.00
11.00	0.00	0.00	0.00
11.50	0.00	0.00	0.00
12.00	28.87	0.00	28.87
12.50	38.65	0.00	38.65
13.00	17.40	0.00	17.40
13.50	11.04	0.00	11.04
14.00	8.34	0.00	8.34
14.50	6.84	0.00	6.84
15.00	6.15	0.00	6.15
15.50	5.57	0.00	5.57
16.00	4.98	0.00	4.98
16.50	4.49	0.00	4.49
17.00 17.50	4.24	0.00	4.24
17.50	4.02 3.81	0.00 0.00	4.02 3.81
18.50	3.59	0.00	3.59
19.00	3.37	0.00	3.37
19.50	3.13	0.00	3.13
20.00	2.90	0.00	2.90
20.50	2.72	0.00	2.72
21.00	2.65	0.00	2.65
21.50	2.61	0.00	2.61
22.00	2.57	0.00	2.57
22.50	2.53	0.00	2.53
23.00	2.49	0.00	2.49
23.50	2.45	0.00	2.45
24.00	2.40	0.00	2.40
24.50	0.76	0.00	0.76
25.00 25.50	0.11 0.02	0.00 0.00	0.11 0.02
26.00	0.02	0.00	0.02
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50	0.00	0.00	0.00
28.00	0.00	0.00	0.00
28.50	0.00	0.00	0.00
29.00	0.00	0.00	0.00
29.50	0.00	0.00	0.00
30.00	0.00	0.00	0.00
30.50	0.00	0.00	0.00
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#### Summary for Link T-P: Total Proposed

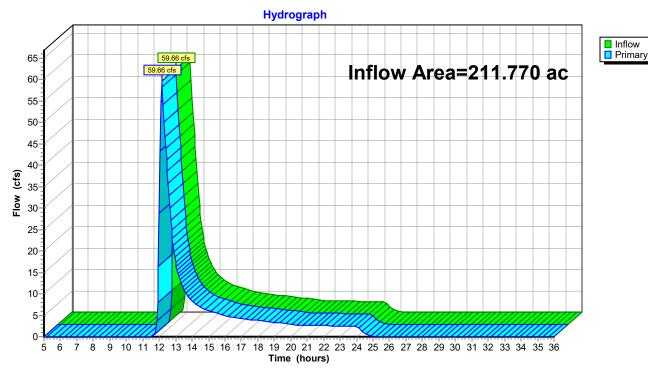
Inflow Area = 211.770 ac, 3.84% Impervious, Inflow Depth = 0.42" for 1-YR event

Inflow = 59.66 cfs @ 12.17 hrs, Volume= 7.468 af

Primary = 59.66 cfs @ 12.17 hrs, Volume= 7.468 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

#### **Link T-P: Total Proposed**



Primary

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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# Hydrograph for Link T-P: Total Proposed

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00 5.50	0.00 0.00	<b>0.00</b> 0.00	0.00 0.00
6.00	0.00	0.00	0.00
6.50	0.00	0.00	0.00
7.00	0.00	0.00	0.00
7.50	0.00	0.00	0.00
8.00	0.00	0.00	0.00
8.50	0.00	0.00	0.00
9.00	0.00	0.00	0.00
9.50	0.00	0.00	0.00
10.00 10.50	0.00	0.00 0.00	0.00 0.00
11.00	0.00	0.00	0.00
11.50	0.00	0.00	0.00
12.00	28.87	0.00	28.87
12.50	38.65	0.00	38.65
13.00	17.40	0.00	17.40
13.50	11.04	0.00	11.04
14.00	8.34	0.00	8.34
14.50 15.00	6.84 6.15	0.00 0.00	6.84 6.15
15.50	5.57	0.00	5.57
16.00	4.98	0.00	4.98
16.50	4.49	0.00	4.49
17.00	4.24	0.00	4.24
17.50	4.02	0.00	4.02
18.00	3.81	0.00	3.81
18.50 19.00	3.59 3.37	0.00 0.00	3.59 3.37
19.50	3.13	0.00	3.13
20.00	2.90	0.00	2.90
20.50	2.72	0.00	2.72
21.00	2.65	0.00	2.65
21.50	2.61	0.00	2.61
22.00	2.57	0.00	2.57
22.50 23.00	2.53	0.00 0.00	2.53 2.49
23.50	2.49 2.45	0.00	2.49
24.00	2.40	0.00	2.40
24.50	0.76	0.00	0.76
25.00	0.11	0.00	0.11
25.50	0.02	0.00	0.02
26.00	0.00	0.00	0.00
26.50	0.00	0.00	0.00
27.00 27.50	0.00	0.00	0.00 0.00
28.00	0.00 0.00	0.00 0.00	0.00
28.50	0.00	0.00	0.00
29.00	0.00	0.00	0.00
29.50	0.00	0.00	0.00
30.00	0.00	0.00	0.00
30.50	0.00	0.00	0.00
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Time span=5.00-36.00 hrs, dt=0.05 hrs, 621 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment DA-E1: Existing DA-E1 Runoff Area=92.840 ac 4.35% Impervious Runoff Depth=1.23" Flow Length=3,681' Tc=40.6 min CN=79 Runoff=77.98 cfs 9.536 af

Subcatchment DA-E2: Existing DA-E2

Runoff Area=118.930 ac 2.71% Impervious Runoff Depth=1.17"

Flow Length=1,836' Tc=18.3 min CN=78 Runoff=158.40 cfs 11.613 af

**Subcatchment DA-P1: Proposed DA-P1** Runoff Area=92.840 ac 4.36% Impervious Runoff Depth=1.23" Flow Length=3,681' Tc=40.6 min CN=79 Runoff=77.98 cfs 9.536 af

Subcatchment DA-P2: Proposed DA-P1 Runoff Area=118.930 ac 3.43% Impervious Runoff Depth=1.17" Flow Length=1,836' Tc=18.3 min CN=78 Runoff=158.40 cfs 11.613 af

Link DP-E1: Design Point #1 Inflow=77.98 cfs 9.536 af Primary=77.98 cfs 9.536 af

Link DP-E2: Design Point #2 Inflow=158.40 cfs 11.613 af

Primary=158.40 cfs 11.613 af

Link DP-P1: Design Point #1 Inflow=77.98 cfs 9.536 af Primary=77.98 cfs 9.536 af

**Link DP-P2: Design Point #1**Inflow=158.40 cfs 11.613 af Primary=158.40 cfs 11.613 af

Link T-E: Total Existing

Inflow=200.72 cfs 21.149 af
Primary=200.72 cfs 21.149 af

Link T-P: Total Proposed Inflow=200.72 cfs 21.149 af Primary=200.72 cfs 21.149 af

Total Runoff Area = 423.540 ac Runoff Volume = 42.297 af Average Runoff Depth = 1.20" 96.37% Pervious = 408.153 ac 3.63% Impervious = 15.387 ac

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### Summary for Subcatchment DA-E1: Existing DA-E1

[47] Hint: Peak is 265% of capacity of segment #4

Runoff = 77.98 cfs @ 12.39 hrs, Volume=

9.536 af, Depth= 1.23"

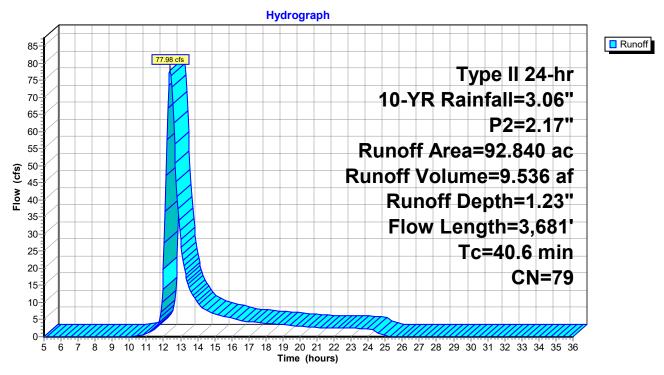
Routed to Link DP-E1 : Design Point #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs Type II 24-hr 10-YR Rainfall=3.06", P2=2.17"

_	Area	(ac) (	CN Des	cription		
	5.	520	77 Woo	ds, Good,	HSG D	
*	3.	710	98 Exis	ting Imper	vious	
	0.	330	98 Wat	er Surface	, HSG D	
	83.	180	78 Mea	dow, non-	grazed, HS	G D
	0.	100			grazed, HS	
	92.	840	79 Wei	ghted Avei	age	
	88.	800	95.6	5% Pervio	us Area	
	4.	040	4.35	% Impervi	ous Area	
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	3.0	33	0.1818	0.19		Sheet Flow,
						Grass: Dense n= 0.240 P2= 2.17"
	9.8	67	0.0373	0.11		Sheet Flow,
						Grass: Dense n= 0.240 P2= 2.17"
	23.6	2,596	0.0130	1.84		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	4.2	985	0.0160	3.93	29.44	Channel Flow,
						Area= 7.5 sf Perim= 12.0' r= 0.63'
						n= 0.035 Earth, dense weeds
	40.6	3,681	Total			

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# **Subcatchment DA-E1: Existing DA-E1**



Runoff

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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# Hydrograph for Subcatchment DA-E1: Existing DA-E1

(hours) (inches) (inches)

3.06

3.06

3.06

3.06

3.06

3.06

3.06

3.06

3.06

3.06

3.06

Time

31.00

31.50

32.00

32.50

33.00

33.50

34.00

34.50

35.00

35.50

36.00

Precip. Excess

1.23

1.23

1.23

1.23

1.23

1.23

1.23

1.23

1.23

1.23

Time	Drasin	Гуссов	Dunaff
Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
5.00	0.19	0.00	0.00
5.50	0.22	0.00	0.00
6.00	0.24	0.00	0.00
6.50	0.27	0.00	0.00
7.00	0.30	0.00	0.00
7.50	0.33	0.00	0.00
8.00 8.50	0.37	0.00	0.00 0.00
9.00	0.40 0.45	0.00	0.00
9.50	0.50	0.00	0.00
10.00	0.55	0.00	0.00
10.50	0.62	0.00	0.17
11.00	0.72	0.01	0.85
11.50	0.87	0.04	2.43
12.00 12.50	2.03 2.25	0.54	19.71 72.92
13.00	2.25	0.67 0.75	28.40
13.50	2.44	0.80	14.63
14.00	2.51	0.84	10.02
14.50	2.56	0.88	7.76
15.00	2.61	0.91	6.74
15.50	2.65	0.94	6.05
16.00 16.50	2.69 2.73	0.97 0.99	5.41 4.80
17.00	2.73	1.02	4.44
17.50	2.79	1.04	4.19
18.00	2.82	1.06	3.96
18.50	2.84	1.08	3.72
19.00	2.87	1.09	3.49
19.50	2.89	1.11	3.24
20.00 20.50	2.91 2.93	1.13 1.14	3.00
21.00	2.95	1.14	2.77 2.67
21.50	2.97	1.17	2.61
22.00	2.99	1.18	2.57
22.50	3.01	1.19	2.52
23.00	3.03	1.21	2.47
23.50	3.04	1.22	2.43
24.00 24.50	<b>3.06</b> 3.06	<b>1.23</b> 1.23	2.38 1.46
25.00	3.06	1.23	0.25
25.50	3.06	1.23	0.04
26.00	3.06	1.23	0.00
26.50	3.06	1.23	0.00
27.00	3.06	1.23	0.00
27.50	3.06	1.23	0.00
28.00 28.50	3.06 3.06	1.23 1.23	0.00 0.00
29.00	3.06	1.23	0.00
29.50	3.06	1.23	0.00
30.00	3.06	1.23	0.00
30.50	3.06	1.23	0.00
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### Summary for Subcatchment DA-E2: Existing DA-E2

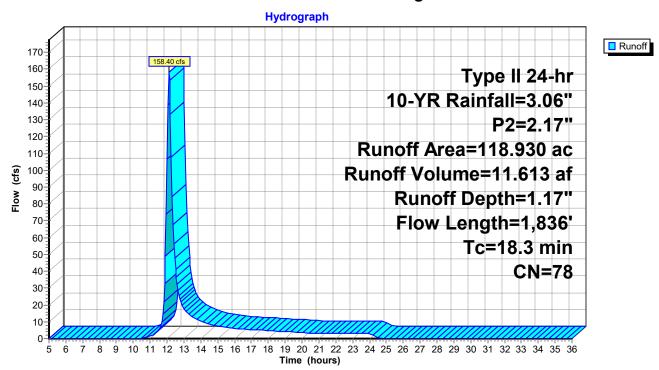
Runoff = 158.40 cfs @ 12.12 hrs, Volume= 11.613 af, Depth= 1.17"

Routed to Link DP-E2: Design Point #2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs Type II 24-hr 10-YR Rainfall=3.06", P2=2.17"

	Area	(ac)	CN	Desc	cription		
	29.	160	77	Woo	ds, Good,	HSG D	
*	2.	260	98	Exist	ting Imper	vious	
	86.	550	78	Mea	dow, non-g	grazed, HS	G D
*	0.	960	98	Wate	er Surface	, HSG D	
	118.	930	78	Weig	ghted Aver	age	
	115.	710		97.2	9% Pervio	us Area	
	3.	220		2.71	% Impervi	ous Area	
	_			01			
	Tc	Lengt		Slope	Velocity	Capacity	Description
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	11.6	10	0 0	.0550	0.14		Sheet Flow,
							Grass: Dense n= 0.240 P2= 2.17"
	6.7	1,73	6 0	.0730	4.35		Shallow Concentrated Flow,
							Unpaved Kv= 16.1 fps
	18.3	1,83	6 T	otal			

### **Subcatchment DA-E2: Existing DA-E2**



Runoff

(cfs)

0.00

0.00

 $0.00 \\ 0.00$ 

0.00

0.00

0.00

 $0.00\\0.00$ 

0.00

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# Hydrograph for Subcatchment DA-E2: Existing DA-E2

(hours) (inches) (inches)

3.06

3.06

3.06

3.06

3.06

3.06

3.06

3.06

3.06

3.06

3.06

Precip. Excess

1.17

1.17

1.17

1.17

1.17

1.17

1.17

1.17

1.17

1.17

1.17

Time

31.00

31.50

32.00

32.50

33.00

33.50

34.00

34.50

35.00

35.50

			1
Time	Precip. (inches)	Excess	Runoff
(hours) 5.00	0.19	(inches) 0.00	(cfs) 0.00
5.50	0.13	0.00	0.00
6.00	0.24	0.00	0.00
6.50	0.27	0.00	0.00
7.00	0.30	0.00	0.00
7.50	0.33	0.00	0.00
8.00 8.50	0.37 0.40	0.00	0.00 0.00
9.00	0.45	0.00	0.00
9.50	0.50	0.00	0.00
10.00	0.55	0.00	0.00
10.50	0.62	0.00	0.25
11.00	0.72	0.01	1.51
11.50 12.00	0.87 2.03	0.03 0.50	4.70 <b>106.42</b>
12.50	2.25	0.63	41.32
13.00	2.36	0.70	17.45
13.50	2.44	0.75	12.80
14.00	2.51	0.79	10.10
14.50 15.00	2.56 2.61	0.83 0.86	8.53 7.75
15.50	2.65	0.89	6.97
16.00	2.69	0.92	6.16
16.50	2.73	0.94	5.59
17.00	2.76	0.96	5.30
17.50 18.00	2.79 2.82	0.98 1.00	5.02 4.73
18.50	2.84	1.02	4.44
19.00	2.87	1.04	4.14
19.50	2.89	1.05	3.83
20.00	2.91	1.07	3.53
20.50 21.00	2.93 2.95	1.08 1.09	3.33 3.27
21.50	2.93	1.09	3.22
22.00	2.99	1.12	3.16
22.50	3.01	1.13	3.11
23.00	3.03	1.15	3.05
23.50	3.04	1.16	2.99
24.00 24.50	<b>3.06</b> 3.06	<b>1.17</b> 1.17	2.93 0.21
25.00	3.06	1.17	0.00
25.50	3.06	1.17	0.00
26.00	3.06	1.17	0.00
26.50	3.06	1.17	0.00
27.00 27.50	3.06 3.06	1.17 1.17	0.00 0.00
28.00	3.06	1.17	0.00
28.50	3.06	1.17	0.00
29.00	3.06	1.17	0.00
29.50	3.06	1.17	0.00
30.00	3.06	1.17 1.17	0.00
30.50	3.06	1.17	0.00

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### Summary for Subcatchment DA-P1: Proposed DA-P1

[47] Hint: Peak is 265% of capacity of segment #4

Runoff = 77.98 cfs @ 12.39 hrs, Volume=

9.536 af, Depth= 1.23"

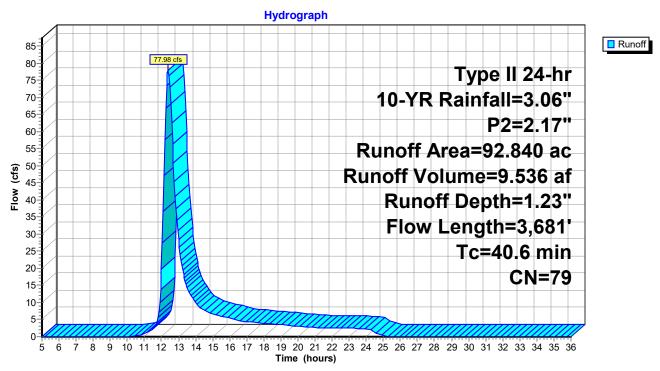
Routed to Link DP-P1 : Design Point #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs Type II 24-hr 10-YR Rainfall=3.06", P2=2.17"

	Area	(ac)	CN	Desc	cription		
	5.	520	77	Woods, Good, HSG D			
*	3.	297	98	Exis	ting Imper	vious	
*	0.	420	98		soed Impe		
	0.	330	98	Wate	er Surface	, HSG D	
	83.	173	78	Mea	dow, non-g	grazed, HS	GD
	0.	100	58			grazed, HS	
92.840 79 Weighted Average				Weig			
	88.	793		95.6	4% Pervio	us Area	
	4.047			4.36	% Impervi	ous Area	
	Тс	Length	S	lope	Velocity	Capacity	Description
_	(min)	(feet	) (	(ft/ft)	(ft/sec)	(cfs)	
	3.0	33	0.1	1818	0.19		Sheet Flow,
							Grass: Dense n= 0.240 P2= 2.17"
	9.8	67	0.0	)373	0.11		Sheet Flow,
							Grass: Dense n= 0.240 P2= 2.17"
	23.6	2,596	0.0	)130	1.84		Shallow Concentrated Flow,
							Unpaved Kv= 16.1 fps
	4.2	985	0.0	)160	3.93	29.44	Channel Flow,
_							Area= 7.5 sf Perim= 12.0' r= 0.63' n= 0.035
	40.6	3,681	To	tal			

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# Subcatchment DA-P1: Proposed DA-P1



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# Hydrograph for Subcatchment DA-P1: Proposed DA-P1

	_	_	1
Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
5.00	0.19	0.00	0.00
5.50	0.22	0.00	0.00
6.00	0.24	0.00	0.00
6.50	0.27	0.00	0.00
7.00	0.30	0.00	0.00
7.50	0.33	0.00	0.00
8.00 8.50	0.37 0.40	0.00	0.00 0.00
9.00	0.45	0.00	0.00
9.50	0.50	0.00	0.00
10.00	0.55	0.00	0.00
10.50	0.62	0.00	0.17
11.00 11.50	0.72 0.87	0.01 0.04	0.85 2.43
12.00	2.03	0.54	19.71
12.50	2.25	0.67	72.92
13.00	2.36	0.75	28.40
13.50	2.44	0.80	14.63
14.00 14.50	2.51 2.56	0.84 0.88	10.02 7.76
15.00	2.61	0.91	6.74
15.50	2.65	0.94	6.05
16.00	2.69	0.97	5.41
16.50 17.00	2.73 2.76	0.99 1.02	4.80 4.44
17.50	2.79	1.02	4.19
18.00	2.82	1.06	3.96
18.50	2.84	1.08	3.72
19.00	2.87	1.09	3.49
19.50 20.00	2.89 2.91	1.11 1.13	3.24 3.00
20.50	2.93	1.13	2.77
21.00	2.95	1.15	2.67
21.50	2.97	1.17	2.61
22.00	2.99	1.18	2.57
22.50 23.00	3.01 3.03	1.19 1.21	2.52 2.47
23.50	3.04	1.21	2.43
24.00	3.06	1.23	2.38
24.50	3.06	1.23	1.46
25.00	3.06	1.23	0.25
25.50 26.00	3.06 3.06	1.23 1.23	0.04 0.00
26.50	3.06	1.23	0.00
27.00	3.06	1.23	0.00
27.50	3.06	1.23	0.00
28.00 28.50	3.06 3.06	1.23 1.23	0.00 0.00
29.00	3.06	1.23	0.00
29.50	3.06	1.23	0.00
30.00	3.06	1.23	0.00
30.50	3.06	1.23	0.00

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
31.00	3.06	1.23	0.00
31.50	3.06	1.23	0.00
32.00	3.06	1.23	0.00
32.50	3.06	1.23	0.00
33.00	3.06	1.23	0.00
33.50	3.06	1.23	0.00
34.00	3.06	1.23	0.00
34.50	3.06	1.23	0.00
35.00	3.06	1.23	0.00
35.50	3.06	1.23	0.00
36.00	3.06	1.23	0.00

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#### Summary for Subcatchment DA-P2: Proposed DA-P1

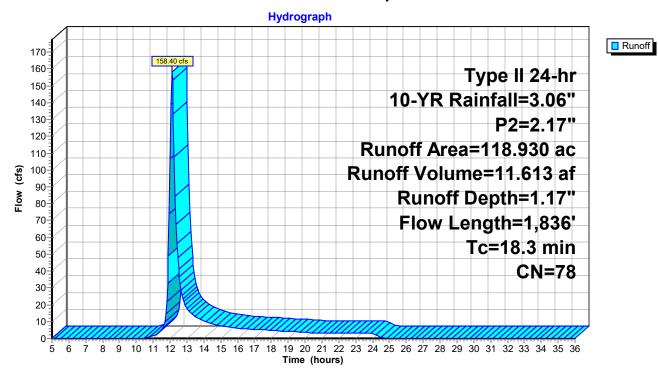
Runoff = 158.40 cfs @ 12.12 hrs, Volume= 11.613 af, Depth= 1.17"

Routed to Link DP-P2 : Design Point #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs Type II 24-hr 10-YR Rainfall=3.06", P2=2.17"

	Area	(ac)	CN	Desc	cription		
	26.	093	77	Woo	ds, Good,	HSG D	
*	2.	080	98	Exist	ting Imper	vious	
*	1.	040	98	Prop	osed Impy	erious/	
*	0.	960	98	Wate	er Surface	, HSG D	
	88.	757	78	Mea	dow, non-ເ	grazed, HS	G D
	118.	930	78	Weig	hted Aver	age	
	114.	850		96.5	7% Pervio	us Area	
	4.	080		3.43	% Impervi	ous Area	
					-		
	Тс	Lengt	:h	Slope	Velocity	Capacity	Description
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	11.6	10	0 (	0.0550	0.14		Sheet Flow,
							Grass: Dense n= 0.240 P2= 2.17"
	6.7	1,73	6	0.0730	4.35		Shallow Concentrated Flow,
		-					Unpaved Kv= 16.1 fps
	18.3	1,83	6	Total			

#### Subcatchment DA-P2: Proposed DA-P1



Runoff

(cfs)

0.00

0.00

0.00

 $0.00 \\ 0.00$ 

0.00

0.00

0.00

0.00

0.00

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### Hydrograph for Subcatchment DA-P2: Proposed DA-P1

(hours) (inches) (inches)

3.06

3.06

3.06

3.06

3.06

3.06

3.06

3.06

3.06

3.06

3.06

Precip. Excess

1.17

1.17

1.17

1.17

1.17

1.17

1.17

1.17

1.17

1.17

1.17

Time

31.00

31.50

32.00

32.50

33.00

33.50

34.00

34.50

35.00

35.50

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.19	0.00	0.00
5.50	0.22	0.00	0.00
6.00	0.24	0.00	0.00
6.50	0.27	0.00	0.00
7.00	0.30	0.00	0.00
7.50	0.33	0.00	0.00
8.00	0.37	0.00	0.00
8.50	0.40	0.00	0.00
9.00	0.45	0.00	0.00
9.50	0.50	0.00	0.00
10.00	0.55	0.00	0.00
10.50	0.62	0.00	0.25
11.00 11.50	0.72 0.87	0.01	1.51 4.70
12.00	2.03	0.03 0.50	106.42
12.50	2.25	0.63	41.32
13.00	2.36	0.70	17.45
13.50	2.44	0.75	12.80
14.00	2.51	0.79	10.10
14.50	2.56	0.83	8.53
15.00	2.61	0.86	7.75
15.50	2.65	0.89	6.97
16.00	2.69	0.92	6.16
16.50	2.73	0.94	5.59
17.00	2.76	0.96	5.30
17.50	2.79	0.98	5.02
18.00	2.82	1.00	4.73
18.50	2.84	1.02	4.44 4.14
19.00 19.50	2.87 2.89	1.04 1.05	3.83
20.00	2.09	1.03	3.53
20.50	2.93	1.08	3.33
21.00	2.95	1.09	3.27
21.50	2.97	1.11	3.22
22.00	2.99	1.12	3.16
22.50	3.01	1.13	3.11
23.00	3.03	1.15	3.05
23.50	3.04	1.16	2.99
24.00	3.06	1.17	2.93
24.50	3.06	1.17	0.21
25.00	3.06	1.17	0.00
25.50 26.00	3.06 3.06	1.17 1.17	0.00 0.00
26.50	3.06	1.17	0.00
27.00	3.06	1.17	0.00
27.50	3.06	1.17	0.00
28.00	3.06	1.17	0.00
28.50	3.06	1.17	0.00
29.00	3.06	1.17	0.00
29.50	3.06	1.17	0.00
30.00	3.06	1.17	0.00
30.50	3.06	1.17	0.00
			I

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### Summary for Link DP-E1: Design Point #1

Inflow Area = 92.840 ac, 4.35% Impervious, Inflow Depth = 1.23" for 10-YR event

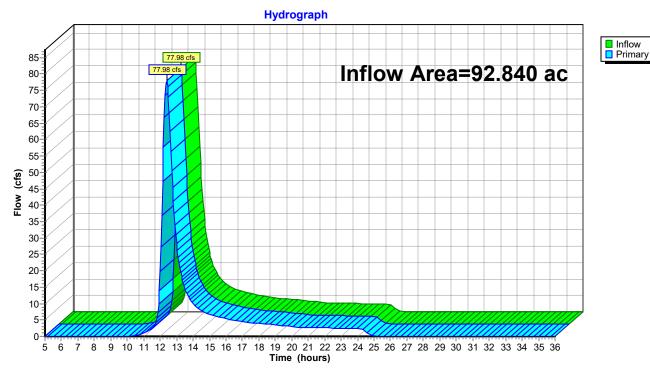
Inflow = 77.98 cfs @ 12.39 hrs, Volume= 9.536 af

Primary = 77.98 cfs @ 12.39 hrs, Volume= 9.536 af, Atten= 0%, Lag= 0.0 min

Routed to Link T-E: Total Existing

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

### Link DP-E1: Design Point #1



Primary

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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# Hydrograph for Link DP-E1: Design Point #1

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00	0.00	0.00	0.00
5.50 6.00	0.00	0.00 0.00	0.00 0.00
6.50	0.00	0.00	0.00
7.00	0.00	0.00	0.00
7.50 8.00	0.00	0.00 0.00	0.00 0.00
8.50	0.00	0.00	0.00
9.00	0.00	0.00	0.00
9.50	0.00	0.00	0.00
10.00 10.50	0.00 0.17	0.00 0.00	0.00 0.17
11.00	0.85	0.00	0.85
11.50	2.43	0.00	2.43
12.00 12.50	19.71 72.92	0.00 0.00	19.71 72.92
13.00	28.40	0.00	28.40
13.50	14.63	0.00	14.63
14.00	10.02	0.00	10.02
14.50 15.00	7.76 6.74	0.00 0.00	7.76 6.74
15.50	6.05	0.00	6.05
16.00	5.41	0.00	5.41
16.50 17.00	4.80 4.44	0.00 0.00	4.80 4.44
17.50	4.19	0.00	4.19
18.00	3.96	0.00	3.96
18.50	3.72	0.00 0.00	3.72
19.00 19.50	3.49 3.24	0.00	3.49 3.24
20.00	3.00	0.00	3.00
20.50	2.77	0.00	2.77
21.00 21.50	2.67 2.61	0.00 0.00	2.67 2.61
22.00	2.57	0.00	2.57
22.50	2.52	0.00	2.52
23.00 23.50	2.47 2.43	0.00 0.00	2.47 2.43
24.00	2.43	0.00	2.43
24.50	1.46	0.00	1.46
25.00	0.25	0.00	0.25
25.50 26.00	0.04 0.00	0.00 0.00	0.04 0.00
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50 28.00	0.00 0.00	0.00 0.00	0.00 0.00
28.50	0.00	0.00	0.00
29.00	0.00	0.00	0.00
29.50	0.00	0.00	0.00
30.00 30.50	0.00 0.00	0.00 0.00	0.00 0.00

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### Summary for Link DP-E2: Design Point #2

Inflow Area = 118.930 ac, 2.71% Impervious, Inflow Depth = 1.17" for 10-YR event

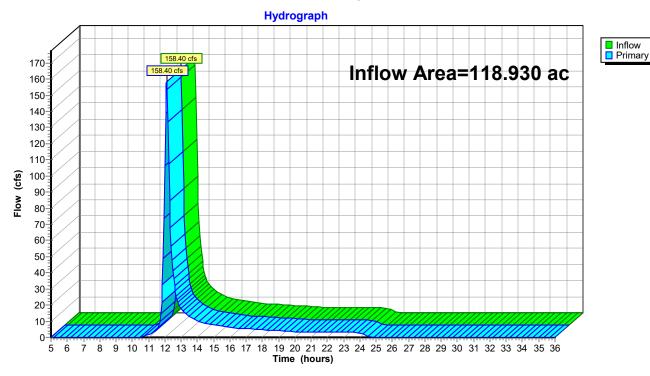
Inflow = 158.40 cfs @ 12.12 hrs, Volume= 11.613 af

Primary = 158.40 cfs @ 12.12 hrs, Volume= 11.613 af, Atten= 0%, Lag= 0.0 min

Routed to Link T-E: Total Existing

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

#### Link DP-E2: Design Point #2



Primary

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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### Hydrograph for Link DP-E2: Design Point #2

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00	0.00	0.00	0.00
5.50	0.00	0.00	0.00
6.00 6.50	0.00 0.00	0.00 0.00	0.00 0.00
7.00	0.00	0.00	0.00
7.50	0.00	0.00	0.00
8.00	0.00	0.00	0.00
8.50 9.00	0.00 0.00	0.00 0.00	0.00 0.00
9.50	0.00	0.00	0.00
10.00	0.00	0.00	0.00
10.50	0.25	0.00	0.25
11.00 11.50	1.51 4.70	0.00 0.00	1.51 4.70
12.00	106.42	0.00	106.42
12.50	41.32	0.00	41.32
13.00	17.45	0.00	17.45
13.50 14.00	12.80 10.10	0.00 0.00	12.80 10.10
14.50	8.53	0.00	8.53
15.00	7.75	0.00	7.75
15.50	6.97	0.00	6.97 6.16
16.00 16.50	6.16 5.59	0.00 0.00	5.59
17.00	5.30	0.00	5.30
17.50	5.02	0.00	5.02
18.00	4.73	0.00	4.73
18.50 19.00	4.44 4.14	0.00 0.00	4.44 4.14
19.50	3.83	0.00	3.83
20.00	3.53	0.00	3.53
20.50 21.00	3.33 3.27	0.00 0.00	3.33 3.27
21.50	3.27	0.00	3.22
22.00	3.16	0.00	3.16
22.50	3.11	0.00	3.11
23.00 23.50	3.05 2.99	0.00 0.00	3.05 2.99
24.00	2.93	0.00	2.93
24.50	0.21	0.00	0.21
25.00	0.00	0.00	0.00
25.50 26.00	0.00	0.00 0.00	0.00 0.00
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50	0.00	0.00	0.00
28.00 28.50	0.00 0.00	0.00 0.00	0.00 0.00
29.00	0.00	0.00	0.00
29.50	0.00	0.00	0.00
30.00 30.50	0.00 0.00	0.00 0.00	0.00 0.00
00.00	0.00	0.00	0.00

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### Summary for Link DP-P1: Design Point #1

Inflow Area = 92.840 ac, 4.36% Impervious, Inflow Depth = 1.23" for 10-YR event

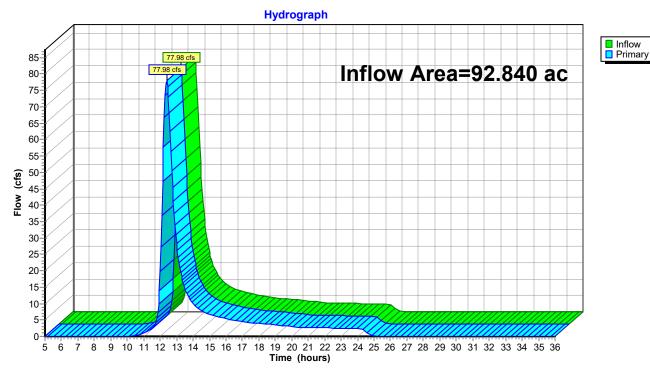
Inflow = 77.98 cfs @ 12.39 hrs, Volume= 9.536 af

Primary = 77.98 cfs @ 12.39 hrs, Volume= 9.536 af, Atten= 0%, Lag= 0.0 min

Routed to Link T-P: Total Proposed

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

### Link DP-P1: Design Point #1



Primary

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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# Hydrograph for Link DP-P1: Design Point #1

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00	0.00	0.00	0.00
5.50	0.00	0.00	0.00
6.00 6.50	0.00 0.00	0.00 0.00	0.00 0.00
7.00	0.00	0.00	0.00
7.50	0.00	0.00	0.00
8.00	0.00	0.00	0.00
8.50	0.00	0.00	0.00
9.00 9.50	0.00	0.00	0.00
9.50 10.00	0.00	0.00 0.00	0.00 0.00
10.50	0.17	0.00	0.17
11.00	0.85	0.00	0.85
11.50	2.43	0.00	2.43
12.00	19.71	0.00	19.71
12.50 13.00	<b>72.92</b> 28.40	0.00 0.00	<b>72.92</b> 28.40
13.50	14.63	0.00	14.63
14.00	10.02	0.00	10.02
14.50	7.76	0.00	7.76
15.00	6.74	0.00	6.74
15.50 16.00	6.05 5.41	0.00	6.05
16.50	4.80	0.00 0.00	5.41 4.80
17.00	4.44	0.00	4.44
17.50	4.19	0.00	4.19
18.00	3.96	0.00	3.96
18.50	3.72	0.00	3.72
19.00 19.50	3.49 3.24	0.00 0.00	3.49 3.24
20.00	3.24	0.00	3.24
20.50	2.77	0.00	2.77
21.00	2.67	0.00	2.67
21.50	2.61	0.00	2.61
22.00	2.57	0.00	2.57
22.50 23.00	2.52 2.47	0.00 0.00	2.52 2.47
23.50	2.43	0.00	2.43
24.00	2.38	0.00	2.38
24.50	1.46	0.00	1.46
25.00	0.25	0.00	0.25
25.50 26.00	0.04 0.00	0.00 0.00	0.04 0.00
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50	0.00	0.00	0.00
28.00	0.00	0.00	0.00
28.50	0.00	0.00	0.00
29.00 29.50	0.00 0.00	0.00 0.00	0.00 0.00
30.00	0.00	0.00	0.00
30.50	0.00	0.00	0.00

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### Summary for Link DP-P2: Design Point #1

Inflow Area = 118.930 ac, 3.43% Impervious, Inflow Depth = 1.17" for 10-YR event

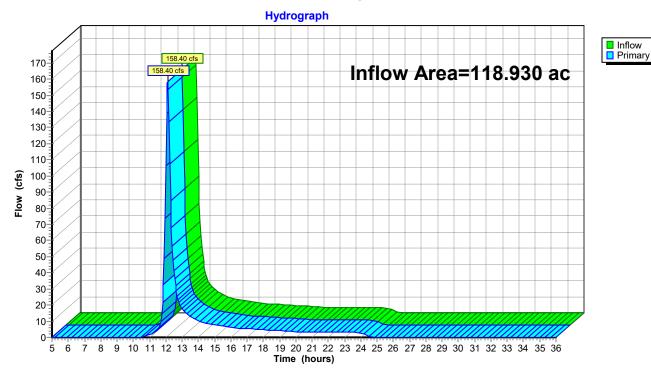
Inflow = 158.40 cfs @ 12.12 hrs, Volume= 11.613 af

Primary = 158.40 cfs @ 12.12 hrs, Volume= 11.613 af, Atten= 0%, Lag= 0.0 min

Routed to Link T-P: Total Proposed

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

#### Link DP-P2: Design Point #1



Primary

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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# Hydrograph for Link DP-P2: Design Point #1

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00	0.00	0.00	0.00
5.50	0.00	0.00	0.00
6.00 6.50	0.00 0.00	0.00 0.00	0.00 0.00
7.00	0.00	0.00	0.00
7.50	0.00	0.00	0.00
8.00	0.00	0.00	0.00
8.50 9.00	0.00 0.00	0.00 0.00	0.00 0.00
9.50	0.00	0.00	0.00
10.00	0.00	0.00	0.00
10.50	0.25	0.00	0.25
11.00 11.50	1.51 4.70	0.00 0.00	1.51 4.70
12.00	106.42	0.00	106.42
12.50	41.32	0.00	41.32
13.00	17.45	0.00	17.45
13.50 14.00	12.80 10.10	0.00 0.00	12.80 10.10
14.50	8.53	0.00	8.53
15.00	7.75	0.00	7.75
15.50	6.97	0.00	6.97 6.16
16.00 16.50	6.16 5.59	0.00 0.00	5.59
17.00	5.30	0.00	5.30
17.50	5.02	0.00	5.02
18.00	4.73	0.00	4.73
18.50 19.00	4.44 4.14	0.00 0.00	4.44 4.14
19.50	3.83	0.00	3.83
20.00	3.53	0.00	3.53
20.50 21.00	3.33 3.27	0.00 0.00	3.33 3.27
21.50	3.27	0.00	3.22
22.00	3.16	0.00	3.16
22.50	3.11	0.00	3.11
23.00 23.50	3.05 2.99	0.00 0.00	3.05 2.99
24.00	2.93	0.00	2.93
24.50	0.21	0.00	0.21
25.00	0.00	0.00	0.00
25.50 26.00	0.00	0.00 0.00	0.00 0.00
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50	0.00	0.00	0.00
28.00 28.50	0.00 0.00	0.00 0.00	0.00 0.00
29.00	0.00	0.00	0.00
29.50	0.00	0.00	0.00
30.00 30.50	0.00 0.00	0.00 0.00	0.00 0.00
00.00	0.00	0.00	0.00

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### **Summary for Link T-E: Total Existing**

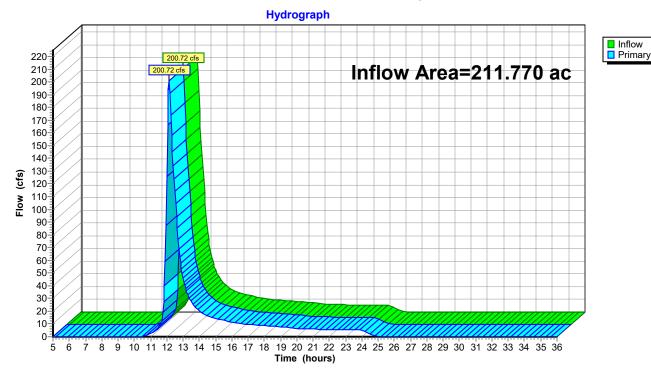
Inflow Area = 211.770 ac, 3.43% Impervious, Inflow Depth = 1.20" for 10-YR event

Inflow = 200.72 cfs @ 12.15 hrs, Volume= 21.149 af

Primary = 200.72 cfs @ 12.15 hrs, Volume= 21.149 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

### **Link T-E: Total Existing**



Primary

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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# Hydrograph for Link T-E: Total Existing

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00 5.50	0.00 0.00	<b>0.00</b> 0.00	0.00 0.00
6.00	0.00	0.00	0.00
6.50	0.00	0.00	0.00
7.00	0.00	0.00	0.00
7.50	0.00	0.00	0.00
8.00	0.00	0.00	0.00
8.50	0.00	0.00	0.00
9.00	0.00	0.00	0.00
9.50	0.00	0.00	0.00
10.00	0.00	0.00	0.00
10.50 11.00	0.42 2.36	0.00 0.00	0.42 2.36
11.50	7.14	0.00	7.14
12.00	126.13	0.00	126.13
12.50	114.24	0.00	114.24
13.00	45.85	0.00	45.85
13.50	27.43	0.00	27.43
14.00	20.12	0.00	20.12
14.50	16.29	0.00	16.29
15.00	14.49	0.00 0.00	14.49
15.50 16.00	13.02 11.57	0.00	13.02 11.57
16.50	10.39	0.00	10.39
17.00	9.75	0.00	9.75
17.50	9.21	0.00	9.21
18.00	8.69	0.00	8.69
18.50	8.16	0.00	8.16
19.00	7.63	0.00	7.63
19.50 20.00	7.08 6.53	0.00 0.00	7.08 6.53
20.50	6.10	0.00	6.10
21.00	5.94	0.00	5.94
21.50	5.83	0.00	5.83
22.00	5.73	0.00	5.73
22.50	5.63	0.00	5.63
23.00	5.52	0.00	5.52
23.50	5.42	0.00	5.42
24.00 24.50	5.32 1.67	0.00	5.32
25.00	0.25	0.00 0.00	1.67 0.25
25.50	0.23	0.00	0.23
26.00	0.00	0.00	0.00
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50	0.00	0.00	0.00
28.00	0.00	0.00	0.00
28.50	0.00 0.00	0.00 0.00	0.00
29.00 29.50	0.00	0.00	0.00 0.00
30.00	0.00	0.00	0.00
30.50	0.00	0.00	0.00

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### **Summary for Link T-P: Total Proposed**

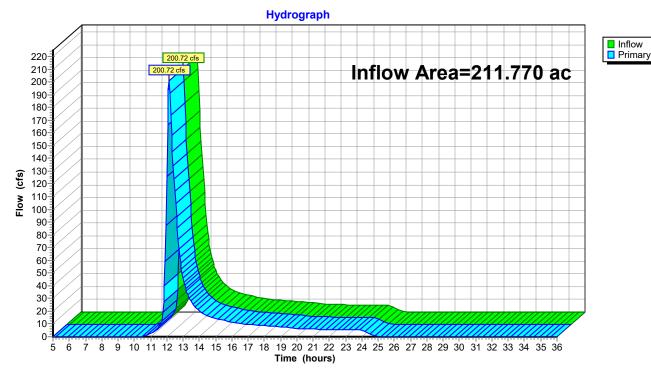
Inflow Area = 211.770 ac, 3.84% Impervious, Inflow Depth = 1.20" for 10-YR event

Inflow = 200.72 cfs @ 12.15 hrs, Volume= 21.149 af

Primary = 200.72 cfs @ 12.15 hrs, Volume= 21.149 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

### **Link T-P: Total Proposed**



Primary

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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# Hydrograph for Link T-P: Total Proposed

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00 5.50	0.00 0.00	<b>0.00</b> 0.00	0.00 0.00
6.00	0.00	0.00	0.00
6.50	0.00	0.00	0.00
7.00	0.00	0.00	0.00
7.50	0.00	0.00	0.00
8.00	0.00	0.00	0.00
8.50	0.00	0.00	0.00
9.00	0.00	0.00	0.00
9.50	0.00	0.00	0.00
10.00	0.00	0.00	0.00
10.50 11.00	0.42 2.36	0.00 0.00	0.42 2.36
11.50	7.14	0.00	7.14
12.00	126.13	0.00	126.13
12.50	114.24	0.00	114.24
13.00	45.85	0.00	45.85
13.50	27.43	0.00	27.43
14.00	20.12	0.00	20.12
14.50	16.29	0.00	16.29
15.00	14.49	0.00 0.00	14.49
15.50 16.00	13.02 11.57	0.00	13.02 11.57
16.50	10.39	0.00	10.39
17.00	9.75	0.00	9.75
17.50	9.21	0.00	9.21
18.00	8.69	0.00	8.69
18.50	8.16	0.00	8.16
19.00	7.63	0.00	7.63
19.50 20.00	7.08 6.53	0.00 0.00	7.08 6.53
20.50	6.10	0.00	6.10
21.00	5.94	0.00	5.94
21.50	5.83	0.00	5.83
22.00	5.73	0.00	5.73
22.50	5.63	0.00	5.63
23.00	5.52	0.00	5.52
23.50	5.42	0.00	5.42
24.00 24.50	5.32 1.67	0.00	5.32
25.00	0.25	0.00 0.00	1.67 0.25
25.50	0.23	0.00	0.23
26.00	0.00	0.00	0.00
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50	0.00	0.00	0.00
28.00	0.00	0.00	0.00
28.50	0.00 0.00	0.00 0.00	0.00
29.00 29.50	0.00	0.00	0.00 0.00
30.00	0.00	0.00	0.00
30.50	0.00	0.00	0.00

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Time span=5.00-36.00 hrs, dt=0.05 hrs, 621 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment DA-E1: Existing DA-E1**Runoff Area=92.840 ac 4.35% Impervious Runoff Depth=2.79"
Flow Length=3,681' Tc=40.6 min CN=79 Runoff=182.90 cfs 21.609 af

Subcatchment DA-E2: Existing DA-E2

Runoff Area=118.930 ac 2.71% Impervious Runoff Depth=2.70"

Flow Length=1,836' Tc=18.3 min CN=78 Runoff=374.10 cfs 26.791 af

**Subcatchment DA-P1: Proposed DA-P1** Runoff Area=92.840 ac 4.36% Impervious Runoff Depth=2.79" Flow Length=3,681' Tc=40.6 min CN=79 Runoff=182.90 cfs 21.609 af

Subcatchment DA-P2: Proposed DA-P1 Runoff Area=118.930 ac 3.43% Impervious Runoff Depth=2.70" Flow Length=1,836' Tc=18.3 min CN=78 Runoff=374.10 cfs 26.791 af

Link DP-E1: Design Point #1 Inflow=182.90 cfs 21.609 af Primary=182.90 cfs 21.609 af

**Link DP-E2: Design Point #2**Inflow=374.10 cfs 26.791 af
Primary=374.10 cfs 26.791 af

Link DP-P1: Design Point #1 Inflow=182.90 cfs 21.609 af Primary=182.90 cfs 21.609 af

**Link DP-P2: Design Point #1**Inflow=374.10 cfs 26.791 af Primary=374.10 cfs 26.791 af

Link T-E: Total Existing

Inflow=478.90 cfs 48.400 af
Primary=478.90 cfs 48.400 af

Link T-P: Total Proposed Inflow=478.90 cfs 48.400 af Primary=478.90 cfs 48.400 af

Total Runoff Area = 423.540 ac Runoff Volume = 96.800 af Average Runoff Depth = 2.74" 96.37% Pervious = 408.153 ac 3.63% Impervious = 15.387 ac

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### **Summary for Subcatchment DA-E1: Existing DA-E1**

[47] Hint: Peak is 621% of capacity of segment #4

Runoff = 182.90 cfs @ 12.38 hrs, Volume=

21.609 af, Depth= 2.79"

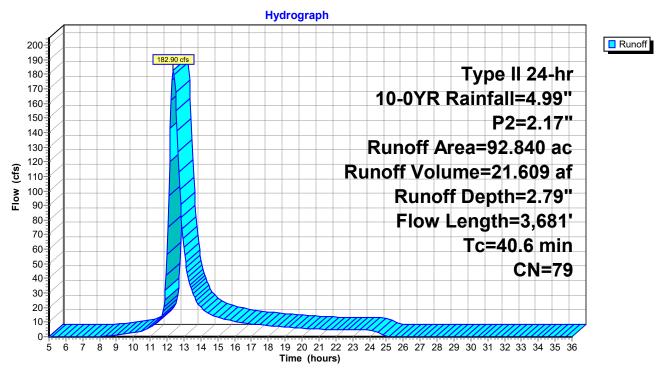
Routed to Link DP-E1 : Design Point #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs Type II 24-hr 10-0YR Rainfall=4.99", P2=2.17"

	Area	(ac)	CN	Desc	cription		
	5.	520	77	Woo	ds, Good,	HSG D	
*	3.	710	98	Exis	ting Imperv	/ious	
	0.	330			er Surface		
	83.	180	78	Mea	dow, non-g	grazed, HS	GD
	0.	100				grazed, HS	
	92.	840	79	Weig	ghted Aver	age	
	88.	800		95.6	5% Pervio	us Area	
	4.	040		4.35	% Impervi	ous Area	
					•		
	Tc	Length	ı SI	ope	Velocity	Capacity	Description
	(min)	(feet)	) (1	ft/ft)	(ft/sec)	(cfs)	
	3.0	33	0.1	818	0.19		Sheet Flow,
							Grass: Dense n= 0.240 P2= 2.17"
	9.8	67	0.0	373	0.11		Sheet Flow,
							Grass: Dense n= 0.240 P2= 2.17"
	23.6	2,596	0.0	130	1.84		Shallow Concentrated Flow,
							Unpaved Kv= 16.1 fps
	4.2	985	0.0	160	3.93	29.44	Channel Flow,
							Area= 7.5 sf Perim= 12.0' r= 0.63'
_							n= 0.035 Earth, dense weeds
	40.6	3,681	Tot	al			

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# **Subcatchment DA-E1: Existing DA-E1**



Runoff

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

 $0.00\\0.00$ 

0.00

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# Hydrograph for Subcatchment DA-E1: Existing DA-E1

(hours) (inches) (inches)

4.99

4.99

4.99

4.99

4.99

4.99

4.99

4.99

4.99

4.99

4.99

Time

31.00

31.50

32.00

32.50

33.00

33.50

34.00

34.50

35.00

35.50

36.00

Precip. Excess

2.79

2.79

2.79

2.79

2.79

2.79

2.79

2.79

2.79

2.79

<b>T</b> :	D		D
Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
5.00	0.31	0.00	0.00
5.50	0.36	0.00	0.00
6.00	0.40	0.00	0.00
6.50	0.45	0.00	0.00
7.00	0.49	0.00	0.00
7.50	0.55	0.00	0.00
8.00	0.60	0.00	0.09
8.50	0.66	0.01	0.43
9.00	0.73	0.01	1.00
9.50	0.81	0.03	1.79
10.00	0.90	0.05	2.62
10.50	1.02	0.08	4.02
11.00	1.17	0.12	6.47
11.50	1.41	0.22	11.28
12.00 12.50	3.31 3.67	1.42 1.70	56.59 167.33
13.00	3.85	1.70	61.20
13.50	3.99	1.95	30.15
14.00	4.09	2.04	20.15
14.50	4.18	2.11	15.44
15.00	4.26	2.18	13.34
15.50	4.33	2.23	11.93
16.00	4.39	2.29	10.61
16.50	4.45	2.33	9.38
17.00	4.50	2.38	8.67
17.50	4.55	2.42	8.16
18.00	4.60	2.46	7.69
18.50	4.64	2.49	7.21
19.00	4.68	2.53	6.74
19.50 20.00	4.72 4.75	2.56 2.59	6.26 5.78
20.50	4.78	2.62	5.33
21.00	4.81	2.64	5.12
21.50	4.84	2.67	5.01
22.00	4.88	2.69	4.91
22.50	4.90	2.72	4.82
23.00	4.93	2.74	4.73
23.50	4.96	2.77	4.64
24.00	4.99	2.79	4.54
24.50	4.99	2.79	2.77
25.00	4.99	2.79	0.48
25.50	4.99	2.79	0.07
26.00 26.50	4.99 4.99	2.79 2.79	0.01 0.00
27.00	4.99	2.79	0.00
27.50	4.99	2.79	0.00
28.00	4.99	2.79	0.00
28.50	4.99	2.79	0.00
29.00	4.99	2.79	0.00
29.50	4.99	2.79	0.00
30.00	4.99	2.79	0.00
30.50	4.99	2.79	0.00
			I

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### Summary for Subcatchment DA-E2: Existing DA-E2

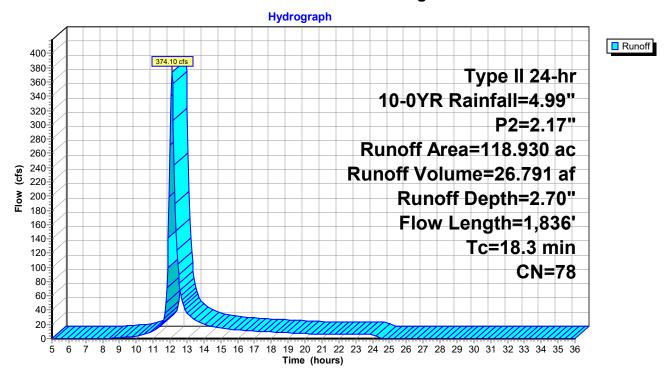
Runoff = 374.10 cfs @ 12.11 hrs, Volume= 26.791 af, Depth= 2.70"

Routed to Link DP-E2: Design Point #2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs Type II 24-hr 10-0YR Rainfall=4.99", P2=2.17"

_	Area	(ac)	CN	Desc	cription		
	29.	160	77	Woo	ds, Good,	HSG D	
*	2.	260	98	Exis	ting Imper	vious	
	86.	550	78	Mea	dow, non-g	grazed, HS	G D
*	0.	960	98	Wate	er Surface	, HSG D	
	118.	930	78	Weig	ghted Aver	age	
	115.	710		97.2	9% Pervio	us Area	
	3.	220		2.71	% Impervi	ous Area	
	_					_	
	Tc	Lengt		Slope	Velocity	Capacity	Description
_	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	11.6	10	0 (	0.0550	0.14		Sheet Flow,
							Grass: Dense n= 0.240 P2= 2.17"
	6.7	1,73	6 (	0.0730	4.35		Shallow Concentrated Flow,
_							Unpaved Kv= 16.1 fps
	18.3	1,83	6	Total			

#### **Subcatchment DA-E2: Existing DA-E2**



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# Hydrograph for Subcatchment DA-E2: Existing DA-E2

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00 5.50	0.31 0.36	0.00 0.00	0.00
6.00	0.40	0.00	0.00
6.50	0.45	0.00	0.00
7.00	0.49	0.00	0.00
7.50 8.00	0.55 0.60	0.00	0.00 0.07
8.50	0.66	0.00	0.61
9.00	0.73	0.01	1.48
9.50 10.00	0.81 0.90	0.02 0.04	2.49 3.76
10.50	1.02	0.04	6.16
11.00	1.17	0.11	10.44
11.50	1.41	0.20	20.16
12.00 12.50	3.31 3.67	1.35 1.63	271.64 89.17
13.00	3.85	1.77	35.99
13.50	3.99	1.88	26.09
14.00 14.50	4.09 4.18	1.96 2.03	20.42 17.14
15.00	4.16	2.03	15.50
15.50	4.33	2.15	13.88
16.00	4.39	2.20	12.23
16.50 17.00	4.45 4.50	2.25 2.29	11.06 10.46
17.50	4.55	2.33	9.87
18.00	4.60	2.37	9.28
18.50 19.00	4.64 4.68	2.41 2.44	8.69 8.08
19.50	4.72	2.44	7.48
20.00	4.75	2.50	6.87
20.50	4.78	2.53	6.48
21.00 21.50	4.81 4.84	2.55 2.58	6.35 6.24
22.00	4.88	2.61	6.12
22.50	4.90	2.63	6.01
23.00 23.50	4.93 4.96	2.66 2.68	5.89 5.77
24.00	4.90 <b>4.99</b>	2.00 <b>2.70</b>	5.65
24.50	4.99	2.70	0.41
25.00	4.99	2.70	0.00
25.50 26.00	4.99 4.99	2.70 2.70	0.00 0.00
26.50	4.99	2.70	0.00
27.00	4.99	2.70	0.00
27.50	4.99	2.70	0.00
28.00 28.50	4.99 4.99	2.70 2.70	0.00 0.00
29.00	4.99	2.70	0.00
29.50	4.99	2.70	0.00
30.00 30.50	4.99 4.99	2.70 2.70	0.00 0.00
30.30	7.33	2.10	0.00

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
31.00	4.99	2.70	0.00
31.50	4.99	2.70	0.00
32.00	4.99	2.70	0.00
32.50	4.99	2.70	0.00
33.00	4.99	2.70	0.00
33.50	4.99	2.70	0.00
34.00	4.99	2.70	0.00
34.50	4.99	2.70	0.00
35.00	4.99	2.70	0.00
35.50	4.99	2.70	0.00
36.00	4.99	2.70	0.00

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### **Summary for Subcatchment DA-P1: Proposed DA-P1**

[47] Hint: Peak is 621% of capacity of segment #4

Runoff = 182.90 cfs @ 12.38 hrs, Volume=

21.609 af, Depth= 2.79"

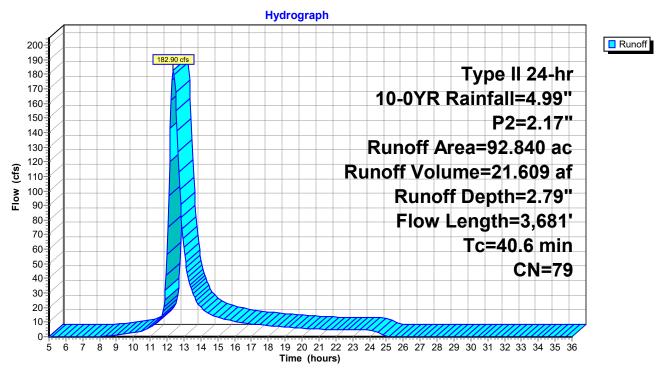
Routed to Link DP-P1 : Design Point #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs Type II 24-hr 10-0YR Rainfall=4.99", P2=2.17"

D
В
Description
Sheet Flow,
Grass: Dense n= 0.240 P2= 2.17"
Sheet Flow,
Grass: Dense n= 0.240 P2= 2.17"
Shallow Concentrated Flow,
Jnpaved Kv= 16.1 fps
511paroa 111 10.11po
Channel Flow,

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# Subcatchment DA-P1: Proposed DA-P1



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### Hydrograph for Subcatchment DA-P1: Proposed DA-P1

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.31	0.00	0.00
5.50	0.36	0.00	0.00
6.00	0.40	0.00	0.00
6.50	0.45		0.00
7.00	0.49	0.00	0.00
7.50	0.55		0.00
8.00	0.60	0.00	0.00
8.50	0.66	0.01	0.43
9.00	0.73	0.01	1.00
9.50	0.81	0.03	1.79
10.00	0.90	0.05	2.62
10.50	1.02	0.08	4.02
11.00	1.17	0.12	6.47
11.50		0.22	11.28
12.00	1.41 3.31	1.42	56.59
12.50	3.67	1.70	<b>167.33</b> 61.20
13.00	3.85	1.84	
13.50	3.99	1.95	30.15
14.00	4.09	2.04	20.15
14.50	4.18	2.11	15.44
15.00	4.26	2.18	13.34
15.50	4.33	2.23	11.93
16.00	4.39	2.29	10.61
16.50	4.45	2.33	9.38
17.00	4.50	2.38	8.67
17.50	4.55	2.42	8.16
18.00	4.60	2.46	7.69
18.50	4.64	2.49	7.21
19.00	4.68	2.53	6.74
19.50	4.72	2.56	6.26
20.00	4.75	2.59	5.78
20.50	4.78	2.62 2.64	5.33
21.00	4.81	2.64	5.12
21.50	4.84		5.01
22.00	4.88	2.69	4.91
22.50	4.90	2.72	4.82
23.00	4.93	2.74	4.73
23.50	4.96	2.77	4.64
24.00	<b>4.99</b>	<b>2.79</b>	4.54
24.50	4.99	2.79	2.77
25.00	4.99	2.79	0.48
25.50	4.99	2.79	0.07
26.00	4.99	2.79	0.01
26.50	4.99	2.79	0.00
27.00	4.99	2.79	0.00
27.50	4.99	2.79	0.00
28.00	4.99	2.79	0.00
28.50	4.99	2.79	0.00
29.00	4.99	2.79	0.00
29.50	4.99	2.79	0.00
30.00	4.99	2.79	0.00
30.50	4.99	2.79	0.00
30.30	4.99	۷.۱۶	0.00

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
31.00	4.99	2.79	0.00
31.50	4.99	2.79	0.00
32.00	4.99	2.79	0.00
32.50	4.99	2.79	0.00
33.00	4.99	2.79	0.00
33.50	4.99	2.79	0.00
34.00	4.99	2.79	0.00
34.50	4.99	2.79	0.00
35.00	4.99	2.79	0.00
35.50	4.99	2.79	0.00
36.00	4.99	2.79	0.00

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### Summary for Subcatchment DA-P2: Proposed DA-P1

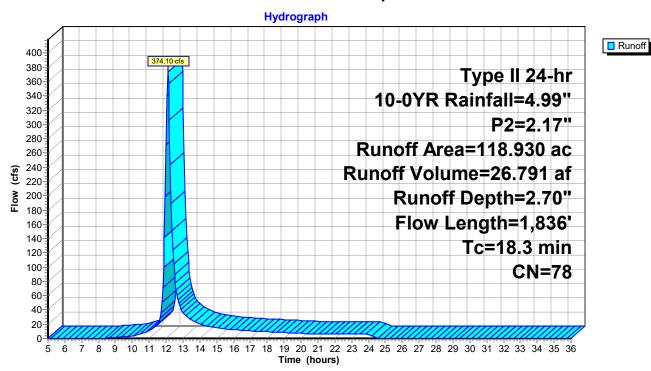
Runoff = 374.10 cfs @ 12.11 hrs, Volume= 26.791 af, Depth= 2.70"

Routed to Link DP-P2 : Design Point #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs Type II 24-hr 10-0YR Rainfall=4.99", P2=2.17"

	Area	(ac)	CN	Desc	cription		
	26.	093	77	Woo	ds, Good,	HSG D	
*	2.	080	98	Exist	ting Imper	vious	
*	1.	.040 98 Proposed Impverious					
*	0.960 98		Wate	Water Surface, HSG D			
	88.	757	78	Mea	dow, non-ເ	grazed, HS	G D
	118.930 78 Weighted Average						
	114.850			96.57% Pervious Area			
	4.080			3.43% Impervious Area			
					-		
	Тс	Lengt	:h	Slope	Velocity	Capacity	Description
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	11.6	10	0 (	0.0550	0.14		Sheet Flow,
							Grass: Dense n= 0.240 P2= 2.17"
	6.7	1,73	6 (	0.0730	4.35		Shallow Concentrated Flow,
							Unpaved Kv= 16.1 fps
	18.3	1,83	6	Total			

#### Subcatchment DA-P2: Proposed DA-P1



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### Hydrograph for Subcatchment DA-P2: Proposed DA-P1

T:	Duasia	Г	D ee
Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
5.00	0.31	0.00	0.00
5.50	0.36	0.00	0.00
6.00	0.40	0.00	0.00
6.50	0.45	0.00	0.00
7.00	0.49	0.00	0.00
7.50	0.55	0.00	0.00
8.00	0.60	0.00	0.07
8.50	0.66	0.00	0.61
9.00 9.50	0.73 0.81	0.01 0.02	1.48 2.49
10.00	0.90	0.02	3.76
10.50	1.02	0.04	6.16
11.00	1.17	0.11	10.44
11.50	1.41	0.20	20.16
12.00	3.31	1.35	271.64
12.50	3.67	1.63	89.17
13.00	3.85	1.77	35.99
13.50 14.00	3.99 4.09	1.88 1.96	26.09 20.42
14.50	4.09	2.03	17.14
15.00	4.26	2.10	15.50
15.50	4.33	2.15	13.88
16.00	4.39	2.20	12.23
16.50	4.45	2.25	11.06
17.00	4.50	2.29	10.46
17.50	4.55	2.33	9.87
18.00 18.50	4.60 4.64	2.37 2.41	9.28 8.69
19.00	4.68	2.41	8.08
19.50	4.72	2.47	7.48
20.00	4.75	2.50	6.87
20.50	4.78	2.53	6.48
21.00	4.81	2.55	6.35
21.50	4.84	2.58	6.24
22.00 22.50	4.88 4.90	2.61 2.63	6.12 6.01
23.00	4.93	2.66	5.89
23.50	4.96	2.68	5.77
24.00	4.99	2.70	5.65
24.50	4.99	2.70	0.41
25.00	4.99	2.70	0.00
25.50	4.99	2.70	0.00
26.00	4.99	2.70	0.00
26.50 27.00	4.99 4.99	2.70 2.70	0.00 0.00
27.50	4.99	2.70	0.00
28.00	4.99	2.70	0.00
28.50	4.99	2.70	0.00
29.00	4.99	2.70	0.00
29.50	4.99	2.70	0.00
30.00	4.99	2.70	0.00
30.50	4.99	2.70	0.00

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
31.00	4.99	2.70	0.00
31.50	4.99	2.70	0.00
32.00	4.99	2.70	0.00
32.50	4.99	2.70	0.00
33.00	4.99	2.70	0.00
33.50	4.99	2.70	0.00
34.00	4.99	2.70	0.00
34.50	4.99	2.70	0.00
35.00	4.99	2.70	0.00
35.50	4.99	2.70	0.00
36.00	4.99	2.70	0.00

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### Summary for Link DP-E1: Design Point #1

Inflow Area = 92.840 ac, 4.35% Impervious, Inflow Depth = 2.79" for 10-0YR event

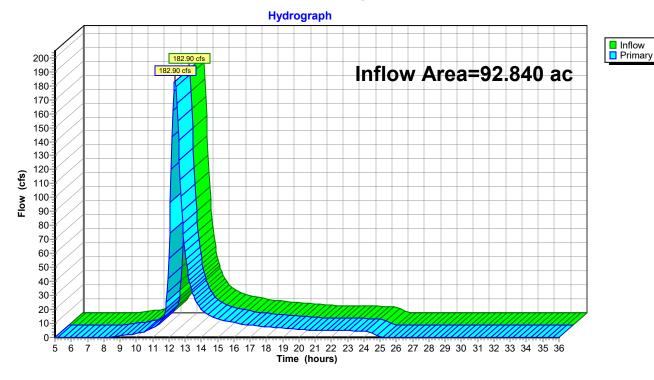
Inflow = 182.90 cfs @ 12.38 hrs, Volume= 21.609 af

Primary = 182.90 cfs @ 12.38 hrs, Volume= 21.609 af, Atten= 0%, Lag= 0.0 min

Routed to Link T-E: Total Existing

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

#### Link DP-E1: Design Point #1



(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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## Hydrograph for Link DP-E1: Design Point #1

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00	0.00	0.00	0.00
5.50	0.00	0.00	0.00
6.00	0.00	0.00	0.00
6.50	0.00	0.00	0.00
7.00 7.50	0.00	0.00 0.00	0.00
7.50 8.00	0.00	0.00	0.00 0.09
8.50	0.09	0.00	0.09
9.00	1.00	0.00	1.00
9.50	1.79	0.00	1.79
10.00	2.62	0.00	2.62
10.50	4.02	0.00	4.02
11.00	6.47	0.00	6.47
11.50	11.28	0.00	11.28
12.00	56.59	0.00	56.59
12.50	167.33	0.00	<b>167.33</b> 61.20
13.00 13.50	61.20 30.15	0.00 0.00	30.15
14.00	20.15	0.00	20.15
14.50	15.44	0.00	15.44
15.00	13.34	0.00	13.34
15.50	11.93	0.00	11.93
16.00	10.61	0.00	10.61
16.50	9.38	0.00	9.38
17.00	8.67	0.00	8.67
17.50	8.16	0.00	8.16
18.00 18.50	7.69 7.21	0.00 0.00	7.69 7.21
19.00	6.74	0.00	6.74
19.50	6.26	0.00	6.26
20.00	5.78	0.00	5.78
20.50	5.33	0.00	5.33
21.00	5.12	0.00	5.12
21.50	5.01	0.00	5.01
22.00	4.91	0.00	4.91
22.50	4.82	0.00	4.82
23.00	4.73	0.00	4.73
23.50 24.00	4.64 4.54	0.00 0.00	4.64 4.54
24.50	2.77	0.00	2.77
25.00	0.48	0.00	0.48
25.50	0.07	0.00	0.07
26.00	0.01	0.00	0.01
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50	0.00	0.00	0.00
28.00	0.00	0.00	0.00
28.50	0.00	0.00 0.00	0.00 0.00
29.00 29.50	0.00 0.00	0.00	0.00
30.00	0.00	0.00	0.00
30.50	0.00	0.00	0.00
-			

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## Summary for Link DP-E2: Design Point #2

Inflow Area = 118.930 ac, 2.71% Impervious, Inflow Depth = 2.70" for 10-0YR event

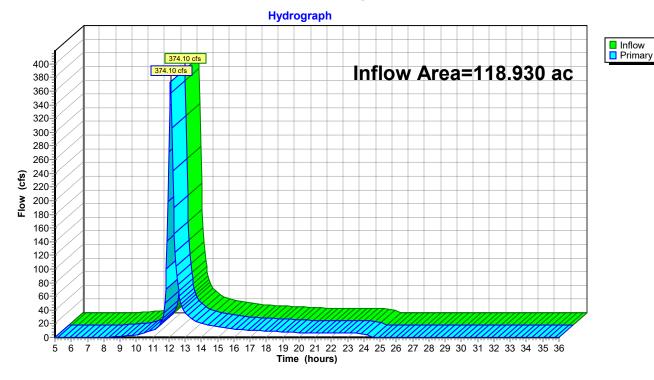
Inflow = 374.10 cfs @ 12.11 hrs, Volume= 26.791 af

Primary = 374.10 cfs @ 12.11 hrs, Volume= 26.791 af, Atten= 0%, Lag= 0.0 min

Routed to Link T-E: Total Existing

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

## Link DP-E2: Design Point #2



(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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## Hydrograph for Link DP-E2: Design Point #2

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00	0.00	0.00	0.00
5.50 6.00	0.00 0.00	0.00 0.00	0.00 0.00
6.50	0.00	0.00	0.00
7.00	0.00	0.00	0.00
7.50 8.00	0.00 0.07	0.00 0.00	0.00 0.07
8.50	0.61	0.00	0.61
9.00	1.48	0.00	1.48
9.50 10.00	2.49 3.76	0.00 0.00	2.49 3.76
10.50	6.16	0.00	6.16
11.00	10.44	0.00	10.44
11.50 12.00	20.16 <b>271.64</b>	0.00 0.00	20.16 <b>271.64</b>
12.50	89.17	0.00	89.17
13.00	35.99	0.00	35.99
13.50 14.00	26.09 20.42	0.00 0.00	26.09 20.42
14.50	17.14	0.00	17.14
15.00	15.50	0.00	15.50
15.50 16.00	13.88 12.23	0.00 0.00	13.88 12.23
16.50	11.06	0.00	11.06
17.00	10.46	0.00	10.46
17.50 18.00	9.87 9.28	0.00 0.00	9.87 9.28
18.50	8.69	0.00	8.69
19.00	8.08	0.00	8.08
19.50 20.00	7.48 6.87	0.00 0.00	7.48 6.87
20.50	6.48	0.00	6.48
21.00	6.35	0.00	6.35
21.50 22.00	6.24 6.12	0.00 0.00	6.24 6.12
22.50	6.01	0.00	6.01
23.00	5.89	0.00	5.89
23.50 24.00	5.77 5.65	0.00 0.00	5.77 5.65
24.50	0.41	0.00	0.41
25.00	0.00	0.00	0.00
25.50 26.00	0.00 0.00	0.00 0.00	0.00 0.00
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50 28.00	0.00 0.00	0.00 0.00	0.00 0.00
28.50	0.00	0.00	0.00
29.00	0.00	0.00	0.00
29.50 30.00	0.00 0.00	0.00 0.00	0.00 0.00
30.50	0.00	0.00	0.00

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## Summary for Link DP-P1: Design Point #1

Inflow Area = 92.840 ac, 4.36% Impervious, Inflow Depth = 2.79" for 10-0YR event

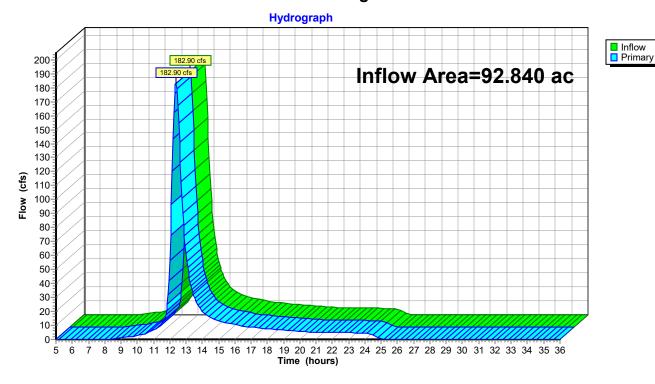
Inflow = 182.90 cfs @ 12.38 hrs, Volume= 21.609 af

Primary = 182.90 cfs @ 12.38 hrs, Volume= 21.609 af, Atten= 0%, Lag= 0.0 min

Routed to Link T-P: Total Proposed

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

### Link DP-P1: Design Point #1



(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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## Hydrograph for Link DP-P1: Design Point #1

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00	0.00	0.00	0.00
5.50 6.00	0.00 0.00	0.00 0.00	0.00 0.00
6.50	0.00	0.00	0.00
7.00	0.00	0.00	0.00
7.50 8.00	0.00 0.09	0.00 0.00	0.00 0.09
8.50	0.09	0.00	0.09
9.00	1.00	0.00	1.00
9.50 10.00	1.79 2.62	0.00 0.00	1.79 2.62
10.50	4.02	0.00	4.02
11.00	6.47	0.00	6.47
11.50 12.00	11.28 <b>56.59</b>	0.00 0.00	11.28 <b>56.59</b>
12.50	167.33	0.00	167.33
13.00	61.20	0.00	61.20
13.50 14.00	30.15 20.15	0.00 0.00	30.15 20.15
14.50	15.44	0.00	15.44
15.00	13.34	0.00	13.34
15.50 16.00	11.93 10.61	0.00 0.00	11.93 10.61
16.50	9.38	0.00	9.38
17.00	8.67	0.00	8.67
17.50 18.00	8.16 7.69	0.00 0.00	8.16 7.69
18.50	7.09	0.00	7.09
19.00	6.74	0.00	6.74
19.50 20.00	6.26 5.78	0.00 0.00	6.26 5.78
20.50	5.73	0.00	5.33
21.00	5.12	0.00	5.12
21.50 22.00	5.01 4.91	0.00 0.00	5.01 4.91
22.50	4.82	0.00	4.82
23.00	4.73	0.00	4.73
23.50 24.00	4.64 4.54	0.00 0.00	4.64 4.54
24.50	2.77	0.00	2.77
25.00	0.48	0.00	0.48
25.50 26.00	0.07 0.01	0.00 0.00	0.07 0.01
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50 28.00	0.00 0.00	0.00 0.00	0.00 0.00
28.50	0.00	0.00	0.00
29.00	0.00	0.00	0.00
29.50 30.00	0.00 0.00	0.00 0.00	0.00 0.00
30.50	0.00	0.00	0.00

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## Summary for Link DP-P2: Design Point #1

Inflow Area = 118.930 ac, 3.43% Impervious, Inflow Depth = 2.70" for 10-0YR event

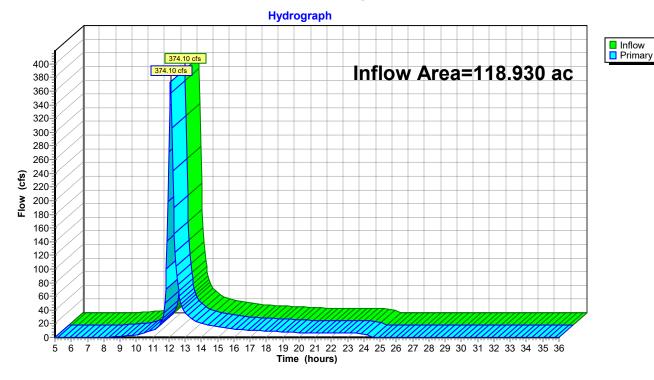
Inflow = 374.10 cfs @ 12.11 hrs, Volume= 26.791 af

Primary = 374.10 cfs @ 12.11 hrs, Volume= 26.791 af, Atten= 0%, Lag= 0.0 min

Routed to Link T-P: Total Proposed

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

## Link DP-P2: Design Point #1



(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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## Hydrograph for Link DP-P2: Design Point #1

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00	0.00	0.00	0.00
5.50	0.00	0.00	0.00
6.00 6.50	0.00 0.00	0.00 0.00	0.00 0.00
7.00	0.00	0.00	0.00
7.50	0.00	0.00	0.00
8.00	0.07	0.00	0.07
8.50	0.61	0.00	0.61
9.00 9.50	1.48 2.49	0.00 0.00	1.48 2.49
10.00	3.76	0.00	3.76
10.50	6.16	0.00	6.16
11.00	10.44	0.00	10.44
11.50	20.16	0.00	20.16
12.00 12.50	271.64 89.17	0.00 0.00	271.64 89.17
13.00	35.99	0.00	35.99
13.50	26.09	0.00	26.09
14.00	20.42	0.00	20.42
14.50	17.14	0.00	17.14
15.00 15.50	15.50 13.88	0.00	15.50 13.88
16.00	12.23	0.00 0.00	12.23
16.50	11.06	0.00	11.06
17.00	10.46	0.00	10.46
17.50	9.87	0.00	9.87
18.00	9.28	0.00	9.28
18.50 19.00	8.69 8.08	0.00 0.00	8.69 8.08
19.50	7.48	0.00	7.48
20.00	6.87	0.00	6.87
20.50	6.48	0.00	6.48
21.00 21.50	6.35	0.00	6.35
21.50	6.24 6.12	0.00 0.00	6.24 6.12
22.50	6.01	0.00	6.01
23.00	5.89	0.00	5.89
23.50	5.77	0.00	5.77
24.00	5.65	0.00	5.65
24.50 25.00	0.41 0.00	0.00 0.00	0.41 0.00
25.50	0.00	0.00	0.00
26.00	0.00	0.00	0.00
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50 28.00	0.00 0.00	0.00 0.00	0.00 0.00
28.50	0.00	0.00	0.00
29.00	0.00	0.00	0.00
29.50	0.00	0.00	0.00
30.00	0.00	0.00	0.00
30.50	0.00	0.00	0.00
			1

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## **Summary for Link T-E: Total Existing**

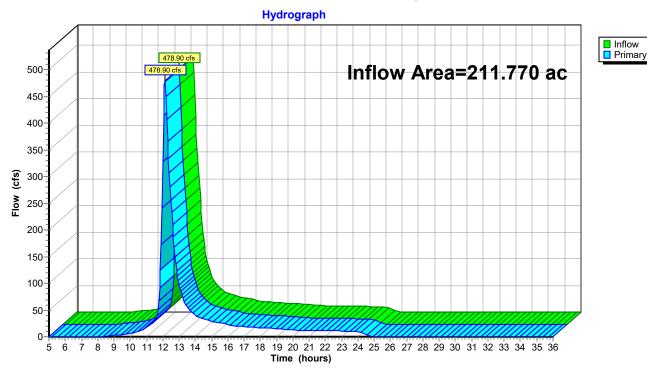
Inflow Area = 211.770 ac, 3.43% Impervious, Inflow Depth = 2.74" for 10-0YR event

Inflow = 478.90 cfs @ 12.14 hrs, Volume= 48.400 af

Primary = 478.90 cfs @ 12.14 hrs, Volume= 48.400 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

## **Link T-E: Total Existing**



(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

 $0.00 \\ 0.00$ 

0.00

0.00

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## Hydrograph for Link T-E: Total Existing

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)
5.00	0.00	0.00	0.00
5.50 6.00	0.00 0.00	0.00 0.00	0.00 0.00
6.50	0.00	0.00	0.00
7.00	0.00	0.00	0.00
7.50	0.00	0.00	0.00
8.00	0.16	0.00	0.16
8.50	1.03	0.00	1.03
9.00	2.47	0.00	2.47
9.50	4.28	0.00	4.28
10.00	6.38	0.00	6.38
10.50	10.17	0.00	10.17
11.00 11.50	16.91 31.44	0.00 0.00	16.91 31.44
12.00	328.23	0.00	328.23
12.50	256.50	0.00	256.50
13.00	97.19	0.00	97.19
13.50	56.24	0.00	56.24
14.00	40.58	0.00	40.58
14.50	32.58	0.00	32.58
15.00	28.84	0.00	28.84
15.50	25.81	0.00	25.81
16.00 16.50	22.84 20.44	0.00 0.00	22.84 20.44
17.00	19.13	0.00	19.13
17.50	18.03	0.00	18.03
18.00	16.97	0.00	16.97
18.50	15.90	0.00	15.90
19.00	14.82	0.00	14.82
19.50	13.73	0.00	13.73
20.00	12.64	0.00	12.64
20.50 21.00	11.81 11.47	0.00 0.00	11.81 11.47
21.50	11.47	0.00	11.47
22.00	11.04	0.00	11.04
22.50	10.83	0.00	10.83
23.00	10.62	0.00	10.62
23.50	10.41	0.00	10.41
24.00	10.19	0.00	10.19
24.50	3.18	0.00	3.18
25.00	0.48	0.00	0.48
25.50	0.07	0.00	0.07
26.00 26.50	0.01 0.00	0.00 0.00	0.01 0.00
27.00	0.00	0.00	0.00
27.50	0.00	0.00	0.00
28.00	0.00	0.00	0.00
28.50	0.00	0.00	0.00
29.00	0.00	0.00	0.00
29.50	0.00	0.00	0.00
30.00	0.00	0.00	0.00
30.50	0.00	0.00	0.00
			I

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## Summary for Link T-P: Total Proposed

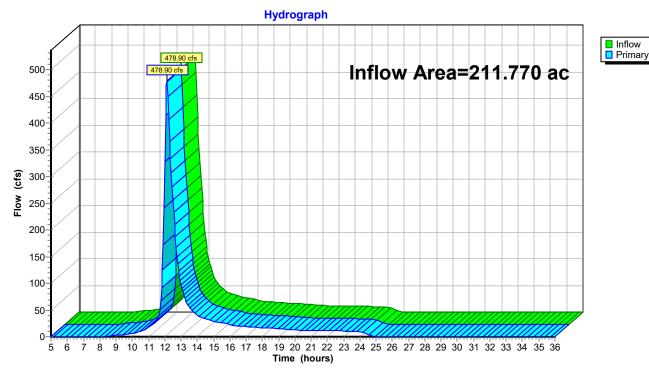
Inflow Area = 211.770 ac, 3.84% Impervious, Inflow Depth = 2.74" for 10-0YR event

Inflow = 478.90 cfs @ 12.14 hrs, Volume= 48.400 af

Primary = 478.90 cfs @ 12.14 hrs, Volume= 48.400 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-36.00 hrs, dt= 0.05 hrs

## **Link T-P: Total Proposed**



(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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## Hydrograph for Link T-P: Total Proposed

Time (hours)

31.00

31.50

32.00

32.50

33.00 33.50

34.00

34.50

35.00

35.50

36.00

Inflow

(cfs)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

Elevation

(feet)

0.00

0.00

0.00

0.00

0.00

0.00

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0.00

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<b>T</b> :	lfl	ГI	D.: 1
Time (hours)	Inflow (cfs)	Elevation (feet)	Primary (cfs)
5.00	0.00	0.00	0.00
5.50	0.00	0.00	0.00
6.00	0.00	0.00	0.00
6.50	0.00	0.00	0.00
7.00 7.50	0.00 0.00	0.00 0.00	0.00 0.00
8.00	0.00	0.00	0.00
8.50	1.03	0.00	1.03
9.00	2.47	0.00	2.47
9.50	4.28	0.00	4.28
10.00 10.50	6.38 10.17	0.00 0.00	6.38 10.17
11.00	16.17	0.00	16.91
11.50	31.44	0.00	31.44
12.00	328.23	0.00	328.23
12.50	256.50	0.00	256.50
13.00 13.50	97.19 56.24	0.00 0.00	97.19 56.24
14.00	40.58	0.00	40.58
14.50	32.58	0.00	32.58
15.00	28.84	0.00	28.84
15.50	25.81	0.00	25.81
16.00 16.50	22.84 20.44	0.00 0.00	22.84 20.44
17.00	19.13	0.00	19.13
17.50	18.03	0.00	18.03
18.00	16.97	0.00	16.97
18.50	15.90	0.00	15.90
19.00 19.50	14.82	0.00 0.00	14.82
20.00	13.73 12.64	0.00	13.73 12.64
20.50	11.81	0.00	11.81
21.00	11.47	0.00	11.47
21.50	11.25	0.00	11.25
22.00	11.04	0.00	11.04
22.50 23.00	10.83 10.62	0.00 0.00	10.83 10.62
23.50	10.41	0.00	10.41
24.00	10.19	0.00	10.19
24.50	3.18	0.00	3.18
25.00 25.50	0.48	0.00	0.48
26.00	0.07 0.01	0.00 0.00	0.07 0.01
26.50	0.00	0.00	0.00
27.00	0.00	0.00	0.00
27.50	0.00	0.00	0.00
28.00 28.50	0.00 0.00	0.00 0.00	0.00 0.00
28.50 29.00	0.00	0.00	0.00
29.50	0.00	0.00	0.00
30.00	0.00	0.00	0.00
30.50	0.00	0.00	0.00
			I



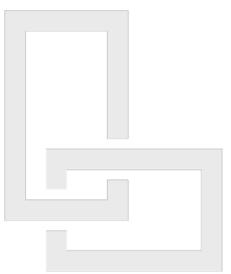
# APPENDIX E: SWPPP INSPECTION REPORT (SAMPLE FORM)



Prepared by: LaBella Associates Choose an item. Choose an item. Choose an item.



SWPPP INSPECTION REPORT NUMBER XX
CLIENT NAME
PROJECT NAME
PROJECT ADDRESS, TOWN OF X, X COUNTY, NY
PERMIT NUMBER



Performed: Report Issued:	2/1/2025 @ 12:00 AM 2/1/2025
Status: SATISFACTORY (All erosion co working order)	ontrol measures are installed and in
Qualified Inspector (name and title)	Qualified Professional (name and title)
Date	Date
Signature	Signature

## **NYSDEC Documentation and SWPPP Forms**

**5-Acre Waiver:** 5-acre waiver approved by NYSDEC and > 5 acres disturbed

303d Status: Project does not directly discharge to a 303d impaired waterbody

Number of Inspections required: 1 / week

Location of SWPPP and Site Log Book on-site:

YES	NO	N/A	CONTAINED IN SITE LOG BOOK?
			Preconstruction Assessment
			Copy of eNOI
			Letter of Authorization
			SWPPP Preparer Certification Form
			Owner/Operator Certification Form
			MS4 SWPPP Acceptance Form
			MS4 No Jurisdiction Form
			NYCDEP SWPPP Acceptance Form
			Contractor and Subcontractor Certifications
			SPDES General Permit
			5 Acre Waiver Authorization
			eNOT
Comm	ents:		

## **Site Conditions**

Total area with inactive soil d Total area that has achieved	otal area with active soil disturbance (not requiring either temporary or final stabilization): XX Acres otal area with inactive soil disturbance (requiring either temporary or final stabilization): XX Acres otal area that has achieved temporary stabilization: XX Acres otal area that has achieved final stabilization: XX Acres					
Allowable Disturbed Area Per	Allowable Disturbed Area Per NOI and/or 5-acre waiver: XX Acres					
Current Status of Construction: Description						
Weather Conditions: Conditions Temperature: XX °F Soil Conditions: Choose an item.						
Description of Discharge	Condition of	Sediment		Corrective Action		

Point/Surface Waters of the State	Runoff	Discharge Noted Y/N	Corrective Action

## **Erosion and Sediment Control Deficiencies and Corrective Actions**

SWPPP Component	Functional Y/N/NA	Deficiency (See Checklist and/or note)	Deficiency Location	Initial Date	Corrective Action	Corrected Y / N
General Site Conditions						
Silt Fence						
Stabilized Construction Access						
Compost Filter Sock						
Inlet Protection						
Soil Stockpiles						
Temporary Stabilization						
Permanent Stabilization						
Dewatering Operations						
Stone Check Dams						
Rock Outlet Protection						
Sediment Traps and Basins						
Temporary Stream Crossing						
Pavement Sweeping						
Concrete Washout						
Filter Strips						
Slope Protection Measures						
Temporary Swales and Berms						
Temporary Parking Areas						
Fiber Roll						
Permanent Turf Reinforcement						
Water Bars						
Flow Diffusers						
Other:						

### **SWPPP Inspection Checklist and Deficiency Numbers**

#### 1 General Site Conditions

- 1A Adjoining properties are not protected from erosion and sediment deposition
- 1B Downstream waterways are not protected from erosion and sediment deposition
- 1C All E&SC measures have not been constructed as detailed in the SWPPP
- 1D Dust is not adequately controlled
- 1E Storage areas contain spills, leaks, or harmful materials
- 1F Garbage and waste building materials are not being managed properly
- 1G Temporary control measures that are no longer needed have not been removed
- 1H Permanent SWM practices not constructed per plans

#### 2 Silt Fence

- 2A Silt fence not installed on contour
- 2B Silt fence not across conveyance channels
- 2C Silt fence not at least 10 feet from toe of slope
- 2D Silt fence not at appropriate spacing intervals based on slope
- 2E Silt fence ends are not wrapped for continuous support
- 2F Silt fence fabric is loose or contains rips or frayed areas
- 2G Silt fence posts are unstable
- 2H Silt fence is not buried 6 inches minimum
- 2I Silt fence contains bulges or material buildup

#### 3 Stabilized Construction Access

- 3A Temporary construction access not installed or not per NYS standards
- 3B Other access areas have not been stabilized immediately as work takes place
- 3C Sediment has tracked onto public streets and is not being cleaned daily
- 3D Stone is not clean enough to effectively remove mud from vehicles
- 3E Adequate drainage not provided to prevent ponding

#### 4 Compost Filter Sock

- 4A Filter sock not installed on contour
- 4B Filter sock terminal ends do not extended 8' upslope at 45° angle
- 4C Inappropriate diameter based on slope steepness and slope length
- 4D Filter sock not anchored at 10' intervals
- 4E More than 50% sediment has built up

#### 5 Inlet Protection

- 5A Inlet protection not installed or installation is not per SWPPP or Blue Book specifications
- 5B Incorrect type(s) of inlet control installed or is inappropriate for location
- 5C Drainage area for inlet protection is greater than 1 acre
- 5D Sediment has not been removed when 50% of storage volume has been achieved
- 5E A 2" x 4" wood frame and wood posts has not been installed
- Filter fabric is not buried a minimum of 1 foot below ground or secured to frame/posts
- 5G Posts are unstable, fabric is loose, and contains rips or frayed areas
- 5H Post spacing exceeds maximum 3' spacing

#### 6 Soil Stockpiles

6A No sediment controls at downhill slope

#### 7 Temporary Stabilization

- 7A Areas inactive for 14 days or more have not been stabilized (If <5 acres disturbed)
- 7B Areas inactive for 7 days or more have not been stabilized (If >5 acres disturbed or 303d)
- 7C Soil preparation has not been applied as specified in the SWPPP or the Blue Book
- 7D Rolled EC products specified for steep slopes or channels have not been installed

#### 8 Permanent Stabilization

- 8A Lawn in disturbed areas has not been established to 80% germination
- 8B Soil preparation has not been applied as specified in the SWPPP or the Blue Book
- 8C Rolled EC products specified for steep slopes or channels have not been installed

#### 9 Dewatering Operations

- 9A Upstream and downstream berms are not installed or functioning poorly
- 9B Clean water from upstream pool is not being pumped to the downstream pool
- 9C Sediment laden water from work area is not being discharged to a silt-trapping device
- 9D Groundwater from excavations managed improperly (No sumps/sediment control)

#### 10 Stone Check Dam

- 10A Not installed per standards
- 10B Channel is unstable (flow is eroding soil underneath or around the structure)
- 10C Check dam in poor condition (rocks not in place or lack of geotextile fabric)
- 10D Sediment needs to be removed

#### 11 Rock Outlet Protection

- 11A Rock outlet protection not installed per plan or Blue Book
- 11B Rock outlet protection not installed concurrently with pipe installation

#### 12 Sediment Traps and Basins

- 12A Outlet structure constructed improperly
- 12B Geotextile fabric has not been placed beneath rock fill
- 12C Depth of sediment in basin has exceeded allowable threshold
- 12D Basin and outlet structure not constructed per the approved plan
- 12E Basin side slopes are not stabilized with seed/mulch
- 12F More than 50% capacity has built up

#### 13 Temporary Stream Crossing

.3A Construction crossings at concentrated flow areas have not been culverted

#### 14 Pavement Sweeping

14A Pavement has not been swept daily and sediment has traveled into road

## Stormwater Management Practice Deficiencies and Corrective Actions

Practice	Sign Y/N	Current Phase of Construction	Items Not in Conformance with SWPPP	Deficiency Location	Initial Date	Corrective Action	Corrected Y / N
Practice 1:							
Practice 2:							
Practice 3:							
Practice 4:							
Practice 5:							
Practice 6:							

## **Deficiencies and Corrective Action Photo Log**

Dhoto 1	Dhoto 14
Photo 1	Photo 1A
Date – Deficiency in need of repair or maintenance:	Date - Corrective Action:
Bate Bandianay in mode of ropair of maintenance.	Butto Corrective Notion.
Photo 2	Photo 2A
Date – Deficiency in need of repair or maintenance:	Date – Corrective Action:
Photo 3	Photo 3A
Date – Deficiency in need of repair or maintenance:	Date – Corrective Action:

## <u>Deficiencies and Corrective Action Photo Log (continued)</u>

Dhoto 1	Dhoto 44
Photo 4	Photo 4A
Date – Deficiency in need of repair or maintenance:	Date – Corrective Action:
Bate Bonding in nood of ropair of maintenancer	Batto Confedera Materia
Photo 5	Photo 5A
Date – Deficiency in need of repair or maintenance:	Date – Corrective Action:
Photo 6	Photo 6A
Data Deficiency in need of reneix as maintenance	Doto Corrective Actions
Date – Deficiency in need of repair or maintenance:	Date – Corrective Action:

## <u>Disturbance / Photo Location Map</u>

Replace this page to include an 11x17 erosion control plan sketch to scale showing:

- 1. Areas with active soil disturbance activity
- 2. Areas that have been disturbed but are inactive at the time of the inspection
- 3. Areas that have been stabilized (temporary and/or final) since the last inspection
- 4. Limit of disturbance line per the SWPPP and the grading plan
- 5. Photo locations

Use Bluebeam template with standard colors to indicate limits



# APPENDIX F: POST-CONSTRUCTION INSPECTIONS AND MAINTENANCE



# Rainwater Harvesting (Rain Barrels or Cisterns)

	Table 2	3.1 RWH Conveyance System and Filter
Problem (Check if Present)		Follow-Up Actions
	Leaves, sticks, or other debris in gutters and downspouts	☐ Remove all debris by hand. ☐ Other:
☐ Leaves, sticks, or other debris in filter(s) —		<ul> <li>□ Clean out all debris and organic matter buildup by hand or by spraying with a hose.</li> <li>□ Other:</li> </ul>
		Kick-Out to Level 2 Inspection: Filter (first-flush diverter or vortex filter outside the tank) does not seem to be operating, is completely clogged, or does not appear to be trapping any debris.
	Loose or disconnected junctions between gutters, pipes, or filters	<ul> <li>Secure any loose junctions or parts and make sure they are properly sealed to prevent leaks,</li> <li>Other:</li> </ul>
		Table 2.3.2 RWH Storage Tank
Pro	blem (Check if Present)	Follow-Up Actions
	Tank is above ground and not freeze proof	Winterize the tank by performing the following steps:  ☐ Drain down water level in the tank before winter to avoid damage from freezing temperatures.  ☐ Drain water from pipes and pumps.  ☐ Disconnect conveyance pipes from the tank to enable roof runoff to bypass the tank during winter.
	Tank is full between rain events (harvested water is not being used).	Drain down any remaining water in the tank before predicted rain events.
	Mosquito larvae or other insects present in the water	<ul> <li>□ Add mosquito dunks to water.</li> <li>□ Ensure that insect screens are installed on all openings and are properly sealed (inlet and outlets).</li> <li>□ Other:</li> </ul>
	Debris, algae, or organic matter	Remove as much as possible, by hand. Other:
accumulated in tank	□ Kick-Out to Level 2 Inspection: For large tanks that cannot easily be accessed for inspection and/or cleaning, defer to Level 2 Inspection.	
	Tank does not appear to fill fully even during large rains, or water level drops quickly after filling.	☐ Kick-Out to Level 2 Inspection: Water is bypassing the tank and/or there are leaks in the tank wall. This will likely require special expertise to diagnose and fix.
	Problems with pumps, filters, or other mechanical components	☐ Kick-Out to Level 2 Inspection: This will likely require special expertise to diagnose and fix.

Pro	blem (Check if Present)	Follow-Up Actions		
	Slow flow from outlet caused	<ul> <li>☐ If clogging seems to be the problem, ream out sediment from valve if this can be done from exterior.</li> <li>☐ Other:</li> </ul>		
	by faulty or clogged valve	Kick-Out to Level 2 Inspection: Valve needs to be replaced or cannot be cleaned out from outside of tank.		
	Flow from outlet is backing up toward building foundation.	<ul> <li>Add flexible pipe to end of outlet pipe to divert flow further away and downhill from building.</li> </ul>		
	Erosion or drainage issues at	Add a gravel and/or stone pad to reduce the impact from the water flowing out of the outlet pipe during storms.      Other:		
outlet	utlet	Kick-Out to Level 2 Inspection: Rills have formed, erosion or drainage problems are more severe or cannot be resolved, or there is discoloration or other unusual conditions around the outlet.		

# Disconnection and Sheetflow (Rooftop Disconnection, Filter Strip, Riparian Buffer)

Problem (Check if Present)			Follow-Up Actions
	П	Changes in flow; more runoff; runoff bypassing the practice	For rooftop areas, make sure downspouts are still disconnected and conveying water into the treatment area.  Look for and remove any "dams" of sediment and grass clippings that prevent water from entering the treatment area as sheet flow.  Other:  Kick-Out to Level 2 Inspection: Changes to drainage area size or amount of runoff due to construction, tillage, etc.
	For parking lots in the drainage area—sediment, grass clippings, or other debris has accumulated at pavement edge.	For small, isolated amounts of debris, sweep up by hand and dispose properly so that it will not be exposed to runoff.  Other:	
		Kick-Out to Level 2 Inspection: Sediment is widespread and cannot be removed by manual sweeping.	
		For parking lots in the drainage area—dips or damage at pavement edge caused flow to concentrate.	Kick-Out to Level 2 Inspection: This will likely require special expertise to diagnose and fix pavement edge.

Problem (Check if Present)			Follow-Up Actions
		Debris and/or sediment accumulated behind or around the level spreader.	Remove debris and sediment by hand and ensure that the area behind the level spreader is relatively flat. Too much debris and sediment can cause runoff to bypass the level spreader structure.  Other:
		Sinking, cracking,	For stone/gravel spreaders, add new material or rake out as needed to make it even.  Other:
	Z.	sloughing, or other structural problem makes the energy dissipator no longer level.	☐ Kick-Out to Level 2 Inspection: Structural issues that cannot be easily fixed by hand

Table 2.4.3 D&S Ti	reatment Area	
Problem (Check if Present)	Follow-Up Actions	
☐ Trash and/or debris in the treatment area	☐ Collect trash/debris and dispose of properly.	
Grass filter strip has grown very tall, to the point that runoff cannot easily enter or is getting concentrated.	Mow filter strip twice a year or more frequently in a residential yard.	
☐ Sparse vegetation or bare spots	<ul> <li>For grassy areas, add topsoil (as needed), grass seed mulch, and water during the growing season to reestablish consistent vegetation cover.</li> <li>Other:</li> </ul>	
	□ For minor rills, fill in with soil, compact, and add seed and straw to establish vegetation.     □ Other:	
Rills or gullies are forming in treatment area where flow has become concentrated	☐ Kick-Out to Level 2 Inspection: Rills are more than 2" to 3" deep and require more than just hand raking and re-seeding.	

# Swales (Vegetated Swale, Wet Swale)

Table 2.5.1 SW Drainage Area			
Problem (Check if Present)	Follow-Up Actions		
Bare soil, erosion of the ground (rills washing out the dirt)	Seed and mulch or sod areas of bare soil to establish vegetation.  Fill in erosion areas with soil, compact, and add seed and straw to establish vegetation.  If a rill or small channel is forming, try to redirect water flowing to this area by creating a small berm or adding topsoil to areas that are heavily compacted.  Other:  Kick-Out to Level 2 Inspection: Large areas of soil have been eroded, or larger channels are forming. May require rerouting of flow paths		
Piles of grass clippings, mulch, dirt, salt, or other materials	□ Remove or cover piles of grass clippings, mulch, dirt, etc. □ Other:		
☐ Open containers of oil, grease, paint, or other substances	Cover or properly dispose of materials; consult your local solid waste authority for guidance on materials that may be toxic or hazardous.		
	Kick-Out to Level 2 Inspection: Grass on edge of pavement continues to die off for unknown reasons. Swale edge may need to be replaced with other materials (e.g., stone diaphragm).		
	□ Seed and mulch; add topsoil or compost if needed. □ Other:		
☐ Grass dying at edge of road	Kick-Out to Level 2 Inspection: Grass on edge of pavement continues to die off for unknown reasons. Swale edge may need to be replaced with other materials (e.g., stone diaphragm).		

Table 2.5.2 SW Inlets			
Problem (Check if Present)	Follow-Up Actions		
□ Inlets or the swale edge are collecting grit, grass clippings, or debris or have grass/weeds growing. Some water may not be getting into the swale. The objective is to have a clear pathway for water to flow into the swale.	<ul> <li>Use a flat shovel to remove grit and debris (especially at curb inlets or opening). Parking lots will generate fine grit that will accumulate at these spots.</li> <li>Pull out clumps of growing grass or weeds, and scoop out the soil or grit that the plants are growing in.</li> <li>Remove any grass clippings, leaves, sticks, and other debris that is collecting at inlets or along the edge of the swale where water is supposed to enter.</li> <li>For pipes and ditches, remove sediment and debris that is partially blocking the pipe or ditch opening where it enters the swale.</li> <li>Dispose of all material properly in an area where it will not re-enter the swale.</li> <li>Other:</li> <li>Kick-Out to Level 2 Inspection: Inlets are blocked to the extent that most of the water does not seem to be entering the swale.</li> </ul>		
Some or all of the inlets are eroding so that rills, gullies, and other erosion are present, or there is bare dirt that is washing into the swale.	For small areas of erosion, smooth out the eroded part and apply rock or stone (e.g., river cobble) to prevent further erosion. Usually, filter fabric is placed under the rock or stone.  In some cases, reseeding and applying an erosion control matting can be used to prevent further erosion. Some of these materials may be available at a garden center, but it may be best to consult a landscape contractor.  Other:  Level 2 Inspection: Erosion is occurring at most of the inlets or along much of the swale edge. The inlet design may have to be modified.		

Problem (Check if Present)	Follow-Up Actions		
Minor areas of sediment, grit, trash, or other debris are accumulating in the swale.	Use a shovel to scoop out minor areas of sediment or grit, especially in the spring after winter sanding materials may wash in and accumulate. Dispose of the material where it cannot re-enter the swale.  If removing the material creates a hole or low area, fill with good topsoil and add seed and straw to re-vegetate.  Remove trash, vegetative debris, and other undesirable materials.  If the swale is densely vegetated, it may be difficult to do the maintenance; check for excessive ponding or other issues described in this section to see if the accumulated material is causing a problem.  Other:		
	Kick-Out to Level 2 Inspection: Sediment has accumulated more than 3 inches deep and covers 25% or more of the swale surface.      The source of sediment is unknown or cannot be controlled with simple measures.		
	Try filling the eroded areas with clean topsoil, and then seed and mulch to establish vegetation.      If the problem recurs, you may have to use some type of matting, stone (e.g., river cobble), or other material to fill in eroded areas.      If the erosion is on a side slope, fill with soil and cover with erosion-control matting or at least straw mulch after re-seeding.		
There is erosion in the bottom or on the side slopes. Water seems to be carving out rills as it flows through the swale or on the slopes.	Kick-Out to Level 2 Inspection: The problem persists or the erosion is more than 3 inches deep and seems to be an issue with how water enters and moves through the swale.  Kick-Out to Level 2 Inspection: The problem does not seem to be caused by flowing water, but a collapse or sinking of the surface (e.g., "sinkhole") due to some underground problem.		
☐ Water does not flow evenly down the length of the swale, but ponds in certain	<ul> <li>☐ If the problem is minor (just small, isolated areas), try using a metal rake or other tools to create a more even flow path; remove excessive vegetative growth, sediment, or other debris that may be blocking the flow.</li> <li>☐ Other:</li> </ul>		
areas for long periods of time (e.g., 72 hours after a storm). The swale does not seem to have "positive drainage." Check during or immediately after a rain storm.	<ul> <li>Kick-Out to Level 2 Inspection: Water ponds in more than 25% of the swale for three days or more after a storm. The issue may be with the underlying soil or the grade of the swale.</li> <li>Water ponds behind check dams for three days or more after a storm. Check dams may be clogged or not functioning properly.</li> </ul>		



downhill side, or the check dams are breaking apart or breaching.

#### Table 2.5.4 SW Vegetation Problem (Check if Present) Follow-Up Actions Mow or bush-hog the path. Other: Vegetation is too overgrown to access swale for maintenance activities If you can identify which plants are weeds or not intended to be part of the planting plan, eliminate these, preferably by hand pulling. If weeds are widespread, check with the local stormwater authority and/or Extension Office about proper use of herbicides for areas connected with the flow of water. Even vegetation that is intended to be present can become large, overgrown, block flow, and/or crowd out surrounding plants. Prune and thin accordingly. If weeds or invasive plants have overtaken the whole swale, bush-hog the entire area before seed heads form in the spring. It will be necessary to remove the root mat manually or with appropriate herbicides, as Replant with species that are aesthetically pleasing and seem to be doing well in the swale. п Kick-Out to Level 2 Inspection: You are unsure of the original planting Vegetation requires regular maintenance: design or the vegetation maintenance task is beyond your capabilities of pulling weeds, removing dead and diseased time, expertise, or resources. If you are unsure of the health of the plants, adding plants to fill in areas that are vegetation (e.g. salt damage, invasives, which plants are undesirable) or not well vegetated, etc. the appropriate season to conduct vegetation management, consult a landscape professional before undertaking any cutting, pruning, mowing, or brush hogging. The original plants are likely not suited for the actual conditions within the swale. If you are knowledgeable about plants, select and plant more appropriate vegetation (preferably native plants) so that almost the entire □ Vegetation is too thin, is not healthy, surface area will be covered by the end of the second growing season. and there are many spots that are not well vegetated. Kick-Out to Level 2 Inspection: For all but small practices (e.g., in residential yards), this task will likely require a landscape design professional or horticulturalist. Table 2.5.5 SW Outlets Follow-Up Actions Problem (Check if Present) Remove the debris and dispose of it where it cannot re-enter the swale. ☐ Other: Outlet is obstructed with mulch, sediment, debris, trash, etc. Kick-Out to Level 2 Inspection: Outlet is completely clogged or obstructed; there is too much material to remove by hand or with simple hand tools.

# Tree Planting

	Ta	ble 2.6.1 TP Watering
Pro	blem (Check if Present)	Follow-Up Actions
D	Soil is not moist to the touch and/or it has not rained in a week, and leaves/needles are starting to appear wilted/dry.	<ul> <li>□ Water trees deeply and slowly near the base. Soaker hoses and drip irrigation work best for deep watering of trees and shrubs.</li> <li>□ Other:</li> </ul>
		Table 2.6.2 TP Mulch
Prol	blem (Check if Present)	Follow-Up Actions
П	Mulch is too thin or thick (should be approximately 3" deep) or does not extend to tree canopy (or 5' radius if tree has a larger than 10' canopy reach).	Add or remove mulch around tree canopy to maximum 5' radius but no within 3" of the bark.  If mulch is against the stems or tree trunks, pull it back several inches to expose the base of the trunk and root crown.  Other:
	Tab	le 2.6.3 TP Pruning
Prol	blem (Check if Present)	Follow-Up Actions
	Presence of suckers, dead or diseased branches, branches that interfere with pedestrian traffic	<ul> <li>Selective cutting</li> <li>Prune to make the tree more aesthetically pleasing and remove disease.</li> <li>Other:</li> </ul>
	<ul> <li>Kick-Out to Level 2 Inspection: Use an arborist or landscaper for more extensive pruning jobs.</li> </ul>	

# Bioretention (Bioretention Cell, Dry Swale, Rain Garden, Stormwater Planters, Tree Pits)

Table 2.7.1 BR		
Problem (Check if Present)	Follow-Up Actions	
Bare soil, erosion of the ground (rills washing out the dirt)	Seed and mulch areas of bare soil to establish vegetation.  Fill in erosion areas with soil, compact, and seed and straw to establish vegetation.  If a rill or small channel is forming, try to redirect water flowing to this area by creating a small berm or adding topsoil to areas that are heavily compacted.  Other:  Kick-Out to Level 2 Inspection: Large areas of soil have been eroded, or larger channels are forming. May require rerouting of flow paths.	
Piles of grass clippings, mulch, dirt, salt, or other materials	□ Remove or cover piles of grass clippings, mulch, dirt, etc. □ Other:	
Open containers of oil, grease, paint, or other substances	Cover or properly dispose of materials; consult your local solid waste authority for guidance on materials that may be toxic or hazardous.  Other:	

Table 2.7.2 BR Inlets	
Problem (Check if Present)	Follow-Up Actions
	Use a flat shovel to remove grit and debris (especially at curb inlets or openings). Parking lots generate fine grit that will accumulate at these spots.
	<ul> <li>Pull out clumps of growing grass or weeds and scoop out the soil or grit that the plants are growing in.</li> </ul>
	<ul> <li>Remove any grass clippings, leaves, sticks, and other debris that is collecting at inlets.</li> </ul>
	For pipes and ditches, remove sediment and debris that is partially blocking the pipe or ditch opening where it enters the Bioretention cell.
	☐ Dispose of all material properly where it will not re-enter the Bioretention cell.
	Other:
Inlets collect grit and debris or grass/weeds. Some water may not be getting into the Bioretention cell. The objective is to have a clear pathway for water to flow into the cell.	Kick-Out to Level 2 Inspection: Inlets are blocked to the extent that most of the water does not seem to be entering the Bioretention cell.
	For small areas of erosion, smooth out the eroded part and apply rock or stone (e.g., river cobble) to prevent further erosion. Usually, filter fabric is placed under the rock or stone.
	In some cases, reseeding and applying erosion-control matting can be used to prevent further erosion. Some of these materials may be available at a garden center, but it may be best to consult a landscape contractor.
	□ Other:
Some or all of the inlets are eroding so that rills, gullies, and other erosion is present, or there is bare dirt that is washing into the Bioretention cell.	Kick-Out to Level 2 Inspection: Erosion is occurring at most of the inlets, and it looks like there is too much water that is concentrating at these points. The inlet design may have to be modified.

#### Table 2.7.3 BR Ponding Area

#### Problem (Check if Present)

#### Follow-Up Actions



- Mulch (if used) needs to be replaced or replenished. The mulch layer had decomposed or is less than 1-inch thick.
- Add new mulch to a total depth (including any existing mulch that is left) of 2 to 3 inches. The mulch should be shredded hardwood mulch that is less likely to float away during rainstorms.
- Avoid adding too much mulch so that inlets are obstructed or certain areas become higher than the rest of the Bioretention surface.
- Othor



Minor areas of sediment, grit, trash, or other debris are accumulating on the bottom.

- Use a shovel to scoop out minor areas of sediment or grit, especially in the spring after winter sanding materials may wash in and accumulate. Dispose of the material where it cannot re-enter the Bioretention cell.
- If removing the material creates a hole or low area, fill with soil mix that matches original mix and cover with mulch so that the Bioretention surface area is as flat as possible.
- Remove trash, vegetative debris, and other undesirable materials.
- Other
- Kick-Out to Level 2 Inspection: Sediment has accumulated more than 2-inches deep and covers 25% or more of the Bioretention surface.
- Kick-Out to Level 2 Inspection: The Bioretention cell is too densely vegetated to assess sediment accumulation or ponding; see BR-4, Vegetation.



- There is erosion in the bottom or on the side slopes. Water seems to be carving out rills as it flows across the Bioretention surface or on the slopes, or sinkholes are forming in certain areas.
- Source: Stormwater Maintenance, LLC.

- □ Try filling the eroded areas with clean topsoil or sand, and cover with mulch.
   □ If the problem recurs, you may have to use stone (e.g., river cobble) to fill in
- problem areas.

  If the erosion is on a side slope, fill with clay that can be compacted and
- seed and mulch the area.
- ☐ Other:
- Kick-Out to Level 2 Inspection: The problem persists or the erosion is more than 3-inches deep and seems to be an issue with how water enters and moves through the Bioretention cell.
- Kick-Out to Level 2 Inspection: The problem does not seem to be caused by flowing water, but a collapse or sinking of the surface (e.g., "sinkhole") due to some underground problem.

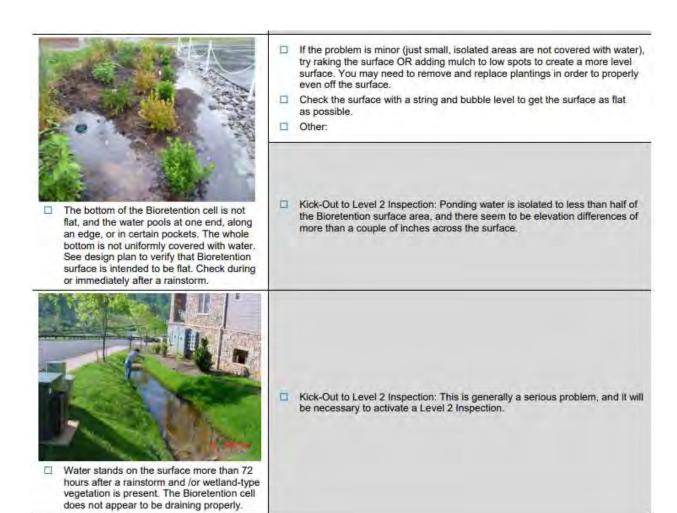


Table 2.	7.4 BR Vegetation
Problem (Check if Present)	Follow-Up Actions
Vegetation requires regular maintenance—pulling weeds, removing dead and diseased plants, replacing mulch around plants, adding plants to fill in areas that are not well vegetated, etc.	If you can identify which plants are weeds or not intended to be part of the planting plan, eliminate these, preferably by hand pulling.  If weeds are widespread, check with the local stormwater authority and/or Extension Office about proper use of herbicides for areas connected with the flow of water.  Even vegetation that is intended to be present can become large, overgrown, and/or crowd out surrounding plants. Prune and thin accordingly.  If weeds or invasive plants have overtaken the whole Bioretention cell, bush-hog the entire area before seedheads form in the spring. It will be necessary to remove the root ma manually or with appropriate herbicides, as noted above.  Re-plant with species that are aesthetically pleasing and seem to be doing well in the Bioretention cell.  Other:  Kick-Out to Level 2 Inspection: You are unsure of the original planting design, or the vegetation maintenance task is beyon your capabilities of time, expertise, or resources. If you are unsure of the health of the vegetation (e.g. salt damage, invasives, which plants are undesirable) or the appropriate season to conduct vegetation management, consult a landscape professional before undertaking any cutting, pruning, mowing, or brush hogging.
	<ul> <li>The original plants are likely not suited for the actual conditions within the Bioretention cell. If you are knowledgeable about plants, select and plant more appropriate vegetation (preferably native plants) so that almost the entire surface area will be covered by the end of the second growing season.</li> <li>Other:</li> </ul>

- Vegetation is too thin, is not healthy, and there are many spots that are not well vegetated.
- Kick-Out to Level 2 Inspection: For all but small practices (e.g., rain gardens), this task will likely require a landscape design professional or horticulturalist.

Table 2.7.5 BR Outlets	
Problem (Check if Present)	Follow-Up Actions
☐ Erosion at outlet	<ul> <li>□ Add stone to reduce the impact from the water flowing out of the outlet pipe of weir during storms.</li> <li>□ Other:</li> </ul>
	Kick-Out to Level 2 Inspection: Rills have formed and erosion problem becomes more severe.
	Remove the debris and dispose of it where it cannot re-enter the Bioretention cell.  Other:
<ul> <li>Outlet obstructed with mulch, sediment, debris, trash, etc.</li> </ul>	Kick-Out to Level 2 Inspection: Outlet is completely clogged or obstructed; there is too much material to remove by hand or with simple hand tools.

### **Green Roofs**

Ta	ble 2.8.1 GR Vegetation and Surface	
Problem (Check if Present)	Follow-Up Actions	
	<ul> <li>□ Water or irrigate.</li> <li>□ Prune or remove dead or dying vegetation.</li> <li>□ Other:</li> </ul>	
☐ Wilting or nutrient-deprived vegetation; bare areas developing on the roof	<ul> <li>Kick-Out to Level 2 Inspection: Greater than 20% plant dieoff or wilting, even after rainy periods. May require new vegetation or indicate a problem with the soil medium.</li> <li>Kick-Out to Level 2 Inspection: Yellowing vegetation may indicate a need for</li> </ul>	
	fertilizer, but do not fertilize unless explicitly included in the management plan or with a Level 2 Inspection.  Kick-Out to Level 2 Inspection: Bare areas with no vegetation growing. These may become weed problems in the future.	
The second secon	Remove weeds by hand. Apply lime to kill moss. Other:	
	Kick-Out to Level 2 Inspection: Weeds cover more than 25% of the surface, or the original planting plan has been compromised.	
□ Weeds or moss		
☐ Ponding between storm events	Kick-Out to Level 2 Inspection: Surface ponding more than 24 hours after a storm event presents a hazard and needs to be addressed immediately.	
7	able 2.8.2 GR Overflows and Drains	
Problem (Check if Present)	Follow-Up Actions	
☐ Inspection port for roof drainage (can be	Remove debris by hand or flush through with a hose.  Other:	
with debris)	Kick-Out to Level 2 Inspection: Debris cannot be removed, or it appears that debris has accumulated in the underdrains.	
☐ Damage to other roof drainage structures (e.g., roof scuppers)	Call contractor or individual in charge of regular building maintenance. This is a building maintenance issue.	

## Permeable Pavement (Permeable Pavers, Porous Asphalt/Concrete)

Table 2.9.1 PP Drainage Area		
Problem (Check if Present)		Follow-Up Actions
	Bare soil, erosion of the ground (rills washing out the dirt)	<ul> <li>Seed and straw areas of bare soil to establish vegetation.</li> <li>Fill in erosion areas with soil, compact, and seed and straw to establish vegetation.</li> <li>If a rill or small channel is forming, try to redirect water flowing to this area by creating a small berm or adding topsoil to areas that are heavily compacted.</li> <li>Other:</li> </ul>
		Kick-Out to Level 2 Inspection: Large areas of soil have been eroded, or larger channels are forming. May require rerouting of flow paths.
	☐ Piles of grass clippings, mulch, dirt, salt, or other materials	<ul> <li>□ Remove or cover piles of grass clippings, mulch, dirt, etc.</li> <li>□ Other:</li> </ul>
	Open containers of oil, grease, paint, or other substances	Cover or properly dispose of materials; consult your local solid waste authority for guidance on materials that may be toxic or hazardous.  Other:

Table 2.9.2 PP Surface		
Problem (Check if Present)	Follow-Up Actions	
	For small areas (e.g., driveways, patios), try a leaf blower or sweep the area to remove the dirt/grit from the Permeable pavement and properly dispose of the material.  If dirt/grit remain in the joint areas between paver blocks, agitate with a rough brush and vacuum the surface with a wet/dry vac.  Remove and replace clogged blocks in segmented pavers.  For larger areas (e.g., parking lots, courtyards), hire a vacuum sweeper to restore the surface to a cleaner condition.  Other:	
	<ul> <li>Kick-Out to Level 2 Inspection: Grit is widespread and cannot be removed by manual sweeping.</li> </ul>	
are grov permea paveme (applies paveme are not be cove	nt surface only to nt types that intended to red in Other:	
vegetati	on).  Kick-Out to Level 2 Inspection: Grass/weeds cover more than 25% of surface area.	
cracking	For small areas (e.g., patios, small driveway), it may be possible to remove the damaged pavers, check and fill in the underlying gravel, and replace with new materials.  Other:	
surface	Wice: CSN, 2013)  Kick-Out to Level 2 Inspection: Problem affects more than a small, isolated area. Will typically require a qualified contractor to fix it.  Problem recurs or occurs in multiple small locations.	
Permea paveme after a repermea paveme and doe through	nt for days ainstorm; the ble serious problem, and it will be necessary to activate a Level 2 Inspection.  Kick-Out to Level 2 Inspection: This is generally a serious problem, and it will be necessary to activate a Level 2 Inspection.	

## Ponds and Wetlands (Wet Ponds, Stormwater Wetlands)

Table 2.10.1 PV	V Drainage Area
Problem (Check if Present)	Follow-Up Actions
	Seed and straw areas of bare soil to establish vegetation.      Fill in eroded areas with soil, compact, seed and mulch with straw to establish vegetation.      Other:
☐ Bare soil, erosion of the ground (rills washing out the dirt)	Kick-Out to Level 2 Inspection: If a rill or small channel is forming, try to redirect water flowing to this area by creating a small berm or adding topsoil to areas that are heavily compacted.
	If large areas of soil have been eroded or larger channels are forming, this may require rerouting of flow paths or use of an erosion-control seed mat or blanket to reestablish acceptable ground cover or anchor sod where it is practical.
Piles of grass clippings, mulch, dirt, salt, or other materials	<ul> <li>□ Remove or cover piles of grass clippings, mulch, dirt, etc.</li> <li>□ Remove excessive vegetation or woody debris that can block drainage systems.</li> <li>□ Other:</li> </ul>
Open containers of oil, grease, paint, or other substances exposed to rain in the drainage area	Cover or properly dispose of materials; consult your local solid waste authority for guidance on materials that may be toxic or hazardous.      Other:

	Table 2.10.2 Pond Inle	ts
Problem (Check if Present)		Follow-Up Actions
		☐ If the problem can be remedied with hand tool and done in a safe manner, remove vegetation, trash, woody debris, etc. from blocking inlet structures. ☐ Other:
	Inlets are buried, covered or filled with silt, debris, or trash, or blocked by excessive vegetation.	Kick-Out to Level 2 or 3 Inspection: If the amount of material is too large to handle OR there are ANY safety concerns about working in standing water, soft sediment, etc., the wor will likely have to be performed by a qualified contractor.
	Inlets are broken, and, with pieces of pipe or concrete falling into the pond, there is erosion around the inlet, there is open space under the pipe, or there is erosion where the inlet meets the pond	☐ Kick-Out to Level 2 Inspection: These types of structural or erosion problems are more serious and will require a qualified contractor to repair.
Table 2	10.3 PW Pond Area and E	mhankments
Problem (Check if Present)		Follow-Up Actions
	☐ The pretreatment area(s) or forebay(s) are filled with sediment, trash,	<ul> <li>If the problem can be remedied with hand tools and done in a safe manner, use a flat shovel or other equipment to remove small amounts of sediment.</li> <li>Remove trash and excessive vegetation from forebays if this can be done in a safe manner.</li> <li>Other:</li> </ul>
	vegetation, or other debris.	Kick-Out to Level 2 Inspection: Large amounts of sediment or debris will have to be removed by a qualified contractor. ANY condition that poses a safety concern for working in standing water or soft sediments should be referred to a Level 2 Inspection or qualified contractor.

Problem (Check if Present)		Follow-Up Actions
	The pond area itself has accumulated sediment, trash, debris, or excessive vegetation that is choking the flow of the water, OR the pond area is covered with algae or aquatic plants.	Level 1 includes handling only small amounts of material that can be removed by hand, or with rakes or other hand tools. Do not attempt any repair that poses a safety issue.  Other:  Kick-Out to Level 2 Inspection: Most cases will call for a Level 2 Inspection and/or a qualified contractor.  You are not sure what type and amount of vegetation is supposed to be in the pond.  The algae or aquatic plants should be identified so that proper control techniques can be applied.
	The side slopes of the pond are unstable, eroding, and have areas of bare dirt.	If there are only minor areas, try filling in small rills or gullies with topsoil, compacting, and seeding and mulching all bare dirt areas with an appropriate seed. Alternatively, try using herbaceous plugs to get vegetation established in tricky areas, such as steep slopes.  Other:  Kick-Out to Level 2 Inspection: Erosion and many bare dirt areas on steep side slopes will require a Level 2 Inspection and repair by a qualified contractor.
	☐ The riser structure is clogged with trash, debris, sediment,	If you can safely access the riser on foot or with a small boat, clear minor amounts of debris and remove it from the pond area for safe disposal.  Other:
	vegetation, etc., OR is open, unlocked, or has a steep drop and poses a safety concern. The pond level may have dropped below its "normal" level.	<ul> <li>Kick-Out to Level 2 Inspection: The riser cannot be accessed safely, the amount of debris is substantial, or the riser seems to be completely clogged and the water level has risen too high.</li> <li>There are safety issues with the riser and concern about access to pipes, drops, or any other life safety concern.</li> <li>The riser is leaning, broken, settling or slumping, corroded, eroded or any other structural problem.</li> </ul>

#### Table 2.10.3 PW Pond Area and Embankments Follow-Up Actions Problem (Check if Present) If there are small isolated areas, try to fix them by adding clean material (clay and topsoil) and seeding and mulching. Periodically mow embankments to enable inspection of the banks and to minimize establishment of woody vegetation. □ The Remove any woody vegetation that has already dam/embankment is established on embankments. slumping, sinking, Other: settling, eroding, or has medium or large trees growing on it. Kick-Out to Level 2 Inspection: Most of these situations will require a Level 2 Inspection or evaluation and repair by a qualified contractor. Seepage through the dam or problems with the pipe through the dam can be a serious issue that should be addressed to avoid possible dam failure. Clear light debris and vegetation. ☐ Other: The emergency spillway or outfall (if it Kick-Out to Level 2 Inspection: Displacement of rock lining, excessive vegetation and exists) has erosion/settlement may warrant review and erosion, settlement, decision by Level 2 Inspector to check against or loss of material. original plan. Rock-lined spillways Any uncertainty about the integrity of the have excessive emergency spillway should be referred to a debris or vegetation. Level 2 Inspector. Erosion or settlement such that design has been compromised should be reviewed by an engineer. Table 2.10.4 PW Pond Outlet Problem (Check if Present) Follow-Up Actions If there is a minor blockage, remove the debris or vegetation to allow free flow of water. Remove any accumulated trash at the outlet. Outlet: Kick-Out to Level 2 Inspection: If the area at the outlet cannot be easily accessed or if the

The pond outlet is clogged with sediment, trash, debris, vegetation, or is eroding, caving in, slumping, or falling apart.

- blockage is substantial, a Level 2 Inspection is warranted.
- Erosion at and downstream of the outfall should be evaluated by a qualified professional.
- Any structural problems, such as broken pipes, structures falling into the stream, or holes or tunnels around the outfall pipe, should be evaluated by a Level 2 Inspector and will require repair by a qualified contractor.
- The pool of water at the outlet pipe is discolored, has an odor, or has excessive algae or vegetative growth.

## Infiltration (Infiltration Trench, Infiltration Basin, Dry Well)

and the second second	Table 11.1.1 IN Dra	
Problem (Check if Present)		Follow-Up Actions
	Bare soil, erosion of the ground (rills washing out the dirt)	Seed and straw areas of bare soil to establish vegetation.  Fill in erosion areas with soil, compact, and seed and straw to get vegetation established.  If a rill or small channel is forming, try to redirect water flowing to this area by creating a small berm or adding topsoil to areas that are heavily compacted.  Other:
		Kick-Out to Level 2 Inspection: Large areas of soil have been eroded, or larger channels are forming. May require rerouting of flow paths.
☐ For Dry Wells: Leaves, sticks, or other debris in gutters and downspouts		☐ Remove all debris by hand. ☐ Other:
	Piles of grass clippings, mulch, dirt, salt, or other materials	Remove or cover piles of grass clippings, mulch, dirt, etc.  Other:
	Open containers of oil, grease, paint, or other substances	□ Cover or properly dispose of materials; consult your local solid waste authority for guidance on materials that may be toxic or hazardous.     □ Other:

Problem (Check if Present)	Follow-Up Actions	
	Use a flat shovel to remove grit and debris (especially at curb inlets or openings). Parking lots generate fine grit that will accumulate at these spots.	
	<ul> <li>Pull out clumps of growing grass or weeds and scoop out the soil or grit that the plants are growing in.</li> </ul>	
	<ul> <li>Remove any grass clippings, leaves, sticks, and other debris that is collecting at inlets.</li> </ul>	
	For pipes and ditches, remove sediment and debris that is partially blocking the pipe or ditch opening where it enters the infiltration practice.	
	<ul> <li>Dispose of all material properly in an area where it will not re-enter the practice.</li> </ul>	
	Other:	
Inlets are collecting grit and debris or grass/weeds are growing. Some water may not be getting into the Infiltration practice.	Kick-Out to Level 2 Inspection: Inlets are blocked to the extent that most of the water does not seem to be entering the Infiltration practice.	
	For small areas of erosion, smooth out the eroded part and apply rock or stone (e.g., river cobble) to prevent further erosion. Usually, filter fabric is placed under the rock or stone.	
Some or all of the inlets are eroding so that rills, gullies, and other erosion is present, or there is bare dirt that is washing into the Infiltration practice.	<ul> <li>In some cases, reseeding and applying erosion-control matting can be used to prevent further erosion. Some of these materials may be available at a garden center, but it may be best to consult a landscape contractor.</li> <li>Other:</li> </ul>	
	Kick-Out to Level 2 Inspection: Erosion is occurring at most of the inlets and it looks like there is too much water that is concentrating a these points. The inlet design may have to be modified.	

Problem (Check if Present)	1.1.3 IN Infiltration Area Follow-Up Actions	
For grass-covered Infiltration practices: grass has grown very tall,  (Photo credit: Stormwater Maintenance, LLC)	☐ Mow infiltration area at least twice per year. ☐ Other:	
	Add topsoil (as needed), grass seed, straw, and water during the growing season to re-establish consistent grass coverage.      Other:	
For grass-covered Infiltration practices: sparse vegetation cover or bare spots	Kick-Out to Level 2 Inspection: Sparse vegetation cover can be a sign that the infiltration area is not infiltrating at the proper rate and water is standing too long after a storm. The surface may be saturated or squishy, and the conditions do not enable grass to grow. This situation should be evaluated by a Level 2 Inspection and likely corrected by a qualified contractor.	
Minor areas of sediment, grit, trash, or other debris are accumulating on the surface.	Use a shovel to scoop out minor areas of sediment or grit, especially in the spring after winter sanding materials may wash in and accumulate. Dispose of the material where it cannot re-enter the Infiltration practice.  If removing the material creates a hole or low area, rake the surface smooth and level.  Remove trash, debris, and other undesirable materials.  Other:	
	Kick-Out to Level 2 Inspection: Sediment has accumulated more than 2-inches deep and covers 25% or more of the surface of the infiltration area.	

Table 11.1.3 IN Infiltration Area		
Problem (Check if Present)	Follow-Up Actions	
	<ul> <li>For minor areas of erosion, try filling the eroded areas with clean topsoil, sand, or stone (whatever the existing cover is).</li> <li>If the problem recurs, you may have to use larger stone (e.g., rive cobble) to fill in problem areas.</li> <li>Other:</li> </ul>	
There is erosion on the surface; water seems to be carving out rills as it flows across the surface of the infiltration area or sinkholes are forming in certain areas.	<ul> <li>Kick-Out to Level 2 Inspection: The problem persists or the erosio is more than 3-inches deep and seems to be an issue with how water enters and moves through the infiltration area.</li> <li>Kick-Out to Level 2 Inspection: The problem does not seem to be caused by flowing water but a collapse or sinking of the surface (e.g., "sinkhole") due to some underground problem.</li> </ul>	
Observation well is damaged or cap is missing	Kick-Out to Level 2 Inspection: Requires replacing pipes or caps.	
Water still visible in the observation well more than 72 hours after a rain storm. The infiltration practice does not appear to be draining properly.	☐ Kick-Out to Level 2 Inspection: This is generally a serious problem and it will be necessary to activate a Level 2 Inspection.	

Problem (Check if Present)	Follow-Up Actions
	<ul> <li>□ Remove the debris and dispose of it where it cannot re-enter the infiltration area.</li> <li>□ Other:</li> </ul>
□ Outlet obstructed with sediment, debris, trash, etc.	Kick-Out to Level 2 Inspection: Outlet is completely obstructed; there is too much material to remove by hand or with simple hand tools.
☐ Rills or gullies are forming at outlet.	For minor rills, fill in with soil, compact, and seed and straw to establish vegetation.
	□ Other:
	Kick-Out to Level 2 Inspection: Rills are more than 2" to 3" deep and require more than just hand raking and re-seeding.

# Sand and Organic Filters (Surface Sand Filters, Underground Sand Filters, Underground Organic Filters)

Problem (Check if Present)	2.12.1 SF Drainage Area Follow-Up Actions		
	Seed and straw areas of bare soil to get vegetation established.  Fill in erosion areas with soil, compact, and seed and straw to establis vegetation.  If a rill or small channel is forming, try to redirect water flowing to this area by creating a small berm or adding topsoil to areas that are heavily compacted.  Other:		
☐ Bare soil, erosion of the ground (rills washing out the dirt)	Kick-Out to Level 2 Inspection: Large areas of soil have been eroded, or larger channels are forming. May require rerouting of flow paths.		
□ Piles of grass clippings, mulch, dirt, salt, or other	□ Remove or cover piles of grass clippings, mulch, dirt, etc. □ Other:		
Den containers of oil, grease, paint, or other substances	Cover or properly dispose of materials; consult your local solid waste authority for guidance on materials that may be toxic or hazardous.  Other:		

Table 2.12.	2 SF Inlets	
Problem (Check if Present)	Follow-Up Actions	
Inlets are collecting grit and debris or grass/weeds growing. Some water may not be getting into the filter practice.	<ul> <li>Use a flat shovel to remove grit and debris (especially at curb inlets or openings). Parking lots generate fine grit that accumulates at these spots.</li> <li>Pull out clumps of growing grass or weeds and scoop out the soil or grit that the plants are growing in.</li> <li>Remove any grass clippings, leaves, sticks, and other debris that is collecting at inlets.</li> <li>For pipes and ditches, remove sediment and debris that is partially blocking the pipe or ditch opening where it enters the Filter practice.</li> <li>Dispose of all material properly in an area where it will not re-enter the practice.</li> <li>Other:</li> </ul>	
Some or all of the inlets are eroding so that rills, gullies, and other erosion are present, or there is dirt washing into the filter practice.	For small areas of erosion, smooth out the eroded part and apply rock or stone (e.g., river cobble) to prevent further erosion. Usually, filter fabric is placed under the rock or stone.      In some cases, reseeding and applying erosion-control matting can be used to prevent further erosion. Some of these materials may be available at a garden center, but it may be best to consult a landscape contractor.      Other:      Kick-Out to Level 2 Inspection: Erosion is occurring at most of the inlets and it looks like there is too much water.	
	concentrating at these points. The inlet design may have to be modified.	
Table 2.12 Problem (Check if Present)	2 SF Inlets Follow-Up Actions	
For an underground filter, water is ponding and doesn't seem to be getting through the filter.	Kick-Out to Level 2 Inspection: This is generally a more serious problem and should be referred for a Level 2 inspection because it will require opening up the filter vault to check for clogging.	

Table 2.12.3 SF Filter Area (for Surface Sand Filters)  Problem (Check if Present) Follow-Up Actions		
	Vegetation growing in the filter bed should be removed either manually or with a water-safe herbicide (e.g., glysophate without surfactants).  Other:	
☐ Filter has grass and vegetation growing on more than 25% of the filter bed, threatening to clog the filter.	Kick-Out to Level 2 Inspection: The filter seems clogged, or vegetation and weeds have proliferated past the point where the Level 1 person can manage it.	
■ Minor amounts of sediment, grit, trash, or other debris are accumulating on the surface.	Use a shovel to scoop out minor amounts of sediment or grit, especially in the spring after winter sanding materials wash in and accumulate. Dispose of the material where it cannot re-enter the filter.  If removing the material creates a hole or low area, rake the surface smooth and level.  Remove trash, debris, and other undesirable materials.	
1	Kick-Out to Level 2 Inspection: Sediment (other than sand) has accumulated more than 2-inches deep and covers 25% or more of the surface of the filter area.	

## Table 2.12.3 SF Filter Area (for Surface Sand Filters) Problem (Check if Present) Follow-Up Actions For minor areas of erosion, try filling the eroded areas with clean, coarse construction sand. □ Other: Kick-Out to Level 2 Inspection: The problem persists or the erosion is more than 3-inches deep and seems to be an issue with how water enters and moves through the filter area. ☐ Kick-Out to Level 2 Inspection: The problem does not seem to be caused by flowing water but by a collapse or sinking of the surface (e.g., There is erosion on the surface; water seems "sinkhole") due to some underground problem. to be carving out rills as it flows across the filter surface, or sinkholes are forming in certain areas. Kick-Out to Level 2 Inspection: This is generally a serious problem, and it will be necessary to activate a Level 2 Inspection.

 Water is still visible on the surface and/or the standpipe (if present) more than 72 hours after a rainstorm. The filter practice drains very slowly or is completely clogged.

### Tree Planting Stormwater Management Practices Level 1 Inspection Checklist SMP ID# **SMP Owner** Private Public **SMP Location** (Address; Latitude & Longitude) Latitude Longitude Party Responsible for System Type Type of Site Maintenance Above Ground Same as SMP Owner Seasonal Commercial Below Ground Industrial Other Continuous Use Residential Other State **Inspection Date Inspection Time** Inspector **Date of Last** Inspection **TP Watering** Inspect the trees to determine whether they need watering. **Problem (Check if Present) Follow-Up Actions** Water trees deeply and slowly near the base. Soaker hoses and drip Soil is not moist to the touch and/or it has not irrigation work best for deep watering of trees and shrubs. rained in a week, and leaves/needles are starting

Other:

to appear wilted/dry.



#### **TP Mulch**

Mulch should be applied in the late spring and during leaf fall. Check the depth of mulch regularly. Rake the old mulch to break up any matted layers and to refresh the appearance.

Problem (Check if Present)	Follow-Up Actions	
Mulch is too thin or thick (should be approximately 3" deep) or does not extend to tree canopy (or 5' radius if tree has a larger than 10' canopy reach).	<ul> <li>Add or remove mulch around tree canopy to maximum 5' radius but not within 3" of the bark.</li> <li>If mulch is against the stems or tree trunks, pull it back several inches to expose the base of the trunk and root crown.</li> <li>Other:</li> </ul>	

TP Pruning			
Examine the branches and tree shape.			
Problem (Check if Present)	Follow-Up Actions		
<ul> <li>Presence of suckers, dead or diseased branches, branches that interfere with pedestrian traffic</li> </ul>	<ul> <li>Selective cutting</li> <li>Prune to make the tree more aesthetically pleasing and remove disease.</li> <li>Other:</li> <li>Kick-Out to Level 2 Inspection: Use an arborist or landscaper for more extensive pruning jobs.</li> </ul>		

Additional Notes:			
	,		



Inspector:	Date:		
Complete the following if follow-up/corrective actions were identified during this inspection:			
Certified Completion of Follow-Up Actions:			
"I hereby certify that the follow-up/corrective actions identified in the inspection performed on (DATE) have been completed and any required maintenance deficiencies have been adequately corrected."			
Inspector/Operator:	Date:		



#### Tree Planting Stormwater Management Practices Level 2 Inspection Checklist Private SMP ID# **SMP Owner** Public **SMP Location** (Address; Latitude & Longitude) Latitude Longitude Party Responsible for System Type Type of Site Maintenance Above Ground Commercial □ Same as SMP Owner Seasonal ■ Below Ground Industrial □ Other Continuous Use Residential Other □ State **Inspection Date Inspection Time** Inspector **Date of Last**

Level 2 Inspection: TREE PLANTING			
Recommended Repairs	Triggers for Level 3 Inspection		
Observed Condition: Appearance of fungus or pest damage			
☐ Condition 1: Fungus, discoloration, browning leaves or holes in leaves  Check with arborist or other tree professional about the best way to proceed.  This requires a Level 3 inspection.	Any concerns about how to address infestation or disease		
☐ Condition 2: Burrowing insects, holes  Check with arborist or other tree professional about the best way to proceed. This requires a Level 3 inspection.	☐ Level 3 inspection necessary		

Inspection



Notes:		
Inspector:	Date:	
	in al alumina Alain in an antion.	
Complete the following if follow-up/corrective actions were identif	led during this inspection:	
Certified Completion of Follow-Up Actions:		
"I hereby certify that the follow-up/corrective actions identified in the inspection performed on (DATE) have been completed and any required maintenance deficiencies have been adequately corrected."		
Inspector/Operator:	Date:	



### **Bioretention Stormwater Management Practices** Level 1 Inspection Checklist SMP ID# **SMP Owner** Private Public **SMP Location** (Address; Latitude & Longitude) Latitude Longitude Party Responsible for Type of Site **System Type** Maintenance ☐ Above Ground Commercial Same as SMP Owner Seasonal Other Continuous Use ■ Below Ground Industrial Residential □ Other ☐ State **Inspection Date Inspection Time** Inspector **Date of Last** Inspection

DR Dialilage Area			
Look for areas that are uphill from the Bioretention cell.			
Problem (Check if Present)		Follow-Up Actions	
	Bare soil, erosion of the ground (rills washing out the dirt)	<ul> <li>Seed and mulch areas of bare soil to establish vegetation.</li> <li>Fill in erosion areas with soil, compact, and seed and straw to establish vegetation.</li> <li>If a rill or small channel is forming, try to redirect water flowing to this area by creating a small berm or adding topsoil to areas that are heavily compacted.</li> <li>Other:</li> </ul>	

BR Drainage Area			
Look for areas that are uphill from the Bioretention cell.			
Problem (Check if Present)	Follow-Up Actions		
	☐ Kick-Out to Level 2 Inspection: Large areas of soil have been eroded, or larger channels are forming. May require rerouting of flow paths.		
Piles of grass clippings, mulch, dirt, salt, or other materials	<ul> <li>Remove or cover piles of grass clippings, mulch, dirt, etc.</li> <li>Other:</li> </ul>		
Open containers of oil, grease, paint, or other substances	<ul> <li>Cover or properly dispose of materials; consult your local solid waste authority for guidance on materials that may be toxic or hazardous.</li> <li>Other:</li> </ul>		



#### **BR Inlets**

Stand in the Bioretention cell itself and look for all the places where water flows in. Often there will be multiple points of inflow to the practice.

Problem (Check if Present)	Follow-Up Actions	
□ Inlets collect grit and debris or grass/weeds. Some water may not be getting into the Bioretention cell. The objective is to have a clear pathway for water to flow into the cell.	<ul> <li>Use a flat shovel to remove grit and debris (especially at curb inlets or openings). Parking lots generate fine grit that will accumulate at these spots.</li> <li>Pull out clumps of growing grass or weeds and scoop out the soil or grit that the plants are growing in.</li> <li>Remove any grass clippings, leaves, sticks, and other debris that is collecting at inlets.</li> <li>For pipes and ditches, remove sediment and debris that is partially blocking the pipe or ditch opening where it enters the Bioretention cell.</li> <li>Dispose of all material properly where it will not re-enter the Bioretention cell.</li> <li>Other:</li> </ul>	
□ Some or all of the inlets are eroding so that	<ul> <li>For small areas of erosion, smooth out the eroded part and apply rock or stone (e.g., river cobble) to prevent further erosion. Usually, filter fabric is placed under the rock or stone.</li> <li>In some cases, reseeding and applying erosion-control matting can be used to prevent further erosion. Some of these materials may be available at a garden center, but it may be best to consult a landscape contractor.</li> <li>Other:</li> </ul> Kick-Out to Level 2 Inspection: Erosion is occurring at most of the inlets, and it looks like there is too much water that is concentrating at these	
rills, gullies, and other erosion is present, or there is bare dirt that is washing into the Bioretention cell.	points. The inlet design may have to be modified.	

#### **BR Ponding Area**

Examine the entire Bioretention surface and side slopes

#### **Problem (Check if Present)**





- Mulch (if used) needs to be replaced or replenished. The mulch layer had decomposed or is less than 1-inch thick.
- Add new mulch to a total depth (including any existing mulch that is left) of 2 to 3 inches. The mulch should be shredded hardwood mulch that is less likely to float away during rainstorms.
- Avoid adding too much mulch so that inlets are obstructed or certain areas become higher than the rest of the Bioretention surface.
- Other:



Minor areas of sediment, grit, trash, or other debris are accumulating on the bottom.

- ☐ Use a shovel to scoop out minor areas of sediment or grit, especially in the spring after winter sanding materials may wash in and accumulate. Dispose of the material where it cannot re-enter the Bioretention cell .
- ☐ If removing the material creates a hole or low area, fill with soil mix that matches original mix and cover with mulch so that the Bioretention surface area is as flat as possible.
- ☐ Remove trash, vegetative debris, and other undesirable materials.
- Other
- ☐ Kick-Out to Level 2 Inspection: Sediment has accumulated more than 2-inches deep and covers 25% or more of the Bioretention surface.
- ☐ Kick-Out to Level 2 Inspection: The Bioretention cell is too densely vegetated to assess sediment accumulation or ponding; see BR-4, Vegetation.

#### **BR Ponding Area**

Examine the entire Bioretention surface and side slopes

#### Problem (Check if Present)

#### **Follow-Up Actions**



- mulch. If the problem recurs, you may have to use stone (e.g., river cobble) to fill
  - in problem areas.

Try filling the eroded areas with clean topsoil or sand, and cover with

- If the erosion is on a side slope, fill with clay that can be compacted and seed and mulch the area.
- Other:
- There is erosion in the bottom or on the side slopes. Water seems to be carving out rills as it flows across the Bioretention surface or on the slopes, or sinkholes are forming in certain areas
- Source: Stormwater Maintenance, LLC.
- Kick-Out to Level 2 Inspection: The problem persists or the erosion is more than 3-inches deep and seems to be an issue with how water enters and moves through the Bioretention cell.
- Kick-Out to Level 2 Inspection: The problem does not seem to be caused by flowing water, but a collapse or sinking of the surface (e.g., "sinkhole") due to some underground problem.



The bottom of the Bioretention cell is not flat, and the water pools at one end, along an edge, or in certain pockets. The whole bottom is not uniformly covered with water. See design plan to verify that bioretention surface is intended to be flat. Check during or immediately after a rainstorm.

- If the problem is minor (just small, isolated areas are not covered with water), try raking the surface OR adding mulch to low spots to create a more level surface. You may need to remove and replace plantings in order to properly even off the surface.
- Check the surface with a string and bubble level to get the surface as flat as possible.
- Other: П
- Kick-Out to Level 2 Inspection: Ponding water is isolated to less than half of the Bioretention surface area, and there seem to be elevation differences of more than a couple of inches across the surface.

#### **BR Ponding Area**

Examine the entire Bioretention surface and side slopes

#### **Problem (Check if Present)**

#### **Follow-Up Actions**



Water stands on the surface more than 72 hours after a rainstorm and /or wetland-type vegetation is present. The Bioretention cell does not appear to be draining properly. Kick-Out to Level 2 Inspection: This is generally a serious problem, and it will be necessary to activate a Level 2 Inspection.

#### **BR Vegetation**

Examine all Bioretention cell vegetation.

#### **Problem (Check if Present)**

Vegetation requires regular maintenance—pulling weeds, removing dead and diseased plants, replacing mulch around plants, adding plants to fill in areas that are not well vegetated, etc.

#### **Follow-Up Actions**

- If you can identify which plants are weeds or not intended to be part of the planting plan, eliminate these, preferably by hand pulling.
  - If weeds are widespread, check with the local stormwater authority and/or Extension Office about proper use of herbicides for areas connected with the flow of water.
- Even vegetation that is intended to be present can become large, overgrown, and/or crowd out surrounding plants. Prune and thin accordingly.
- ☐ If weeds or invasive plants have overtaken the whole Bioretention cell, bush-hog the entire area before seedheads form in the spring. It will be necessary to remove the root mat manually or with appropriate herbicides, as noted above.
- Re-plant with species that are aesthetically pleasing and seem to be doing well in the Bioretention cell.
- Other:
- Kick-Out to Level 2 Inspection: You are unsure of the original planting design, or the vegetation maintenance task is beyond your capabilities of time, expertise, or resources. If you are unsure of the health of the vegetation (e.g. salt damage, invasives, which plants are undesirable) or the appropriate season to conduct vegetation management, consult a landscape professional before undertaking any cutting, pruning, mowing, or brush hogging.

#### **BR Vegetation**

Examine all Bioretention cell vegetation.

#### **Problem (Check if Present)**

Vegetation is too thin, is not healthy, and there are many spots that are not well vegetated.

#### **Follow-Up Actions**

- The original plants are likely not suited for the actual conditions within the Bioretention cell . If you are knowledgeable about plants, select and plant more appropriate vegetation (preferably native plants) so that almost the entire surface area will be covered by the end of the second growing season.
- Other:

☐ Kick-Out to Level 2 Inspection: For all but small practices (e.g., rain gardens), this task will likely require a landscape design professional or horticulturalist.

#### **BR Outlets**

Examine outlets that release water out of the Bioretention cell.		
Problem (Check if Present)	Follow-Up Actions	
□ Erosion at outlet	<ul> <li>Add stone to reduce the impact from the water flowing out of the outlet pipe or weir during storms.</li> <li>Other:</li> </ul>	
	Kick-Out to Level 2 Inspection: Rills have formed and erosion problem becomes more severe.	
Outlet obstructed with mulch, sediment, debris, trash, etc.	Bioretention cell .	
	Kick-Out to Level 2 Inspection: Outlet is completely clogged or obstructed; there is too much material to remove by hand or with simple hand tools.	



Additional Notes:		
Inspector:	Date:	
Complete the following if follow-up/corrective actions we	ere identified during this inspection:	
Certified Completion of Follow-Up Actions:		
"I hereby certify that the follow-up/corrective actions identified in the inspection		
performed on (DATE) have been maintenance deficiencies have been adequately	completed and any required corrected."	
	<b>5</b> .	
Inspector/Operator:	Date:	



#### **Bioretention Stormwater Management Practices** Level 2 Inspection Checklist Private SMP ID# **SMP Owner** Public **SMP Location** (Address; Latitude & Longitude) Latitude Longitude Party Responsible for System Type Type of Site Maintenance Above Ground Commercial □ Same as SMP Owner Seasonal ■ Below Ground Industrial □ Other Continuous Use Residential □ Other □ State **Inspection Date Inspection Time** Inspector **Date of Last** Inspection



#### Level 2 Inspection: BIORETENTION NOTE: Key Source for this Information (CSN, 2013) **Recommended Repairs Triggers for Level 3 Inspection** Observed Condition: Water Stands on Surface for More than 72 Hours after Storm Condition 1: Small pockets of standing water Use a soil probe or auger to examine the soil profile. If isolated areas have accumulated grit, fines, or vegetative debris or have bad soil media, try scraping off top 3 inches of media and replacing with clean material. Also check to see Soil media is clogged and problem is not that surface is level and water is not ponding selectively in certain areas. evident from Level 2 inspection. Level 2 inspection identifies problem, but it Condition 2: Standing water is widespread or covers entire surface cannot be resolved easily or is associated with the original design of the practice. Requires diagnosis and resolution of problem: Clogged underdrain? Filter fabric between soil media and underdrain stone? Need to install underdrain if not present? Level 3 inspection necessary Too much sediment/grit washing in from drainage area? Too much ponding depth? Improper soil media? Observed Condition: Vegetation is sparse or out of control Condition 1: Original design planting plan seems good but has not been maintained, so there are many invasives and/or dead plants Vegetation deviates significantly from original planting plan; Bioretention has Will require some horticultural experience to restore vegetation to intended been neglected and suffered from deferred condition by weeding, pruning, removing plants, and adding new plants. maintenance. Owner/responsible party does not know Condition 2: Original design planting plan is unknown or cannot be how to maintain the practice. actualized A landscape architect or horticulturalist will be needed to redo the planting plan. Will likely require analysis of soil pH, moisture, organic content, sun/shade, and other conditions to make sure plants match conditions. Plan should include Level 3 inspection necessary invasive plant management and maintenance plan to include mulching, watering, disease intervention, periodic thinning/pruning, etc. Observed Condition: Bioretention does not conform to original design plan in surface area or storage Condition 1: Level 2 Inspection reveals that practice is too small based More than a 25% departure from the on design dimension, does not have adequate storage (e.g., ponding approved plan in surface area, storage, or depth) based on the plan, and/or does not treat the drainage area runoff drainage area; sometimes less than this as indicated on the plan threshold at the discretion of the Level 2 inspector. Small areas of deviation can be corrected by the property owner or responsible party, but it is likely that a Qualified Professional will have to revisit the design and attempt a redesign that meets original objectives or that can be resubmitted to the municipality for approval. Level 3 inspection necessary



#### **Level 2 Inspection: BIORETENTION** NOTE: Key Source for this Information (CSN, 2013) **Recommended Repairs Triggers for Level 3 Inspection** Observed Condition: Severe erosion of filter bed, inlets, or around outlets Condition 1: Erosion at inlets The lining (e.g., grass, matting, stone, rock) may not be adequate for the actual flow velocities coming through the inlets. First line of defense is to try a more non-erosive lining and/or to extend the lining further down to where inlet slopes Erosion (rills, gullies) is more than 12 meet the Bioretention surface. If problem persists, analysis by a Qualified inches deep at inlets or the filter bed or Professional is warranted. more than 3 inches deep on side slopes. If the issue is not caused by moving water Condition 2: Erosion of Bioretention filter bed but some sort of subsurface defect. This may manifest as a sinkhole or linear This is often caused by "preferential flow paths" through and along the depression and be associated with Bioretention surface. The source of flow should be analyzed and methods problems with the underdrain stone or pipe employed to dissipate energy and disperse the flow (e.g., check dams, rock or underlying soil. splash pads). Condition 3: Erosion on side slopes Level 3 inspection necessary Again, the issue is likely linked with unanticipated flow paths down the side slopes (probably overland flow that concentrates as it hits the edge of the slope). For small or isolated areas, try filling, compacting, and re-establishing healthy ground cover vegetation. If the problem is more widespread, further analysis is required to determine how to redirect the flow. Observed Condition: Significant sediment accumulation, indicating an uncontrolled source of sediment Condition 1: Isolated areas of sediment accumulation, generally less than 3-inches deep More than 2 inches of accumulated Sediment source may be from a one-time or isolated event. Remove sediment cover 25% or more of the accumulated sediment and top 2 to 3 inches of Bioretention soil media; replace Bioretention surface area. with clean material. Check drainage area for any ongoing sources of sediment. "Hard pan" of thin, crusty layer covers majority of Bioretention surface area and seems to be impeding flow of water down Condition 2: Majority of the surface is caked with "hard pan" (thin layer of through the soil media. clogging material) or accumulated sediment that is 3-inches deep or New sources of sediment seem to be more accumulating with each significant rainfall event. This can be caused by an improper construction sequence (drainage area not fully stabilized prior to installation of Bioretention soil media) or another chronic source of sediment in the drainage area. Augering several holes down through the media can indicate how severe the problem is; often the damage is confined Level 3 inspection necessary to the first several inches of soil media. Removing and replacing this top layer (or to the depth where sediment incursion is seen in auger holes) can be adequate, as long as the problem does not recur.



Notes:		
Inspector:	Date:	
Complete the following if follow-up/corrective actions were ide	ntified during this inspection:	
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Certified Completion of Follow-Up Actions:		
"I hereby certify that the follow-up/corrective actions identified in the inspection		
performed on (DATE) have been complemaintenance deficiencies have been adequately correct	eted and any required cted."	
Inspector/Operator:	Date:	
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## APPENDIX G: NYSDEC "DEEP-RIPPING AND DECOMPACTION," APRIL 2008





# New York State DEPARTMENT OF ENVIRONMENTAL CONSERVATION

Division of Water

# Deep-Ripping and Decompaction

**April 2008** 

New York State Department of Environmental Conservation

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### Alternative Stormwater Management Deep-Ripping and Decompaction

### Description

The two-phase practice of 1) "Deep Ripping;" and 2) "Decompaction" (deep subsoiling), of the soil material as a step in the cleanup and restoration/landscaping of a construction site, helps mitigate the physically induced impacts of soil compression; i.e.: soil compaction or the substantial increase in the bulk density of the soil material.

grading, the ongoing movement of construction equipment and the transport of building Deep Ripping and Decompaction are key factors which help in restoring soil pore space and permeability for water infiltration. Conversely, the physical actions of cut-and-fill work, land materials throughout a site alter the architecture and structure of the soil, resulting in: the mixing of layers (horizons) of soil materials, compression of those materials and diminished soil porosity which, if left unchecked, severely impairs the soil's water holding capacity and vertical drainage (rainfall infiltration), from the surface downward.

decompaction – is complete). A heavy-duty tractor is pulling a three-shank ripper on the first of In a humid climate region, compaction damage on a site is virtually guaranteed over the duration of a project. Soil in very moist to wet condition when compacted, will have severely reduced Figure 1 displays the early stage of the deep-ripping phase (Note that all topsoil several series of incrementally deepening passes through the construction access corridor's Figure 2 illustrates the approximate volumetric composition of a loam surface soil when conditions are good for plant growth, with adequate was stripped prior to construction access, and it remains stockpiled until the next phase natural pore space for fluctuating moisture conditions. compressed subsoil material. permeability. densely



progressively deeper "rips" through severely Fig. 1. A typical deep ripping phase of this practice, during the first in a series of compressed subsoil.



Fig. 2. About 50% of the volume of undisturbed loam surface soil is pore space, when soil is in good condition for plant growth. Brady, 2002.

## Recommended Application of Practice

Decompaction first became established as a "best management practice" through ongoing success reduce runoff. Together with topsoil stripping, (vertically and laterallly) through the thickness (transmission pipelines and large power lines). the "two-phase" practice of Deep Ripping and of the physically compressed subsoil material on commercial farmlands affected by heavy permeability and aiding infiltration to help (see Figure 3), restoring soil porosity and utility construction right-of-way projects Decompaction is to effectively fracture The objective of Deep Ripping and



extends 24 inches below this exposed cut-and-fill work surface.

Soil permeability, soil drainage and cropland productivity were restored. For broader

obstructions for the easy avoidance and maneuvering of a large tractor and ripping/decompacting construction sites and inside long, open construction corridors used as temporary access over the adapted to areas impacted with significant soil compaction, on contiguous open portions of large mplements. Conversely, the complete two-phase practice is not recommended in congested or duration of construction. Each mitigation area should have minimal above-and-below-ground construction application, the two-phase practice of Deep Ripping and Decompaction is best obstructed areas due to the limitations on tractor and implement movement.

#### Benefits

Aggressive "deep ripping" through the compressed thickness of exposed subsoil before the replacement/respreading of the topsoil layer, followed by "decompaction," i.e.: "sub-soiling," through the restored topsoil layer down into the subsoil, offers the following benefits:

- by on rainfall the open site's mitigated soil condition and lowers the demand Increases the project (larger size) area's direct surface infiltration of concentrated runoff control structures providing •
- Enhances direct groundwater recharge through greater dispersion across and through a broader surface than afforded by some runoff-control structural measures
- Decreases runoff volume generated and provides hydrologic source control
- Ħ. or planned for application in feasible open locations either alone May be

conjunction with plans for structural practices (e.g., subsurface drain line or infiltration basin) serving the same or contiguous areas

Promotes successful long-term revegetation by restoring soil permeability, drainage and water holding capacity for healthy (rather than restricted) root-system development of trees, shrubs and deep rooted ground cover, minimizing plant drowning during wet periods and burnout during dry periods.

## Feasibility/Limitations

The effectiveness of Deep Ripping and Decompaction is governed mostly by site factors such as: the original (undisturbed) soil's hydrologic characteristics; the general slope; local weather/timing (soil moisture) for implementation; the space-related freedom of equipment/implement maneuverability (noted above in **Recommended Application of Practice**), and by the proper selection and operation of tractor and implements (explained below in **Design Guidance**). The more notable site-related factors include:

#### 5

In the undisturbed condition, each identified soil type comprising a site is grouped into one of four categories of soil hydrology. Hydrologic Soil Group A. B. C or D, determined primarily by a range of characteristics including soil texture, drainage capability when thoroughly wet, and depth to water table. The natural rates of infiltration and transmission of soil-water through the undisturbed soil layers for Group A is "high" with a low tunoff potential while soils in Group B are moderate in infiltration and the transmission of soil-water with a moderate runoff potential, depending somewhat on slope. Soils in Group C have slow rates of infiltration and transmission of soil-water with a moderate runoff potential, depending somewhat on slope. Soils in Group C have slow rates of infiltration and transmission of soil-water and snoderately high runoff potential influenced by soil texture and slope; while exceptionally slow.

soils in Group D have exceptionally slow rates of infiltration and transmission of soilwater, and high runoff potential. In Figure 4, the profile displays the undisturbed horizons of a soil in Hydrologic Soil Group C and the naturally slow rate of infiltration through the subsoil. The slow rate of infiltration horizon (30 cm), due to the limited amount of macro pores, e.g.: natural subsoil fractures, worm holes and root channels. Infiltration after the construction-induced mixing and compression of such subsoil material is virtually absent; but can be restored back to this natural level with the two-phase practice of deep ripping and decompaction, followed by the permanent establishment of an appropriate, deep taproot

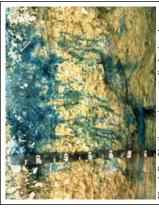


Fig. 4. Profile (in centimeters) displaying the infiltration test result of the natural undisturbed horizons of a soil in Hydrologic Soil Group C.

lawn/ground cover to help maintain the restored subsoil structure. Infiltration after constructioninduced mixing and compression of such subsoil material can be notably rehabilitated with the Deep Ripping and Decompaction practice, which prepares the site for the appropriate long-term lawn/ground cover mix including deep taproot plants such as clover, fescue or trefoil, etc. needed for all rehabilitated soils. Generally, soils in Hydrologic Soil Groups A and B, which respectively may include deep, well-drained, sandy-gravelly materials or deep, moderately well-drained basal till materials, are among the easier ones to restore permeability and infiltration, by deep ripping and decompaction. Among the many different soils in Hydrologic Soil Group C are those unique glacial tills having a natural fragipan zone, beginning about 12 to 18 inches (30 – 45cm), below surface. Although soils in Hydrologic Soil Group C do require a somewhat more carefully applied level of the Deep Ripping and Decompaction practice, it can greatly benefit such affected areas by reducing the runoff and fostering infiltration to a level equal to that of pre-disturbance.

Soils in Hydrologic Soil Group D typically have a permanent high water table close to the surface, influenced by a clay or other highly impervious layer of material. In many locations with clay subsoil material, the bulk density is so naturally high that heavy trafficking has little or no added impact on infiltration; and structural runoff control practices rather than Deep Ripping and Decompaction should be considered.

The information about Hydrologic Soil Groups is merely a general guideline. Site-specific data such as limited depths of cut-and-fill grading with minimal removal or translocation of the inherent subsoil materials (as analyzed in the county soil survey) or, conversely, the excavation and translocation of deeper, unconsolidated substratum or consolidated bedrock materials (unlike the analyzed subsoil horizons' materials referred to in the county soil survey) should always be taken into account.

Sites made up with significant quantities of large rocks, or having a very shallow depth to bedrock, are not conducive to deep ripping and decompation (subsoiling); and other measures may be more practical.

#### Slope

The two-phase application of 1) deep ripping and 2) decompaction (deep subsoiling), is most practical on flat, gentle and moderate slopes. In some situations, such as but not limited to temporary construction access corridors, inclusion areas that are moderately steep along a project's otherwise gentle or moderate slope may also be deep ripped and decompacted. For limited instances of moderate steepness on other projects, however, the post-construction land use and the relative alignment of the potential ripping and decompaction work in relation to the lay of moderately steep or steep slopes, the practice is generally not used.

## Local Weather/Timing/Soil Moisture

Effective fracturing of compressed subsoil material from the exposed work surface, laterally and vertically down through the affected zone is achieved only when the soil material is moderately dry to moderately moist. Neither one of the two-phases, deep ripping nor decompaction (deep

subsoiling), can be effectively conducted when the soil material (subsoil or replaced topsoil) is in either a "plastic" or "liquid" state of soil consistency. Pulling the respective implements legs through the soil when it is overly moist only results in the "slicing and smearing" of the material or added "squeezing and compression" instead of the necessary fracturing. Ample drying time is needed for a "rippable" soil condition not merely in the material close to the surface, but throughout the material located down to the bottom of the physically compressed zone of the

Conversely, as shown in Figure 5, if the rolled The "poor man's Atterberg field test" for soil plasticity is a simple "hand-roll" method used for quick, on-site determination of whether or not the moisture level of the affected soil material is low enough for: effective deep ripping of subsoil; respreading of topsoil in a friable state; and final decompaction (deep subsoiling). Using a sample of soil material obtained from the planned bottom depth of ripping, e.g.: 20 - 24 inches below exposed subsoil surface, the sample is hand rolled between the palms down to a 1/8-inch diameter thread. (Use the same test for stored topsoil material before respreading on the site.) If the segments no greater than 3/8 of an inch long, by the time it is rolled down to 1/8 inch diameter, it is low enough in moisture for deep ripping (or decompaction. sample stretches out in increments greater than apart respective soil sample crumbles replacement),



Fig. 5. Augered from a depth of 19 inches below the surface of the replaced topsoil, this subsoil sample was hand rolled to a 1/8-inch diameter. The test shows the soil at this site stretches out too far without crumbling; it indicates the material is in a plastic state of consistence, too wet for final decompaction (deep subsoiling) at this time.

3/8 of an inch long before crumbling, it is in a "plastic" state of soil consistency and is too wet for subsoil ripping (as well as topsoil replacement) and final decompaction.

### **Design Guidance**

Beyond the above-noted site factors, a vital requirement for the effective Deep Ripping and Decompaction (deep subsoiling), is implementing the practice in its distinct, two-phase process:

- Deep rip the affected thickness of exposed subsoil material (see Figure 10 and 11), aggressively fracturing it before the protected topsoil is reapplied on the site (see Figure 12); and
- 2) Decompact (deep subsoil), simultaneously through the restored topsoil layer and the upper half of the affected subsoil (Figure 13). The second phase, "decompaction," mitigates the partial recompaction which occurs during the heavy process of topsoil spreading/grading. Prior to deep ripping and decompacting the site, all construction activity, including construction equipment and material storage, site cleanup and trafficking (Figure 14), should be finished; and the site closed off to further disturbance. Likewise, once the practice is underway and the area's soil permeability and

rainfall infiltration are being restored, a policy limiting all further traffic to permanent travel lanes is maintained.

The other critical elements, outlined below, are: using the proper implements (deep, heavy-duty rippers and subsoilers), and ample pulling-power equipment (tractors); and conducting the practice at the appropriate speed, depth and pattem(s) of movement.

Note that an appropriate plan for the separate practice of establishing a healthy perennial ground cover, with deep rooting to help maintain the restored soil structure, should be developed in advance. This may require the assistance of an agronomist or landscape horticulturist.

#### nplements

Avoid the use of all undersize implements. The small-to-medium, light-duty tool will, at best, only "scarify" the uppermost surface portion of the mass of compacted subsoil material. The term "chisel plow" is commonly but incorrectly applied to a broad range of implements. While a few may be adapted for the moderate subsoiling of non-impacted soils, the majority are less durable and used for only lighter land-fitting (see Figure 6).



Fig. 6. A light duty chisel implement, not adequate for either the deep ripping or decompaction (deep subsoiling) phase.



Fig. 7. One of several variations of an agricultural ripper. This unit has long, rugged shanks mounted on a steel V-frame for deep, aggressive fracturing through Phase 1.

Use a "heavy duty" agricultural-grade, deep ripper (see Figures 7,9,10 and 11) for the first phase: the lateral and vertical fracturing of the mass of exposed and compressed subsoil, down and through, to the bottom of impact, prior to the replacement of the topsoil layer. (Any oversize rocks which are uplifted to the subsoil surface during the deep ripping phase are picked and removed.) Like the heavy-duty class of implement for the first phase, the decompaction (deep subsoiling) of Phase 2 is conducted with the heavy-duty version of the deep subsoiler. More preferable is the angled-leg variety of deep subsoiler (shown in Figures 8 and 13). It minimizes the inversion of the subsoil and topsoil layers while laterally and vertically fracturing the upper half of the previously ripped subsoil layer and all of the topsoil layer by delivering a momentary, wave-like "lifting and shattering" action up through the soil layers as it is pulled.

## Pulling-Power of Equipment

Use the following rule of thumb for tractor horsepower (hp) whenever deep ripping and decompacting a significantly impacted site: For both types of implement, have at least 40 hp of tractor pull available for each mounted shank/leg.

Using the examples of a 3-shank and a 5-shank implement, the respective tractors should have 120 and 200 hp available for fracturing down to the final depth of 20-to-24 inches per phase. Final depth for the deep ripping in Phase 1 is achieved incrementally by a progressive series of passes (see Depth and Patterns of Movement, below); while for Phase 2, the full operating depth of the deep subsoiler is applied from the beginning.

The operating speed for pulling both types of implement should not exceed 2 to 3 mph. At implement is the 6-leg version of the deep angled-leg subsoiler. Its two outside legs are topsoil and the upper 12 inches of the areas of Phase 1) Deep Ripping, a medium-size tractor with adequate hp, such as the one in by the tractor and the implement performing the Referring to Figure 8, the "chained up" so that only four legs will be less than 160 hp, (rather than 240 hp) of pull. The 4-wheel drive, articulated-frame tractor in Figure 8 is 174 hp. It will be decompacting this previously deep-ripped subsoil. In constricted Figure 9 pulling a 3-shank deep ripper, may be this slow and managed rate of operating speed, maximum functional performance is sustained engaged (at the maximum depth), requiring no unobstructed, former construction access area simultaneously through 11 inches of replaced more maneuverable. fracturing. soil

and stout; and they are mounted too far apart to achieve the well-distributed type of lateral and materials industrial-grade variations of ripping implements are attached to power graders and bulldozers. Although highly durable, they are shanks or "teeth" of these rippers are too short to restore soil permeability and infiltration. In addition, the power graders and bulldozers, as pullers, are far less maneuverable not recommended. Typically, soil for turns and patterns than the tractor. the fracturing of generally necessary vertical



Fig. 8. A deep, angled-leg subsoiler, ideal for Phase 2 decompaction of after the topsoil layer is graded on top of the ripped subsoil.



Fig. 9. This medium tractor is pulling a 3-shank deep ripper. The severely compacted construction access corridor is narrow, and the 120 hp tractor is more maneuverable for Phase I deep ripping (subsoil fracturing), here.

## Depth and Patterns of Movemen

As previously noted both Phase 1 Deep Ripping through significantly compressed, exposed subsoil and Phase 2 Decompaction (deep subsoiling) through the replaced topsoil and upper subsoil need to be performed at maximum capable depth of each implement. With an implement's guide wheels attached, some have a "normal" maximum operating depth of 18 inches, while others may go deeper. In many situations, however, the tractor/implement operator must first remove the guide wheels and other non essential elements from the implement. This adapts the ripper or the deep subsoiler for skilftin pulling with its frame only a few inches above surface, while the shanks or legs, fracture the soil material 20-to-24 inches deep.

There may be construction sites where the depth of the exposed subsoil's compression is moderate, e.g.: 12 inches, rather than deep. This can be verified by using a ¾ inch cone penetrometer and a shovel to test the subsoil for its level of compaction, incrementally, every three inches of increasing depth. Once the full thickness of the subsoil's compacted zone is finally "pieced" and there is a significant drop in the pin measurements of the soil penetrometer, the depth/thickness of compaction is determined. This is repeated at several representative locations of the construction site. If the thickness of the site's subsoil compaction is verified as, for example, ten inches, then the Phase 1 Deep Ripping can be correspondingly reduced to the implement's minimum operable depth of 12 inches. However, the Phase 2 simultaneous Decompation (subsoiling) of an 11 inch pherating depth.



Fig. 10. An early pass with a 3-shank deep ripper penetrating only 8 inches into this worksite's severely compressed subsoil.



Fig. 11. A repeat run of the 3-shank ripper along the same patterned pass area as Fig. 9; here, incrementally reaching 18 of the needed 22 inches of subsoil fracture.

Typically, three separate series (patterns) are used for both the Phase 1 Deep Ripping and the Phase 2 Decompaction on significantly compacted sites. For Phase 1, each series begins with a moderate depth of rip and, by repeat-pass, continues until full depth is reached. Phase 2 applies the full depth of Decompation (subsoiling), from the beginning.

Every separate series (pattern) consists of parallel, forward-and-return runs, with each progressive

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pass of the implement's legs or shanks evenly staggered between those from the previous pass. This compensates for the shank or leg-spacing on the implement, e.g., with 24-to-30 inches between each shank or leg. The staggered return pass ensures lateral and vertical fracturing actuated every 12 to 15 inches across the densely compressed soil mass.

## Large, Unobstructed Areas

For larger easy areas, use the standard patterns of movement:

- The first series (pattern) of passes is applied lengthwise, parallel with the longest spread of the site; gradually progressing across the site's width, with each successive pass.
- The second series runs obliquely, crossing the first series at an angle of about 45 degrees.
- The third series runs at right angle (or 90 degrees), to the first series to complete the fracturing and shattering on severely compacted sites, and avoid leaving large unbroken blocks of compressed soil material. (In certain instances, the third series may be optional, depending on how thoroughly the first two series loosen the material and eliminate large chunks/blocks of material as verified by tests with a ¾-inch cone penetrometer.)



Fig. 12. Moderately dry topsoil is being replaced on the affected site now that Phase 1 deep ripping of the compressed subsoil is complete.



Fig. 13. The same deep, angled-leg subsoiler shown in Fig. 7 is engaged at maximum depth for Phase 2, decompaction (deep soiling), of the replaced topsoil and the upper subsoil materials.

#### Corridors

In long corridors of limited width and less maneuverability than larger sites, e.g.: along compacted areas used as temporary construction access, a modified series of pattern passes are used.

First, apply the same initial lengthwise, parallel series of passes described above.

- A second series of passes makes a broad "S" shaped pattern of rips, continually
  and gradually alternating the "S" curves between opposite edges inside the
  compacted corridor.
- The third and final series again uses the broad, alternating S pattern, but it is "flip-flopped" to continually cross the previous S pattern along the corridor's centerline. This final series of the S pattern curves back along the edge areas skipped by the second series.

## Maintenance and Cost

Once the two-phase practice of Deep Ripping and Decompation is completed, two items are essential for maintaining a site's soil porosity and permeability for infiltration. They are: planting and maintaining the appropriate ground cover with deep roots to maintain the soil structure (see Figure 15); and keeping the site free of traffic or other weight loads.

Note that site-specific choice of an appropriate vegetative ground-cover seed mix, including the proper seeding ratio of one or more perennial species with a deep taproot system and the proper amount of lime and soil nutrients (fertilizer mix) adapted to the soil-needs, are basic to the final practice of landscaping, i.e.: surface tillage, seeding/planting/fertilizing and culti-packing or mulching is applied. The "maintenance" of an effectively deep-ripped and decompacted area is generally limited to the successful perennial (long-term) landscape ground cover; as long as no weight-bearing force of soil compaction is applied.



Fig. 14. The severely compacted soil of a temporary construction yard used daily by heavy equipment for four months; shown before deep ripping, topsoil replacement, and decompaction.



Fig. 15. The same site as Fig. 14 after deep ripping of the exposed subsoil, topsoil replacement, decompaction through the topsoil and upper subsoil and final surface tillage and revegetation to maintain soil permeability and infiltration.

The Deep Ripping and Decompaction practice is, by necessity, more extensive than periodic subsolling of farmland. The cost of deep ripping and decompacting (deep subsolling), will vary according to the depth and severity of soil-material compression and the relative amount of tractor and implement time that is required. In some instances, depending on open maneuverability, two-to-three acres of compacted project area may be deep-ripped in one day. In other situations of more severe compaction and - or less maneuverability, as little as one acre may be fully ripped in a day. Generally, if the Phase 1 Deep Ripping is fully effective, the Phase 2) Decompaction should be completed in 2/3 to 3/4 of the time required for Phase 1.

Using the example of two acres of Phase 1) Deep Ripping in one day, at \$1800 per day, the net cost is \$900 per acre. If the Phase 2) Decompacting or deep subsoiling takes 3/4 the time as Phase 1, it costs \$675 per acre for a combined total of \$1575 per acre to complete the practice (these figures do not include the cost of the separate practice of topsoil stripping and replacement). Due to the many variables, it must be recognized that cost will be determined by the specific conditions or constraints of the site and the availability of proper equipment.

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#### Resources

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   Various years. Soil Survey of (various names) Countly, New York. USDA.

#### rnet Access:

Examples of implements:

V-Rippers. Access by internet search of John Deere Ag.-New Equipment for 915 (larger-frame model) V-Rippe; and, for 913 (smaller-frame model) V-Ripper. Deep, angled-leg subsoiler. Access by internet search of Elighum Brothers Shear Balt Paratill-Subsoiler.

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  Penetrometer (soil compaction tester), PSUExtension; as well as Dickey-john Soil Compaction Tester.
  http://www.dickey-johnproducts.com/pdf/SoilCompactionTest.pdf and http://cropsoil.psu.edu/Extension/Facts/uc178pdf Last
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### APPENDIX H: LABELLA CERTIFYING PROFESSIONALS LETTER





February 21, 2025

RE: LaBella Certifying Professionals for NYSDEC SPDES GP-0-25-001

To Whom it May Concern:

In accordance with the NYSDEC SPDES General Permit GP-0-25-001, Part VII.J.2, Mufuta Tshimanga, PE, a New York State Qualified Professional employed by LaBella Associates, is duly authorized to sign and seal Stormwater Pollution Prevention Plans (SWPPPs), Notice of Intents (NOIs) and Notice of Terminations (NOTs).

Respectfully submitted,

LaBella Associates

Timothy Webber

Vice President, Civil Division Director

Mufuta Tshimanga, PE Senior Civil Engineer Renewable Market Leader

Shimanoa



### APPENDIX I: NYSDEC SPDES GENERAL PERMIT GP-0-25-001





NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION (NYSDEC)

SPDES GENERAL PERMIT FOR STORMWATER DISCHARGES

From

## CONSTRUCTION ACTIVITY

Permit No. GP-0-25-001

Construction General Permit (CGP)

Issued Pursuant to Article 17, Titles 7, 8 and Article 70

of the Environmental Conservation Law

Effective Date: January 29, 2025

Expiration Date: January 28, 2030

Scott E. Sheeley

Chief Permit Administrator

Scott 6.

SAN. 29, 2025

Authorized Signature

Address:

NYSDEC Division of Environmental Permits 625 Broadway, 4th Floor Albany, N.Y. 12233-1750

#### PREFACE

Pursuant to Section 402 of the Clean Water Act (CWA), and 40 CFR 122.26(b)(14)(x), (15)(i), and (15)(ii), stormwater discharges from certain construction activities are unlawful unless they are authorized by a National Pollutant Discharge Elimination System (NPDES) permit or by a state permit program. New York State administers the approved State Pollutant Discharge Elimination System (SPDES) program with permits issued in accordance with the New York State Environmental Conservation Law (ECL) Article 17, Titles 7 and 8, and Article 70, as well as 6 NYCRR Parts 621 and 750.

Construction activities constitute construction of a point source and, therefore, pursuant to ECL sections 17-0505, 17-0701, and 17-0803, the owner or operator must have coverage under a SPDES permit prior to commencement of construction activities. The owner or operator cannot wait until there is an actual discharge from the construction site to obtain permit coverage.

\*Note: The italicized words/phrases within this permit are defined in Appendix A.

ž "	NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION SPDES CONSTRUCTION GENERAL PERMIT (CGP) GP-0-25-001 FOR STORMWATER DISCHARGES FROM CONSTRUCTION ACTIVITIES	
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#### Part |

# Part I. How to Obtain Coverage and General Requirements

To be covered under this permit, the owner or operator must meet all eligibility requirements in Part I.A. and follow the requirements for obtaining permit coverage in Part ID, F, or G.

## A. Eligibility Requirements

For a common plan of development or sale, the phase(s) that meet the eligibility requirements in Part I.A. may obtain coverage under this permit even if other phase(s) of the same common plan of development or sale do not meet the eligibility requirements and require an individual SPDES permit.

- The owner's or operator's construction activities involve soil disturbances of:
- one or more acres; or
- less than one acre which are part of a common plan of development or sale that will ultimately disturb one or more acres; or ٥
- permit is required for stormwater discharges based on the potential for contribution to a violation of a water quality standard or for significant less than one acre where NYSDEC has determined that a SPDES contribution of pollutants to surface waters of the State. ပ
- the New York City Watershed located east of the Hudson River, 5,000 square feet or more, but less than one acre, and are in Appendix C Figure 1; or
- 20,000 square feet or more, but less than one acre, within the municipal boundaries of the City of New York (NYC); or :≓
- of development or sale that will ultimately disturb 20,000 square less than 20,000 square feet which are part of a common plan feet or more, but less than one acre, within the municipal boundaries of NYC; or ≡
- that creates 5,000 square feet or more of impervious area within the municipal boundaries of NYC. .≥

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## Part I A 2

- 2. Discharges from the owner's or operator's construction activities are/were not:
- already covered by a different SPDES permit;
- covered under a different SPDES permit that was denied, terminated, or revoked; or ٥
- identified in an expired individual SPDES permit that was not renewed; ŏ ပ
- required to obtain an individual SPDES permit or another general SPDES permit in accordance with Part VII.K. Ö
- If construction activities may adversely affect a species that is endangered or threatened, the owner or operator must obtain a: က
- permit issued pursuant to 6 NYCRR Part 182 for the project; or
- letter issued by NYSDEC of non-jurisdiction pursuant to 6 NYCRR Part 182 for the project. ٥
- 4. If construction activities have the potential to affect an historic property, the owner or operator must obtain one of the following:
- State Registers of Historic Places, and that there is no new permanent archeological buffer area indicated on the sensitivity map, and that the property listed or determined to be eligible for listing on the National or is such a new permanent building on the construction site within those building, structure, or object that is more than 50 years old, or if there building on the construction site within the following distances from a Certified Local Government, or a qualified preservation professional construction activity is not located on or immediately adjacent to a has determined that the building, structure, or object more than 50 Preservation (OPRHP), a Historic Preservation Commission of a parameters that NYS Office of Parks, Recreation and Historic documentation that the construction activity is not within an years old is not historically/archeologically significant:
- i. 1-5 acres of disturbance 20 feet; or
- ii. 5-20 acres of disturbance 50 feet; or

Part I.A.4 a.iii.

- ii 20+ acres of disturbance 100 feet.
- b. NYSDEC consultation form sent to OPRHP, ¹ and copied to NYSDEC's Agency Historic Preservation Officer (APO), and
- i. the State Environmental Quality Review Act (SEQR)
   Environmental Assessment Form (EAF) with a negative declaration or the Findings Statement, with documentation of OPRHP's agreement with the resolution; or
- ii. documentation from OPRHP that the construction activity will result in No Impact; or
- iii. documentation from OPRHP providing a determination of No Adverse Impact; or
- iv. a Letter of Resolution signed by the owner or operator, OPRHP and the DEC APO which allows for this construction activity to be eligible for coverage under the general permit in terms of the State Historic Preservation Act (SHPA).
- c. documentation of satisfactory compliance with Section 106 of the National Historic Preservation Act for a coterminous project area:
- i. No Affect; or
- ii. No Adverse Affect; or
- iii. Executed Memorandum of Agreement.
- d. documentation that SHPA Section 14.09 has been completed by NYSDEC or another state agency.
- If construction activities are subject to SEQR, the owner or operator must obtain documentation that SEQR has been satisfied.
- If construction activities are not subject to SEQR, but subject to the equivalent environmental review from another New York State or federal agency, the

owner or operator must obtain documentation that project review, pursuant to a process equivalent to SEQR from another New York State or federal agency, has been satisfied.

Part I.A.6.

- If construction activities require Uniform Procedures Act (UPA) Permits (see 6
  NYCRR Part 621) from NYSDEC, or the equivalent from another New York
  State or federal agency, the owner or operator must:
- a. obtain all such necessary permits; or
- b. receive notification from NYSDEC pursuant to 6 NYCRR 621.3(a)(4) excepting Part I.A.7.a.
- Construction activities are not eligible if they meet the following criteria in Part I.A.8.a. or b.:
- For linear transportation and linear utility project types, the construction activities:
- are within the watershed of surface waters of the State classified as AA or AA-S identified utilizing the Stormwater Interactive Map on NYSDEC's website; and
- ii. are undertaken on land with no existing impervious cover, and
- iii. disturb two or more acres of steep slope.
- b. For all other project types, the construction activities:
- are within the watershed of surface waters of the State classified as AA or AA-S identified utilizing the Stormwater Interactive Map on NYSDEC's website; and
- ii. are undertaken on land with no existing impervious cover; and
- iii disturb one or more acres of steep slope.

<sub>∞</sub>

<sup>&</sup>lt;sup>1</sup> The consultation form can be submitted, along with other project information, through OPRHP's Cultural Resource Information System (CRIS) portal. If submitted through CRIS, paper copies of the consultation form need not be mailed.

Part I.B.

## B. Types of Discharges Authorized

- 1. The following stormwater discharges are authorized under this permit:
- Stormwater discharges, including stormwater runoff, snowmelt runoff, and surface runoff and drainage, associated with construction activity, are authorized under this permit provided that appropriate stormwater controls are designed, installed, and maintained in accordance with Part II. and Part III.
- b. Stormwater discharges from construction support activities at the construction site (including concrete or asphalt batch plants, equipment staging yards, material storage areas, excavated material disposal areas, and borrow areas) if the following requirements are met:
- The support activity is directly related to the construction site required to have permit coverage for stormwater discharges; and
- ii. The support activity is not a commercial operation, nor does it serve multiple unrelated construction sites; and
- iii. The support activity does not continue to operate beyond the completion of the construction activity at the site it supports; and
- iv. Stormwater controls are implemented in accordance with Part II. and Part III. for discharges from the support activity areas.
- The following non-stormwater discharges associated with construction activity are authorized under this permit:
- a. Non-stormwater discharges listed in 6 NYCRR 750-1.2(a)(29)(vi), with the following exception: "Discharges from firefighting activities are authorized only when the firefighting activities are emergencies/unplanned"; and
- b. Non-stormwater discharges of waters to which other components have not been added that are used in accordance with the SWPPP to control dust or irrigate vegetation in stabilized areas; and
- Uncontaminated discharges from dewatering operations

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Part I.B.3.

 Authorized discharges of stormwater or authorized discharges of nonstormwater, commingled with a discharge authorized by a different SPDES permit and/or a discharge that does not require SPDES permit authorization, are also authorized under this permit.

## C. Prohibited Discharges

- Non-stormwater discharges prohibited under this permit include but are not limited to:
- Wastewater from washout of concrete; and
- Wastewater from washout and cleanout of stucco, paint, form release oils, curing compounds, and other construction materials; and
- c. Fuels, oils, or other pollutants used in vehicle and equipment operation and maintenance; and
- d. Soaps, solvents, or detergents used in vehicle and equipment washing or external building washdown; and
- e. Toxic or hazardous substances from a spill or other release.

## D. Electronic Notice of Intent (eNOI) Submittal

To receive authorization in accordance with Part I.D.3.b., the *owner or operator* must submit a complete eNOI in accordance with the requirements in Part I.D. The eNOI contains questions to: ensure eligibility requirements in Part I.A. have been met; obtain *owner or operator* contact information; obtain the total area to be disturbed and the existing/future *impervious areas* (rounded to the nearest tenth of an acre); confirm *Traditional Land Use Control MS4 Operator* jurisdiction over construction projects; satisfy the EPA eRule requirements; confirm that the Water Quality-Based Effluent Limitations in Part III, have been met; demonstrate consideration of the future risks due to climate change in accordance with Part III.A.2.; and confirm that the other *Stormwater Pollution Prevention Plan (SWPPP)* requirements in Part III, have been met.

- 1. An eNOI may be submitted for:
- a. construction activities that are not part of a common plan of development or sale; or

Part I D 1 b.

- an entire common plan of development or sale; or و.
- separate phase(s) of a common plan of development or sale if the following requirements are met: ပ
- i. the common plan of development or sale meets the eligibility requirements of Part I.A.5. or 6.; and
- the phase(s) meet(s) all other eligibility requirements of Part ÷
- Part III.C. Required SWPPP Components by Project Type is based on the common plan of development or sale, not the phase(s); or i≡
- energy generation, transmission, or storage project that meets Part d. tree clearing that is associated with, or will support, a renewable I.A.5. and 6., if the tree clearing.
- i. meets all other eligibility requirements of Part I.A.; and
- will occur in NYSDEC's Regions 3-9; and
- is not within 1/4 mile of a bat hibernaculum protected pursuant to 6 NYCRR Part 182; and i≝
- iv. will occur between November 1st and March 31st
- As prerequisites for submitting an eNOI, the owner or operator must:
- prepare a SWPPP for Part I.D.1.a., b., c., or d. in accordance with Part ď
- signed in accordance with Part VII.J. to the eNOI prior to submission: based on the following criteria, upload the following signature forms <u>.</u>
- i. for all eNOIs:
- the SWPPP Preparer Certification Form, Appendix F, signed by the SWPPP preparer; and

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Part ID2bi2

- 2. the Owner/Operator Certification Form, Appendix J, signed by the owner or operator; and
- boundary(ies) of Traditional Land Use Control MS4 Operator(s) ii. if an eNOI includes construction activities within the municipal that will discharge to the MS4(s):
- Operator(s) have review authority. A Traditional Land determine if the Traditional Land Use Control MS4 Use Control MS4 Operator does not have review authority where:
- a. the owner or operator of the construction activities Traditional Land Use Control MS4 Operator in Part I.D.2.b.ii. is the same entity as the identified in Part I D 2 b ii.; or
- there is a statute exempting the owner or operator from zoning review by the Traditional Land Use Control MS4 Operator, or ď
- there is no such statute per Part I.D.2.b.ii.1.b., the Legal Memorandum LU14 Updated January 2020 "Governmental Immunity from Zoning and Other have zoning review authority in accordance with concludes, after public hearing, that it does not Traditional Land Use Control MS4 Operator Legislation"; and ပ
- 2. if the Traditional Land Use Control MS4 Operator(s) have Land Use Control MS4 Operator(s) for review and have: review authority, submit the SWPPP to the Traditional
- signed by the principal executive officer or ranking representative of that person in accordance with if outside the municipal boundaries of NYC: the Control MS4 Operator, or by a duly authorized MS4 SWPPP Acceptance Form, Appendix G, elected official from the Traditional Land Use Part VII.J.2; or

Part I.D.2.b.ii.2.b.

- b. if within the municipal boundaries of NYC: The City of New York Department of Environmental Protection (NYCDEP) SWPPP Acceptance/Approval Form, Appendix H, signed by the principal executive officer or ranking elected official from the Traditional Land Use Control MS4 Operator, or by a duly authorized representative of that person in accordance with Part VII.J.2.; and
- 3. if the *Traditional Land Use Control MS4 Operator* does not have review authority, have the MS4 No Jurisdiction Form, Appendix I, signed by the principal executive officer or ranking elected official from the *Traditional Land Use Control MS4 Operator*, or by a duly authorized representative of that person in accordance with Part VII.J.2.

### 3. Submitting an eNOI:

- The owner or operator must submit a complete Notice of Intent electronically using a NYSDEC approved form.<sup>2</sup>
- b. The owner or operator is authorized to commence construction activity
  as of the authorization date indicated in the Letter of Authorization
  (LOA), which is sent by NYSDEC after a complete eNOI is submitted.
- i. If an eNOI is received for a SWPPP that deviates from one of the technical standards but demonstrates equivalence in accordance with Part III.B.1.a.ii. or Part III.B.2.b.ii., if the SWPPP includes construction activities that are not within the municipal boundary(ies) of Traditional Land Use Control MS4 Operator(s), and/or if the SWPPP includes construction activities within the municipal boundary(ies) of Traditional Land Use Control MS4 Operator(s) that do not have review authority in accordance with Part I.D.2.b.ii.1., the authorization date indicated in the LOA will be 60 business days after the eNOI submission date

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Part I.D.3.c.

c. If Traditional Land Use Control MS4 Operator(s) have review authority in accordance with Part I.D.2.b.ii.2., the owner or operator must, within five business days of receipt of the LOA, send an electronic copy of the LOA to the Traditional Land Use Control MS4 Operator(s) with review authority.

# E. General Requirements for Owners or Operators with Permit Coverage

- As of the date the LOA is received, the owner or operator must make the eNOI, SWPPP, and LOA available for review and copying in accordance with the requirements in Part VII.H. When applicable, as of the date an updated LOA is received, the owner or operator must make the updated LOA available for review and copying in accordance with the requirements in Part VII.H.
- The owner or operator must ensure compliance with all requirements of this
  permit and that the provisions of the SWPPP, including any changes made to
  the SWPPP in accordance with Part III.A.5., are properly implemented and
  maintained from the commencement of construction activity until:
- all areas of disturbance have achieved final stabilization; and
- b. the owner's or operator's coverage under this permit is terminated in accordance with Part V.A.5.a.
- As of the date of the commencement of construction activities until Part
   I.E.2.a. and b. have been met, the owner or operator must maintain at the construction site, a copy of:
- a. all documentation necessary to demonstrate eligibility with this permit; and
- b. this permit; and
- the SWPPP; and
- d. the signed SWPPP Preparer Certification Form; and
- the signed MS4 SWPPP Acceptance Form or signed NYCDEP SWPPP Acceptance/Approval Form or signed MS4 No Jurisdiction Form (when applicable); and
- the signed Owner/Operator Certification Form; and

<sup>&</sup>lt;sup>2</sup> Unless NYSDEC grants a waiver in accordance with 40 CFR 127.15(c) or (d). All waiver requests must be submitted to Stormwater, info@dec.ny.gov or NYSDEC, Bureau of Water Permits, 625 Broadway, 4th Floor, Albany, New York 12233-3505.

Part I.E.3.g.

- the eNOI; and
- h. the LOA; and
- the LOA transmittal to the Traditional Land Use Control MS4 Operator in accordance with Part I.D.3.c. (when applicable).
- 4. The owner or operator must maintain at the construction site, until Part I.E.2.a. and b. have been met, as of the date the documents become final or are received, a copy of the:
- responsible contractor's or subcontractor's certification statement(s) in accordance with Part III.A.7; and
- b. inspection reports in accordance with Part IV.C.4. and 6.; and
- Request to Disturb Greater Than Five Acres and the Authorization Letter to Disturb Greater Than Five Acres in accordance with Part I.E.6. (when applicable); and
- d. Request to Continue Coverage and the Letter of Continued Coverage (LOCC) in accordance with Part I.F.2, and 4. (when applicable); and
- 3. The updated LOA(s) in accordance with Part I.E.9. (when applicable).
- 5. The *owner or operator* must maintain the documents in Part I.E.3. and 4, in a secure location, such as a job trailer, on-site construction office, or mailbox with lock. The secure location must be accessible during normal business hours to an individual performing a compliance inspection. The documents must be paper documents unless electronic documents are accessible to the inspector during an inspection to the same extent as a paper copy stored at the site would be. If electronic documents are kept on site, the *owner or operator* must maintain functional equipment on site available to an inspector during normal hours of operation such that an inspector may view the electronic documents in a format that can be read in a similar manner as a paper record and in a legally dependable format with no less evidentiary value than their paper equivalent.
- The owner or operator must meet the following requirements prior to disturbing greater than five acres of soil at any one time:
- a. The owner or operator must submit a written Request to Disturb Greater Than Five Acres to:

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Part I.E.6.a.i.

- NYSDEC's Regional Office Division of Water staff based on the project location, Appendix E, if a Traditional Land Use Control MS4 Operator does not have review authority in accordance with Part I.D.2.b.ii.1.; or
- ii. the Traditional Land Use Control MS4 Operator, if a Traditional Land Use Control MS4 Operator has review authority in accordance with Part I.D.2.b.ii.1.; or
- iii. NYSDEC's Regional Office Division of Water staff based on the project location, Appendix E, and each involved *Traditional Land Use Control MS4 Operator*, if the project spans multiple municipalities with more than one *Traditional Land Use Control MS4 Operator* involved with review authority in accordance with Part I.D.2.b.ii.1.
- b. The written Request to Disturb Greater Than Five Acres must include:
- . The SPDES permit identification number (Permit ID); and
- ii. Full technical justification demonstrating why alternative methods of construction that would result in five acres of soil disturbance or less at any one time are not feasible; and
- iii. The phasing plan for the project and sequencing plans for all phases from the SWPPP in accordance with Part III.B.1.d.; and
- iv. Plans with locations and details of erosion and sediment control practices such that the heightened concern for erosion when disturbing greater than five acres at one time has been addressed; and
- Acknowledgment that "the owner or operator will comply with the requirements in Part IV.C.2.b."; and
- vi. Acknowledgment that "the owner or operator will comply with the requirements in Part II.B.1.b."
- c. The owner or operator must be in receipt of an Authorization Letter to Disturb Greater Than Five Acres, which will include when the

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authorization begins and ends and indicate a maximum area (acres) of soil disturbance allowed at any one time, from:

- i NYSDEC, if Part I.E.6.a.i. or iii. apply; or
- ii. the Traditional Land Use Control MS4 Operator, if Part I.E.6.a.ii. applies.
- 7. Upon a finding of significant non-compliance with the practices described in the SWPPP or violation of this permit, NYSDEC may order an immediate stop to all construction activity at the site until the non-compliance is remedied. The stop work order must be in writing, describe the non-compliance in detail, and be sent to the owner or operator.
- 8. If any human remains or archaeological remains are encountered during excavation, the *owner or operator* must immediately cease, or cause to cease, all construction activity in the area of the remains and notify the appropriate Regional Water Engineer (RWE).<sup>3</sup> Construction activity shall not resume until written permission to do so has been received from the RWE.
- To be authorized to implement modifications to the information previously submitted in the eNOI, the owner or operator must:
- a. notify NYSDEC via email at Stormwater\_info@dec.ny.gov requesting access to update the eNOI; and
- b. update the eNOI to reflect the modifications and resubmit the eNOI in accordance with Part I.D.; and
- c. receive an updated LOA.
- 10. The eNOI, SWPPP, LOA, updated LOAs (when applicable), and inspection reports required by this permit are public documents that the owner or operator must make available for review and copying by any person within five business days of the owner or operator receiving a written request by any such person to review these documents. Copying of documents will be done at the requester's expense.

<sup>3</sup> The Regional Water Manager where a DEC Region does not have a RWE.

Part I.F.

# F. Permit Coverage for Discharges Authorized Under GP-0-20-001

When applicable:

- Upon the effective date of this permit, an owner or operator of a construction activity, with coverage under GP-0-20-001, will have interim coverage under GP-0-25-001 for 45 calendar days starting on the effective date of GP-0-25-001 so long as the owner or operator maintains compliance with all applicable requirements of this permit.
- Within 30 calendar days of the effective date of this permit, the *owner or operator*, with coverage under GP-0-20-001, must submit a complete Request to Continue Coverage electronically using a NYSDEC approved form,<sup>4</sup> which contains the information identified in Part I.F.3. below, if:
- a. the owner or operator continues to implement the SMP component in conformance with the technical standards in place at the time of initial project authorization; and
- b. the owner or operator will comply with all non-design requirements of GP-0-25-001.
- 3. The Request to Continue Coverage form contains questions to: ensure eligibility requirements in Part I.A. have been met; verify owner or operator contact information; verify the permit identification number; verify the original eNOI submission ID, if applicable; verify Part I.F.2.a. and b.; verify the version of the Design Manual that the technical/design components conform to; and receive an updated Owner/Operator Certification Form, Appendix I.
- 4. The owner or operator has obtained continued coverage under GP-0-25-001 as of the date indicated in the LOCC, which is sent by NYSDEC after a complete Request to Continue Coverage form is submitted.
- If the owner or operator does not submit the Request to Continue Coverage form in accordance with Part I.F.2. and 3., coverage under this permit is automatically terminated after interim coverage expires.

<sup>&</sup>lt;sup>4</sup> Unless NYSDEC grants a waiver in accordance with 40 CFR 127.15(c) or (d). All waiver requests must be submitted to Stormwater\_info@dec.ny.gov or NYSDEC, Bureau of Water Permits, 625 Broadway, 4<sup>th</sup> Floor, Albany, New York 12233-3505.

Part I G

## G. Change of Owner or Operator

When applicable:

- When property ownership changes, or when there is a change in operational control over the construction plans and specifications, the following process applies:
- The new owner or operator must meet the applicable prerequisites for submitting an eNOI in accordance with Part I.D.2.; and
- b. The new owner or operator must submit an eNOI in accordance with Part I.D.3.; and
- Permit coverage for the new owner or operator will be effective upon receipt of the LOA in accordance with Part I.D.3.b.; and
- d. The new owner or operator, upon receipt of their LOA, must provide their Permit ID to the original owner or operator, and
- e. If the original *owner or operator* will no longer be the *owner or operator* of the *construction activity* identified in the original *owner's or operator's* eNOI, the original *owner or operator*, upon receipt of the new *owner's or operator's* Permit ID in accordance with Part I.G.1.d., must submit to NYSDEC a completed eNOT in accordance with Part V. that includes the name and Permit ID of the new *owner or operator*; or
- f. If the original owner or operator maintains ownership of a portion of the construction activity, the original owner or operator must maintain their coverage under the permit by modifying their eNOI; modifications to the eNOI must include:
- i. the revised area of disturbance and/or impervious area(s); and
- ii. the revised SMP information, if applicable, and
- iii. a narrative description of what has changed; and
- iv. the new owner's or operator's Permit ID for the portion of the project removed from the eNOI.

Owners or operators must follow Part I.E.9, to modify the eNOI

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Part II.

## Part III. Water Quality-Based Effluent Limitations

## A. Maintaining Water Quality

NYSDEC expects that compliance with the requirements of this permit will control discharges necessary to meet applicable water quality standards. It shall be a violation of the ECL for any discharge to either cause or contribute to a violation of the following water quality standards as contained in Parts 700 through 705 of Title 6 of the Official Compilation of Codes, Rules and Regulations of the State of New York:

- There must be no increase in turbidity that will cause a substantial visible contrast to natural conditions; and
- There must be no increase in suspended, colloidal or settleable solids that will cause deposition or impair the waters for their best usages; and
- There must be no residue from oil and floating substances, nor visible oil film, nor globules of grease.

If there is evidence indicating that the stormwater discharges authorized by this permit are causing, have the reasonable potential to cause, or are contributing to a violation of the water quality standard, the owner or operator must take appropriate corrective action in accordance with Part IV.C.5. of this permit and document in accordance with Part IV.C.4 of this permit. To address the water quality standard violation the owner or operator must include and implement appropriate controls in the SWPPP to correct the problem or obtain an individual SPDES permit.

If, despite compliance with the requirements of this permit, it is demonstrated that the stormwater discharges authorized by this permit are causing or contributing to a violation of water quality standards, or if NYSDEC determines that a modification of this permit is necessary to prevent a violation of water quality standards, the authorized discharges will no longer be eligible for coverage under this permit, and the owner or operator must obtain an individual SPDES permit prior to further discharges from the construction site.

# B. Effluent Limitations Applicable to Discharges from Construction Activities

Discharges authorized by this permit must achieve, at a minimum, the effluent limitations in Part II.B.1.a., b., c., d., and e. These limitations represent the

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degree of effluent reduction attainable by the application of best practicable technology currently available.

- 1. Erosion and Sediment Control Requirements The *owner or operator* must select, design, install, implement, and maintain control measures to *minimize* the *discharge* of *pollutants* and prevent a violation of the *water quality standards*. The selection, design, installation, implementation, and maintenance of these control measures must meet the non-numeric effluent limitations in Part II.B. 1.a., b., c., d., and e. and be in accordance with the New York State Standards and Specifications for Erosion and Sediment Control (BB), dated November 2016, using sound engineering judgment. Where control measures are not designed in conformance with the design criteria included in the technical standard, the *owner or operator* must include in *SWPPP* the reason(s) for the deviation, or alternative design, and provide information in the *SWPPP* demonstrating that the deviation or alternative design is *equivalent* to the technical standard.
- Erosion and Sediment Controls. At a minimum, erosion and sediment controls must be selected, designed, installed, implemented, and maintained to:
- Minimize soil erosion through application of runoff control and soil stabilization control measure to minimize pollutant discharges; and
- ii. Control stormwater discharges, including both peak flow rates and total stormwater volume, to minimize channel and streambank erosion and scour in the immediate vicinity of the discharge points; and
- iii. Minimize the amount of soil exposed during construction activity, and
- iv. Minimize the disturbance of steep slope; and
- v. Minimize sediment discharges from the site; and
- vi. Provide and maintain natural buffers around surface waters, direct stormwater to vegetated areas and maximize stormwater infiltration to reduce pollutant discharges, unless infeasible; and
- vii. Minimize soil compaction. Minimizing soil compaction is not required

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Part II.B.1.a.vii.

where the intended function of a specific area of the site dictates that it be compacted; and

- viii. Unless infeasible, preserve a sufficient amount of topsoil to complete soil restoration and establish a uniform, dense vegetative cover; and
- ix. Minimize dust. On areas of exposed soil, minimize dust through the
  appropriate application of water or other dust suppression techniques
  to control the generation of pollutants that could be discharged from
  the site.
- b. Soil Stabilization. In areas where soil disturbance activity has ceased, whether permanently or temporarily ceased, the application of soil stabilization measures must be initiated by the end of the next business day and completed within 14 calendar days from the date the current soil disturbance activity ceased. For construction sites that directly discharge to one of the 303(d) segments listed in Appendix D, or are located in one of the watersheds listed in Appendix C, or are authorized to disturb greater than five acres in accordance with Part I.E.5.a.viii. the application of soil stabilization measures must be initiated by the end of the next business day and completed within seven calendar days from the date the soil disturbance activity ceased.
- Cowatering. Discharges from dewatering activities, including discharges from dewatering of trenches and excavations, must be managed by appropriate control measures.
- d. Pollution Prevention Measures. Select, design, install, implement, and maintain effective pollution prevention measures to minimize the discharge of pollutants and prevent a violation of the water quality standards. At a minimum, such measures must be selected, designed, installed, implemented, and maintained to:
- Minimize the discharge of pollutants from equipment and vehicle washing, wheel wash water, and other wash waters. Soaps, detergents and solvents cannot be used; and
- ii. Minimize the exposure of building materials, building products, construction wastes, trash, landscape materials, fertilizers, pesticides, herbicides, detergents, sanitary waste, hazardous and toxic waste, and other materials present on the site to precipitation

### Part II.B.1.d.ii.

and to stormwater. Minimization of exposure is not required in cases where the exposure to precipitation and to stormwater will not result in a discharge of pollutants, or where exposure of a specific material or product poses little risk of stormwater contamination (such as final products and materials intended for outdoor use); and

- iii. Prevent the discharge of pollutants from spills and leaks and implement chemical spill and leak prevention and response procedures.
- Surface Outlets. When discharging from basins and impoundments, the surface outlets must be designed, constructed, and maintained in such a manner that sediment does not leave the basin or impoundment and that erosion at or below the outlet does not occur.

# C. Post-Construction Stormwater Management Practice (SMP) Requirements

- The owner or operator of a construction activity that requires post-construction SMPs, in accordance with Part III.C., must select, design, install, implement, and maintain the SMPs to meet the performance criteria in the New York State Stormwater Management Design Manual, dated July 31, 2024 (DM), using sound engineering judgment. Where SMPs are not designed in conformance with the performance criteria in the DM, the owner or operator must include in the SWPPP the reason(s) for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is equivalent to the technical standard.
- The owner or operator of a construction activity, that requires SMPs in accordance with Part III.C., must design the practices to meet the applicable sizing criteria in Part II.C.2.a., b., c., or d.

## a. Sizing Criteria for New Development

- i. Runoff Reduction Volume (RRv) and Water Quality Volume (WQv):
- Reduce the total WQv by application of RR techniques and standard SMPs with RRv capacity. The total WQv must be calculated in accordance with the criteria in Section 4.2 of the DM; or

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Part II.C.2 a.i.2.

2. Minimum RRv and Treatment of Remaining Total WQv: Construction activities that cannot meet the requirements in Part II.C.2.a.i.1. due to site limitations must direct runoff from all newly constructed impervious areas to a RR technique or standard SMP with RRv capacity unless infeasible. The specific site limitations that prevent the reduction of 100% of the WQv must be documented in the SWPPP. For each impervious area that is not directed to a RR technique or standard SMP with RRv capacity, the SWPPP must include documentation which demonstrates that all options were considered and for each option explains why it is considered infeasible.

In no case shall the runoff reduction achieved from the newly constructed *impervious areas* be less than the Minimum RRv as calculated using the criteria in Section 4.4 of the DM. The remaining portion of the total WQv that cannot be reduced must be treated by application of standard SMPs.

- ii. Channel Protection Volume (CPv): Provide 24 hour extended detention of the post-developed 1-year, 24-hour storm event, remaining after runoff reduction. Where a CPv control orifice is provided, the minimum orifice size must be 3 inches, with acceptable external trash rack or orifice protection. The CPv requirement does not apply when:
- Reduction of the entire CPv is achieved by application of runoff reduction techniques or infiltration systems; or
- The 1-year post-development peak discharge is less than or equal to 2.0 cfs without detention or velocity controls; or
- 3. The site directly discharges into a fifth order or larger water body (stream, river, or lake), or tidal waters, where the increase in smaller flows will not impact the stream bank or channel integrity. However, the point of discharge must be adequately protected against scour and erosion by the increased peak discharge.

Part II.C.2.a.iii.

- iii. Overbank Flood Control Criteria (Qp): Requires storage to attenuate the post-development 10-year, 24-hour peak discharge rate (Qp) to predevelopment rates. The Qp requirement does not apply when:
- the site directly discharges to tidal waters or fifth order or larger streams, or
- A downstream analysis reveals that overbank control is not required.
- iv. Extreme Flood Control Criteria (Qf): Requires storage to attenuate the post-development 100-year, 24-hour peak discharge rate (Qf) to predevelopment rates. The Qf requirement does not apply when:
- the site directly discharges to tidal waters or fifth order or larger streams, or
- A downstream analysis reveals that overbank control is not required.

# b. Sizing Criteria for New Development in Enhanced Phosphorus Removal Watersheds

- i. Runoff Reduction Volume (RRv) and Water Quality Volume (WQv):
- 1. Reduce the WQv by application of RR techniques and standard SMPs with RRv capacity. The total WQv is the runoff volume from the 1-year, 24-hour design storm over the post-developed watershed and must be calculated in accordance with the criteria in Section 4.3 of the DM; or
- Minimum RRv and Treatment of Remaining Total WQv:
   Construction activities that cannot meet the criteria in Part
   ILC.2.b.i.1. due to site limitations must direct runoff from all
   newly constructed impervious areas to a RR technique or
   standard SMP with RRv capacity unless infeasible. The
   specific site limitations that prevent the reduction of 100% of
   the WQv must be documented in the SWPPP. For each
   impervious area that is not directed to a RR technique or
   standard SMP with RRv capacity, the SWPPP must include

Part II.C.2.b.i.2.

documentation which demonstrates that all options were considered and for each option explains why it is considered infeasible

In no case shall the runoff reduction achieved from the newly constructed *impervious areas* be less than the Minimum RRv as calculated using the criteria in Section 4.5 of the DM. The remaining portion of the total WQv that cannot be reduced must be treated by application of standard SMPs.

- ii. Channel Protection Volume (CPv): Provide 24 hour extended detention of the post-developed 1-year, 24-hour storm event, remaining after runoff reduction. Where a CPv control orifice is provided, the minimum orifice size must be 3 inches, with acceptable external trash rack or orifice protection. The CPv requirement does not apply when:
- Reduction of the entire CPv is achieved by application of runoff reduction techniques or infiltration systems; or
- The 1-year post-development peak discharge is less than or equal to 2.0 cfs; or
- The site directly discharges to tidal waters, or a fifth order or larger water body (stream, river, or lake) where the increase in smaller flows will not impact the stream bank or channel integrity. However, the point of discharge must be adequately protected against scour and erosion by the increased peak discharge.
- iii. Overbank Flood Control Criteria (Qp): Requires storage to attenuate the post-development 10-year, 24-hour peak discharge rate (Qp) to predevelopment rates. The Qp requirement does not apply when:
- the site directly discharges to tidal waters or fifth order or larger streams; or
- A downstream analysis reveals that overbank control is not required.

Part II.C.2.b.iv.

- iv. Extreme Flood Control Criteria (Qf): Requires storage to attenuate the post-development 100-year, 24-hour peak discharge rate (Qf) to predevelopment rates. The Qf requirement does not apply when:
- the site directly discharges to tidal waters or fifth order or larger streams; or
- A downstream analysis reveals that overbank control is not required.

## c. Sizing Criteria for Redevelopment Activity

- Water Quality Volume (WQv): The WQv treatment objective for redevelopment activity must be addressed by one of the following options, as outlined in Section 9.2.1. Redevelopment activities located in an Enhanced Phosphorus Removal Watershed (see Part III.B.3. and Appendix C) must calculate the WQv in accordance with Section 4.3 of the DM. All other redevelopment activities must calculate the WQv in accordance with Section 4.2 of the DM.
- Reduce the existing impervious cover by a minimum of 25% of the total disturbed, impervious area. The Soil Restoration criteria in Section 5.1.6 of the DM must be applied to all newly created pervious areas; or
- Capture and treat 100% of the required WQv, for a minimum of 25% of the disturbed redevelopment *impervious area*, by implementation of standard SMPs or reduced by application of runoff reduction techniques; or
- Capture and treat 100% of the required WQv, for a minimum of 75% of the disturbed redevelopment impervious area, by implementation of a volume-based alternative SMP, as defined in Section 9.4 of the DM; or
- Capture and treat 100% of the required WQv, for a minimum of 75% of the disturbed redevelopment impervious area, by implementation of a flow-through alternative SMP sized to treat the peak rate of runoff from the WQv design storm; or

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Part II.C.2.c.i.5.

- 5. Application of a combination of 1 through 4 above that provide a weighted average of at least two of the above methods. Application of this method must be in accordance with the criteria in Section 9.2.1(A)(V) of the DM; or
- 6. If there is an existing SMP located on the site that captures and treats runoff from the *impervious* area that is being disturbed, the WQv treatment option selected must, at a minimum, provide treatment equal to the treatment that was being provided by the existing practice(s) if that treatment is greater than the treatment required by options 1 through 5 above.
- ii. Channel Protection Volume (CPv) is not required if there is 0% change to hydrology that increases the discharge rate and volume from the project site.
- iii. Overbank Flood Control (Qp) is not required if there is 0% change to hydrology that increases the *discharge* rate from the project site.
- iv. Extreme Flood Control (Qf) is not required if there is 0% change to hydrology that increases the discharge rate from the project site.

# d. Sizing Criteria for Combination of Redevelopment Activity and New Development

Construction projects, that include both new development and redevelopment activity, must use SMPs that meet the sizing criteria calculated as an aggregate of the sizing criteria in Part II.C.2.a. or b. for the new development portion of the project and Part II.C.2.c. for the redevelopment activity portion of the project.

Part III. Stormwater Pollution Prevention Plan (SWPPP)

## A. General SWPPP Requirements

 A SWPPP must be prepared and implemented by the owner or operator of all construction activity covered by this permit. All authorized discharges must be identified in the SWPPP. The SWPPP must document the selection, design, installation, implementation and maintenance of the control measures and

Part III.A.1.

practices that will be used to meet the effluent limitations in Part II.B. and, where applicable, the SMP requirements in Part II.C.

- The SWPPP must demonstrate consideration in narrative format of the future physical risks due to climate change pursuant to the Community Risk and Resiliency Act (CRRA), 6 NYCRR Part 490, and associated guidance.
- a The owner or operator must consider:
- i. the following physical risks due to climate change:
- (i) increasing temperature; and
- (ii) increasing precipitation; and
- (iii) increasing variability in precipitation, including chance of drought; and
- (iv) increasing frequency and severity of flooding; and
- (v) rising sea level; and
- (vi) increasing storm surge; and
- (vii) shifting ecology.
- ii. for each of the following:
- (i) overall site planning; and
- (ii) location, elevation, and sizing of:
- a. control measures and practices; and
- b. conveyance system(s); and
- c. detention system(s).
- The SWPPP must describe the erosion and sediment control practices and where required, SMPs that will be used and/or constructed to reduce the pollutants in stormwater discharges and to assure compliance with the

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requirements of this permit. In addition, the SWPPP must identify potential sources of pollution which may reasonably be expected to affect the quality of stormwater discharges.

- All SWPPPs, that require the SMP component in accordance with Part III.B.2., must be prepared by a qualified professional.
- The owner or operator must keep the SWPPP current so that, at all times, it
  accurately documents the erosion and sediment control practices that are
  being used or will be used during construction, and all SMPs that will be
  constructed on the site. At a minimum, the owner or operator must modify the
  SWPPP, including construction drawings:
- a. whenever the current provisions prove to be ineffective in minimizing pollutants in stormwater discharges from the site; and
- b. whenever there is a change in design, construction, or operation at the construction site that has or could have an effect on the discharge of pollutants; and
- to address issues or deficiencies identified during an inspection by the qualified inspector, NYSDEC, or other regulatory authority; and
- d. to document the final construction conditions in an as-built drawing.
- 6. NYSDEC may notify the *owner or operator* at any time that the SWPPP does not meet one or more of the minimum requirements of this permit. The notification must be in writing and identify the provisions of the SWPPP that require modification. Within fourteen (14) calendar days of such notification, or as otherwise indicated by NYSDEC, the *owner or operator* must make the required changes to the SWPPP and submit written notification to NYSDEC that that the changes have been made. If the *owner or operator* does not respond to NYSDEC's comments in the specified time frame, NYSDEC may suspend the *owner's or operator's* coverage under this permit or require the *owner or operator* to obtain coverage under an individual SPDES permit in accordance with Part II.D.4.
- 7. Prior to the commencement of construction activity, the owner or operator must identify the contractor(s) and subcontractor(s) that will be responsible for installing, constructing, repairing, replacing, inspecting, and maintaining the erosion and sediment control practices included in the SWPPP and the

Part III.A.7.

contractor(s) and subcontractor(s) that will be responsible for constructing the SMPs included in the SWPPP. The owner or operator must have each of the company to be trained contractor that will be responsible for implementation of the SWPPP. The owner or operator must ensure that at least one trained contractors and subcontractors identify at least one person from their contractor is on site daily when soil disturbance activities are being The owner or operator must have each of the contractors and subcontractors identified above sign a copy of the following certification statement below before the commencement of construction activities:

water quality standards. Furthermore, I am aware that there are significant comply with the requirements of the SWPPP and agree to implement any inspection. I also understand that the owner or operator must comply with Permit (CGP) for Stormwater Discharges from Construction Activities and penalties for submitting false information, that I do not believe to be true, including the possibility of fine and imprisonment for knowing violations" Pollutant Discharge Elimination System (SPDES) Construction General that it is unlawful for any person to cause or contribute to a violation of "I hereby certify under penalty of law that I understand and agree to the requirements of the most current version of the New York State corrective actions identified by the qualified inspector during a site

contractor and subcontractor will be responsible for and include the name and description) of the site; and the date the certification statement is signed. The owner or operator must attach the certification statement(s) to the copy of the contractor responsible for SWPPP implementation; the name, address and contractors are hired to implement measures identified in the SWPPP after title of the person providing the signature; the name and title of the trained telephone number of the contracting firm; the address (or other identifying In addition to providing the certification statement above, the certification certification statement and provide the information listed above prior to SWPPP that is maintained at the construction site. If new or additional page must also identify the specific elements of the SWPPP that each the commencement of construction activities, they must also sign the performing construction activities.

Part III.B.

## B. Required SWPPP Contents

- prepare a SWPPP that includes erosion and sediment control practices. 1. Erosion and sediment control component - The owner or operator must
- a. Erosion and sediment control practices must be designed:
- i. in conformance with the BB; or
- ii. equivalent to the BB if deviating from Part III.B.1.a.i.
- If the erosion and sediment control practices are designed in conformance with Part III.B.1.a.ii., the SWPPP must include a demonstration of equivalence to the BB.
- At a minimum, the erosion and sediment control component of the SWPPP must include the following: ပံ
- Background information about the scope of the project, including the location, type and size of project; and
- A site map/construction drawing(s) with north arrows for the project. including a general location map. At a minimum, the site map must adjacent off-site surface water(s); floodplain/floodway boundaries; show the total site area; all improvements; areas of disturbance; location(s) of the stormwater discharge(s) and receiving surface areas that will not be disturbed; existing vegetation; on-site and different soil types with boundaries; material, waste, borrow or wetlands and drainage patterns that could be affected by the equipment storage areas located on adjacent properties; and construction activity; existing and final contours; locations of water(s); and
- A description of the soil(s) present at the site, including an identification of the Hydrologic Soil Group (HSG); and ≔
- iv. A phasing plan for the project and sequencing plans for all phases, both of which must address clearing and grubbing, excavation and grading, utility and infrastructure installation, final stabilization,

Part III.B.1.c.iv.

and any other construction activity at the site that will result in soil disturbance.

- 1. The phasing plan must include:
- a map delineating and labeling the limits of soil disturbance for all phases of a project; and
- b. a table identifying the order and intended schedule of when each phase will begin and end its sequencing plan. The table must identify the total disturbed area for each phase at any one time and the total disturbed area for the overall project at any one time all on one timeline showing all overlapping quantities of disturbed area at any one time; and
- 2. A sequencing plan for a specific phase must include:
- a table indicating the order and intended schedule of construction activities within a phase, and corresponding construction drawings with a description of the work to be performed; and
- b. all permanent and temporary stabilization measures; and
- A description of the minimum erosion and sediment control
  practices to be installed or implemented for each construction
  activity that will result in soil disturbance. Include a schedule that
  identifies the timing of initial placement or implementation of each
  erosion and sediment control practice and the minimum time
  frames that each practice should remain in place or be
  implemented; and
- vi. A site map/construction drawing(s) showing the specific location(s), size(s), and length(s) of each erosion and sediment control practice; and
- vii. The dimensions, material specifications, installation details, and operation and maintenance requirements for all erosion and sediment control practices. Include the location and sizing of any

Part III.B.1.c.vii.

temporary sediment basins and structural practices that will be used to divert flows from exposed soils; and

- viii. A maintenance inspection schedule for the contractor(s) and subcontractor(s) identified in Part III.A.7. to ensure continuous and effective operation of the erosion and sediment control practices. The maintenance inspection schedule must be in accordance with the requirements in the BB technical standard; and
- ix. A description of the pollution prevention measures that will be used
  to control litter, construction chemicals and construction debris from
  becoming a pollutant source in the stormwater discharges; and
- A description and location of any stormwater discharges associated with industrial activity other than construction at the site, including, but not limited to, stormwater discharges from asphalt plants and concrete plants located on the construction site; and
- xi. Identification of any elements of the design that are not in conformance with the design criteria in the BB technical standard.
   Include the reason for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is equivalent to the technical standard.
- SMP component The owner or operator of construction activity identified in Table 2 of Appendix B must prepare a SWPPP that includes SMPs.
- a. SMPs must be designed in conformance with the applicable sizing criteria in Part II.C.2.a., c., or d.; and
- b. SMPs must be designed in conformance with the performance criteria:
- i. in the DM; or
- ii. equivalent to the DM if deviating from Part III.B.2.b.i.; or
- iii. in the New York State Stormwater Management Design Manual, dated January 2015 (2015 Design Manual), or equivalent to it, if the following oriteria are met:

### Part III.B.2.b.iii.1.

- The eNOI is submitted in accordance with Part I.D. before January 29, 2027 for construction activities that are either:
- a. subject to governmental review and approval:
- where the owner or operator made any application to that governmental entity prior to the effective date of this permit; and
- ii. such application included a SWPPP developed using the 2015 Design Manual or equivalent to
- b. not subject to governmental review and approval:
- i. where a fiscal allocation for the construction activities has been developed and approved by a governmental entity; and
- ii. the SWPPP was developed using the 2015 Design Manual or equivalent to it; and
- If SMPs are designed in conformance with Part III.B.2.b.ii., the SWPPP must include the reason(s) for the deviation or alternative design and a demonstration of equivalence to the DM; and
- d. If SMPs are designed in conformance with Part III.B.2.b.iii., the SWPPP must include supporting information or documentation demonstrating that Part III.B.2.b.iii.1.a. or b. apply; and
- e. The SMP component of the SWPPP must include the following:
- Identification of all SMPs to be constructed as part of the project, including which option the SMP designs conform to, either Part III.B.2.b.i., ii., or iii. Include the dimensions, material specifications and installation details for each SMP; and
- i. A site map/construction drawing(s) showing the specific location and size of each SMP; and

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Part III B.2 e iii.

- A Stormwater Modeling and Analysis Report that includes:
- (i) Map(s) showing pre-development conditions, including watershed/subcatchments boundaries, flow paths/routing, and design points; and
- (ii) Map(s) showing post-development conditions, including watershed/subcatchments boundaries, flow paths/routing, design points and SMPs; and
- (iii) Results of stormwater modeling (i.e. hydrology and hydraulic analysis) for the required storm events. Include supporting calculations (model runs), methodology, and a summary table that compares pre- and post-development runoff rates and volumes for the different storm events;
- (iv) Summary table, with supporting calculations, which demonstrates that each SMP has been designed in conformance with the sizing criteria included in the DM:
- (v) Identification of any sizing criteria that is not required based on the requirements included in Part II.C.; and
- (vi) Identification of any elements of the design that are not in conformance with the performance criteria in the DM.
   Include the reason(s) for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is equivalent to the DM.
- Soil testing results and locations (test pits, borings); and
- Infiltration test results, when required in accordance with Part III.B.2.a.; and
- vi. An operations and maintenance plan that includes inspection and maintenance schedules and actions to ensure continuous and effective operation of each SMP. The plan must identify the entity

Part III B.2 e vi

that will be responsible for the long-term operation and maintenance of each practice; and

Enhanced Phosphorus Removal Standards - The owner or operator of
construction activity identified in Table 2 of Appendix B that is located in a
watershed identified in Appendix C must prepare a SWPPP that includes
SMPs designed in conformance with the applicable sizing criteria in Part
I.C.2.b., c., or d. and the performance criteria Enhanced Phosphorus
Removal Standards included in the DM. At a minimum, the SMP component
of the SWPPP must meet the requirements of Part III.B.2.

# C. Required SWPPP Components by Project Type

Owners or operators of construction activities, identified in Table 1 of Appendix B, are required to prepare a SWPPP that only includes erosion and sediment control practices designed in accordance with Part III.B.1. Owners or operators of the construction activities, identified in Table 2 of Appendix B, must prepare a SWPPP that also includes SMPs designed in accordance with Part III.B.2 or 3.

For the entire area of disturbance, including the entire common plan of development or sale if applicable, the owner or operator must evaluate every bullet from Appendix B Table 1 and Table 2 separately. If bullets from both Table 1 and Table 2 apply, the SWPPP must include erosion and sediment control practices for all construction activities but SMPs for only those portions of the construction activities that fall under Table 2 bullet(s).

# Part IV. Inspection and Maintenance Requirements

# A. General Construction Site Inspection and Maintenance Requirements

The owner or operator must ensure that all erosion and sediment control
practices (including pollution prevention measures), and all SMPs identified in
the SWPPP, are inspected and maintained in accordance with Part IV.B. and

# B. Contractor Maintenance Inspection Requirements

 The owner or operator of each construction activity, identified in Tables 1 and 2 of Appendix B, must have a trained contractor inspect the erosion and sediment control practices and pollution prevention measures being

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Part IV.B.1.

implemented within the active work area daily to ensure that they are being maintained in effective operating condition at all times. If deficiencies are identified, the contractor must:

- a. if the corrective action does not require engineering design:
- i. begin implementing corrective actions within one business day; and
- ii. complete the corrective actions within five business days; or
- b. if the corrective action requires engineering design:
- begin the engineering design process within five business days;
- ii. complete the corrective action in a reasonable time frame but no later than within 60 calendar days.
- For construction sites where soil disturbance activities have been temporarily suspended (e.g. winter shutdown) and temporary stabilization measures have been applied to all disturbed areas, the trained contractor can stop conducting the maintenance inspections in accordance with Part IV.B.1. The trained contractor must begin conducting the maintenance inspections in accordance with Part IV.B.1. as soon as soil disturbance activities resume.
- 3. For construction sites where soil disturbance activities have been shut down with partial project completion, the trained contractor can stop conducting the maintenance inspections in accordance with Part IV.B.1. if all areas disturbed as of the project shutdown date have achieved final stabilization and all SMPs required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational.

## C. Qualified Inspector Inspection Requirements

- With the exception of the following construction activities identified in Tables 1 and 2 of Appendix B, a qualified inspector must conduct site inspections for all other construction activities identified in Tables 1 and 2 of Appendix B:
- a. the construction of a single-family residential subdivision with 25% or less
  impervious cover at total site build-out that involves a soil disturbance of
  one (1) or more acres of land but less than or equal to five (5) acres and is

Part IV.C.1.a.

<u>not</u> located in one of the watersheds listed in Appendix C and <u>not</u> directly discharging to one of the 303(d) segments listed in Appendix D; and

- b. the construction of a single-family home that involves soil disturbances of one (1) or more acres but less than or equal to five (5) acres and is <u>not</u> located in one of the watersheds listed in Appendix C and <u>not</u> directly discharging to one of the 303(d) segments listed in Appendix D; and
- c. construction on agricultural property that involves soil disturbances of one
   (1) or more acres but less than five (5) acres; and
- d. construction activities located in the New York City Watershed located east of the Hudson River, see Appendix C Figure 1, that involve soil disturbances of 5,000 square feet or more, but less than one acre.
- The qualified inspector must conduct site inspections in accordance with the following timetable:
- a. For construction sites where soil disturbance activities are on-going, the qualified inspector must conduct a site inspection at least once every seven (7) calendar days; or
- b. For construction sites where soil disturbance activities are on-going and the owner or operator has received authorization in accordance with Part I.E.6. to disturb greater than five (5) acres of soil at any one time, the qualified inspector must conduct at least two (2) site inspections every seven (7) calendar days. The two (2) inspections must be separated by a minimum of two (2) full calendar days; or
- c. For *construction sites* where soil disturbance activities have been temporarily suspended (e.g. winter shutdown) and *temporary stabilization* measures have been applied to all disturbed areas, the *qualified inspector* must conduct a site inspection at least once every thirty (30) calendar days. The *owner or operator* must notify the DOW Water (SPDES) Program contact at the Regional Office (see contact information in Appendix E) or, in areas under the jurisdiction of a *Traditional Land Use Control MS4 Operator*, the *Traditional Land Use Control MS4 Operator* (provided the *Traditional Land Use Control MS4 Operator* or *operator* of the *construction activity*) by hard copy or email prior to reducing the inspections to this frequency and again by hard copy or email prior to re-commencing construction; or

Part IV.C.2 d.

down with partial project completion, the requirement to have the qualified conformance with the SWPPP and are operational. The owner or operator required for the completed portion of the project have been constructed in are not the owners or operators of the construction activity) in writing prior the date of shutdown, the owner or operator must terminate coverage by Office (see contact information in Appendix E) or, in areas subject to the activity. If soil disturbance activities are not resumed within 2 years from accordance with Part I.D.2.b.ii.1., the Traditional Land Use Control MS4 Operator(s) (provided the Traditional Land Use Control MS4 Operator(s) must notify the DOW Water (SPDES) Program contact at the Regional For construction sites where soil disturbance activities have been shut project shutdown date have achieved final stabilization and all SMPs review authority of Traditional Land Use Control MS4 Operator(s) in inspector conduct inspections ceases if all areas disturbed as of the to the shutdown and again in writing prior to resuming construction meeting the requirements of Part V; or

e. For *construction sites* involving soil disturbance of one (1) or more acres that *directly discharge* to one of the 303(d) segments listed in Appendix D or is located in one of the watersheds listed in Appendix C, the *qualified inspector* must conduct at least two (2) site inspections every seven (7) calendar days. The two (2) inspections must be separated by a minimum of two (2) full calendar days.

3. At a minimum, the qualified inspector must inspect:

 all erosion and sediment control practices and pollution prevention measures to ensure integrity and effectiveness; and  b. all SMPs under construction to ensure that they are constructed in conformance with the SWPPP; and .. all areas of disturbance that have not achieved final stabilization; and

 d. all points of discharge to surface waters of the State located within, or immediately adjacent to, the property boundaries of the construction site; and

all points of discharge from the construction site.

### Part IV.C.4.

- 4. The qualified inspector must prepare an inspection report subsequent to each and every inspection. At a minimum, the inspection report must include and/or address all of the following, for all construction activities except those listed in Part IV.C.1.:
- a. Permit identification number; and
- . Date and time of inspection; and
- c. Name and title of person(s) performing inspection; and
- d. A description of the weather and soil conditions (e.g. dry, wet, saturated)
  at the time of the inspection, including the temperature at the time of the
  inspection; and
- A description of the condition of the runoff at all points of discharge from the construction site. This must include identification of any discharges of sediment from the construction site. Include discharges from conveyance systems (i.e. pipes, culverts, ditches, etc.) and overland flow; and
- f. A description of the condition of all surface waters of the State located within, or immediately adjacent to, the property boundaries of the construction site which receive runoff from disturbed areas. This must include identification of any discharges of sediment to the surface waters of the State; and
- g. Identification of all erosion and sediment control practices and pollution prevention measures that need repair or maintenance; and
- Identification of all erosion and sediment control practices and pollution
  prevention measures that were not installed properly or are not functioning
  as designed and need to be reinstalled or replaced; and
- Description and sketch (map) of areas with active soil disturbance activity, areas that have been disturbed but are inactive at the time of the inspection, and areas that have been stabilized (temporary and/or final) since the last inspection; and
- j. Estimates, in square feet or acres, of the following areas:

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Part IV C 4 j i

- Total area with active soil disturbance (not requiring either temporary stabilization or final stabilization); and
- ii. Total area with inactive soil disturbance (requiring either temporary stabilization or final stabilization); and
- iii. Total area that has achieved temporary stabilization; and
- iv. Total area that has achieved final stabilization; and
- k. Current stage of construction of all SMPs and identification of all
  construction activity on site that is not in conformance with the SWPPP
  and technical standards; and
- Corrective action(s) that must be taken to install, repair, replace or maintain erosion and sediment control practices and pollution prevention measures; and to correct deficiencies identified with the construction of the SMP(s); and
- Identification and status of all corrective actions that were required by previous inspection; and
- n. Digital photographs, with date stamp, that clearly show the condition of all practices that have been identified as needing corrective actions. The qualified inspector must attach color copies of the digital photographs to the inspection report being maintained onsite within seven (7) calendar days of the date of the inspection. The qualified inspector must also take digital photographs, with date stamp, that clearly show the condition of the practice(s) after the corrective action has been completed. The qualified inspector must attach paper color copies of the digital photographs to the inspection report that documents the completion of the corrective action work within seven (7) calendar days of that inspection.
- Within one business day of the completion of an inspection, the qualified inspector must notify the owner or operator, and appropriate contractor or subcontractor identified in Part III.A.7., of any corrective actions that need to be taken. The contractor or subcontractor must:
- a. if the corrective action does not require engineering design:

Part V A 1 b

### Part IV C.5 a i

- i. begin implementing corrective actions within one business day; and
- complete the corrective actions within five business days; or
- b. if the corrective action requires engineering design;
- begin the engineering design process within five business days;
- . complete the corrective action in a reasonable time frame but no later than within 60 calendar days.
- All inspection reports must be signed by the qualified inspector. In accordance with Part I.E.3., the inspection reports must be maintained on site with the SWPPP.

Part V. How to Terminate CGP Coverage

# V. Electronic Notice of Termination (eNOT) Submittal

The eNOT contains questions to ensure requirements in Part V.A. have been

- An owner or operator must terminate coverage when one or more of the following requirements have been met:
- a Total project completion:
- i. all construction activity identified in the SWPPP has been completed; and
- ii. all areas of disturbance have achieved final stabilization; and
- iii. all temporary, structural erosion and sediment control measures have been removed; and
- iv. all SMPs have been constructed in conformance with the SWPPP and are operational; and
- v. an as-built drawing has been prepared; or

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b. Planned shutdown with partial project completion:

- i. all soil disturbance activities have ceased; and
- ii. all areas disturbed as of the project shutdown date have achieved final stabilization; and
- iii. all temporary, structural erosion and sediment control measures have been removed; and
- iv. all SMPs required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational; and
- v. an as-built drawing has been prepared; or
- c. In accordance with Part I.G. Change of Owner or Operator; or
- d. The owner or operator has obtained coverage under an alternative general SPDES permit or an individual SPDES permit.
- For construction activities that require qualified inspector inspections in accordance with Part IV.C.1. and have met Part V.A.1.a. or b., the owner or operator must have the qualified inspector perform a final site inspection prior to submitting the eNOT. The qualified inspector must, by signing the "Final Stabilization" and "Post-Construction Stormwater Management Practice(s)" certification statements on the eNOT, certify that all the requirements in Part V.A.1.a. or b. have been achieved.
- 3. For construction activities that are subject to the review authority of Traditional Land Use Control MS4 Operator(s) in accordance with Part LD.2.b.ii.1. and meet Part V.A.1.a. or b., the owner or operator must have the Traditional Land Use Control MS4 Operator(s) sign the "MS4 Acceptance" statement on the eNOT in accordance with the requirements in Part VII.J. A Traditional Land Use Control MS4 Operator official, by signing this statement, determined that it is acceptable for the owner or operator to submit the eNOT in accordance with the requirements of this Part. A Traditional Land Use Control MS4 Operator can make this determination by performing a final site inspection themselves or by accepting the qualified inspector's final site inspection certification(s) when required in Part V.A.2.

Part V.A.4

- For construction activities that require SMPs and meet Part V.A.1.a. or b., the owner or operator must, prior to submitting the eNOT, ensure one of the following:
- a. for SMP(s) that were constructed by a private entity, but will be owned, operated, and maintained by a public entity, the SMP(s) and any right-ofway(s) needed to operate and maintain such practice(s) have been deeded to the municipality in which the practice(s) is located; or
- b. for SMP(s) that are privately owned, but will be operated and maintained by a public entity, an executed operation and maintenance agreement is in place with the municipality that will operate and maintain the SMP(s); or
- c. for SMP(s) that are privately owned, the *owner or operator* has a mechanism in place that requires operation and maintenance of the practice(s) in accordance with the operation and maintenance plan, such as a deed covenant in the *owner or operator's* deed of record; or
- d. for SMP(s) that are owned by a public or private institution (e.g. school, university, hospital), government agency or authority, or public utility, the owner or operator has policies and procedures in place that ensure operation and maintenance of the practices in accordance with the operation and maintenance plan.
- An owner or operator that has met the requirements of Part V.A.1, 2, 3, and 4.
  must request termination of coverage under this permit by submitting a complete
  Notice of Termination form electronically using a NYSDEC approved form.<sup>5</sup>
- a. The owner's or operator's coverage is terminated as of the termination date indicated in the Letter of Termination (LOT), which is sent by NYSDEC after a complete eNOT is submitted.

<sup>5</sup> Unless NYSDEC grants a waiver in accordance with 40 CFR 127.15(c) or (d). All waiver requests must be submitted to Stormwater info@dec.ny.gov or NYSDEC, Bureau of Water Permits, 625 Broadway, 4<sup>th</sup> Floor, Albany, New York 12233-3505.

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Part VI.

## Part VI. Record Retention and Reporting

### A. Record Retention

The *owner or operator* must retain a copy of the documents listed in Part I.E.3. and a copy of the LOT for a period of at least five years from the date that NYSDEC accepts a complete NOT submitted in accordance with Part V.

### B. Reporting

Except for the eNOI, the signature forms associated with the eNOI, and the eNOT, all other written correspondence requested by NYSDEC, including individual permit applications, must be sent to the address of the appropriate DOW (SPDES) Program contact at the Regional Office listed in Appendix E.

## Part VII. Standard Permit Requirements

For the purposes of this permit, examples of contractors and subcontractors include: third-party maintenance and construction contractors.

### A. Duty to Comply

The *owner or operator*, and all contractors or subcontractors, must comply with all requirements of this permit. Any non-compliance with the requirements of this permit constitutes a violation of the New York State Environmental Conservation Law (ECL), and its implementing regulations, and is grounds for enforcement action. Filing of a request for termination of coverage under this permit, or a notification of planned changes or anticipated non-compliance, does not limit, diminish or stay compliance with any requirements of this permit.

## B. Need to Halt or Reduce Activity Not a Defense

The necessity to halt or reduce the construction activity regulated by this permit, in order to maintain compliance with the requirements of this permit, must not be a defense in an enforcement action.

### C. Penalties

There are substantial criminal, civil, and administrative penalties associated with violating the requirements of this permit. Fines of up to \$37,500 per day for each

Part VII.C.

violation and imprisonment for up to 15 years may be assessed depending upon the nature and degree of the offense.

### D. False Statements

Any person who knowingly makes any false material statement, representation, or certification in any application, record, report, or other document filed or required to be maintained under this permit, including monitoring reports or reports of compliance or noncompliance must, upon conviction, be punished in accordance with ECL §71-1933 and or New York State Penal Law Articles 175 and 210.

### E. Re-Opener Clause

Upon issuance of this permit, a determination has been made on the basis of a submitted Notice of Intent, plans, or other available information, that compliance with the specified permit requirements will reasonably protect classified water use and assure compliance with applicable water quality standards. Satisfaction of the requirements of this permit notwithstanding, if operation pursuant to this permit causes or contributes to a condition in contravention of State water quality standards or guidance values, or if NYSDEC determines that a modification is necessary to prevent impairment of the best use of the waters or to assure maintenance of water quality standards or compliance with other provisions of ECL Article 17 or the Clean Water Act (CWA), or any regulations adopted pursuant thereto, NYSDEC may require such modification and the Commissioner may require abatement action to be modification has been implemented.

### F. Duty to Mitigate

The owner or operator, and its contractors and subcontractors, must take all reasonable steps to minimize or prevent any discharge in violation of this permit which has a reasonable likelihood of adversely affecting human health or the environment.

# G. Requiring Another General Permit or Individual SPDES Permit

NYSDEC may require any *owner or operator* authorized to *discharge* in accordance with this permit to apply for and obtain an individual SPDES permit or apply for authorization to *discharge* in accordance with another general SPDES permit.

 Cases where an individual SPDES permit or authorization to discharge in accordance with another general SPDES permit may be required include, but is not limited to the following:

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Part VII G 1 a

- a. the owner or operator is not in compliance with the conditions of this
  permit or does not meet the requirements for coverage under this permit;
- b. a change has occurred in the availability of demonstrated technology or practices for the control or abatement of pollutants applicable to the point source; and
- new effluent limitation guidelines or new source performance standards are promulgated that are applicable to point sources authorized to discharge in accordance with this permit; and
- d. existing effluent limitation guidelines or new source performance standards that are applicable to point sources authorized to discharge in accordance with this permit are modified; and
- a water quality management plan containing requirements applicable to such point sources is approved by NYSDEC; and
- f. circumstances have changed since the time of the request to be covered so that the owner or operator is no longer appropriately controlled under this permit, or either a temporary or permanent reduction or elimination of the authorized discharge is necessary; and
- . the discharge is in violation of section 17-0501 of the ECL; and
- h. the *discharge(s)* is a significant contributor of *pollutants*. In making this determination, NYSDEC may consider the following factors:
- i. the location of the discharge(s) with respect to surface waters of the State: and
- ii. the size of the discharge(s); and
- iii. the quantity and nature of the pollutants discharged to surface waters of the State; and
- iv. other relevant factors including compliance with other provisions of ECL Article 17, or the CWA.
- When NYSDEC requires any owner or operator authorized by this permit to apply for an individual SPDES permit as provided for in this subdivision, it must notify the owner or operator in writing that a permit application is required. This notice must include a brief statement of the reasons for this decision, an application

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form, a statement setting a time for the *owner or operator* to file the application for an individual SPDES permit, and a deadline, not sooner than 180 days from the *owner's or operator's* receipt of the notification letter, whereby the authorization to *discharge* under this permit must be terminated. NYSDEC may grant additional time upon demonstration, to the satisfaction of the RWE, <sup>6</sup> that additional time to apply for an alternative authorization is necessary or where NYSDEC has not provided a permit determination in accordance with 6 NYCRR Part 621

3. When an individual SPDES permit is issued to an owner or operator authorized to discharge under this permit for the same discharge(s), this permit authorization for construction activities authorized under the individual SPDES permit is automatically terminated on the effective date of the individual SPDES permit unless termination is earlier in accordance with 6 NYCRR Part 750.

## H. Duty to Provide Information

The *owner or operator* must furnish to NYSDEC, within five business days, unless otherwise set forth by NYSDEC, any information that NYSDEC may request to determine whether cause exists to determine compliance with this permit or to determine whether cause exists for requiring an individual SPDES permit in accordance with 6 NYCRR 750-1.21(e) (see Part VII.G. Requiring Another General Permit or Individual Permit).

The *owner or operator* must make available to NYSDEC, for inspection and copying, or furnish to NYSDEC within 25 business days of receipt of a NYSDEC request for such information, any information retained in accordance with this permit.

Except for Part I.D.4. and 5. and Part I.G., the following applies: where the *owner or operator* becomes aware that it failed to submit any relevant facts on the Notice of Intent, or submitted incorrect information in a Notice of Intent or in any report to NYSDEC, the *owner or operator* must submit such facts or corrected information to NYSDEC within five business days.

### I. Extension

In the event a new permit is not issued and effective prior to the expiration of this permit, and this permit is extended pursuant to the State Administrative Procedure Act and 6 NYCRR Part 621, then the *owner or operator* with coverage under this permit may continue to operate and *discharge* in accordance with the requirements of this permit until a new permit is issued and effective.

<sup>6</sup> The Regional Water Manager where a DEC Region does not have a RWE.

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Part VII.J.

## Signatories and Certification

The Notice of Intent, Notice of Termination, and reports required by this permit must be signed as provided in 40 CFR §122.22.

- 1. All Notices of Intent and Notices of Termination must be signed as follows:
- For a corporation. By a responsible corporate officer. For the purpose of this section, a responsible corporate officer means:
- a president, secretary, treasurer, or vice-president of the corporation in charge of a principal business function, or any other person who performs similar policy- or decision-making functions for the corporation; or
- (ii) the manager of one or more manufacturing, production or operating facilities, provided, the manager is authorized to make management decisions which govern the operation of the regulated facility including having the explicit or implicit duty of making major capital investment recommendations, and initiating and directing other comprehensive measures to assure long term environmental compliance with environmental laws and regulations; the manager can ensure that the necessary systems are established or actions taken to gather complete and accurate information for Notice of Intent or Notice of Termination requirements; and where authority to sign documents has been assigned or delegated to the manager in accordance with corporate procedures.

Note: NYSDEC does not require specific assignments or delegations of authority to responsible corporate officers identified in 40 CFR §122.22(a)(1)(i). NYSDEC will presume that these responsible corporate officers have the requisite authority to sign the Notice of Intent or Notice of Fermination unless the corporation has notified NYSDEC to the contrary. Corporate procedures governing authority to sign a Notice of Intent or Notice of Termination may provide for assignment or delegation to applicable corporate positions under 40 CFR §122.22(a)(1)(ii) rather than to specific individuals.

 b. For a partnership or sole proprietorship. By a general partner or the proprietor, respectively.

### Part VII.J.1.c.

- For a municipality, State, Federal, or other public agency. By either a principal executive officer or ranking elected official. For purposes of this section, a principal executive officer of a Federal agency includes:
- the chief executive officer of the agency; or
- a senior executive officer having responsibility for the overall operations of a principal geographic unit of the agency (e.g., Regional Administrators of EPA).
- All reports required by this permit, and other information requested by NYSDEC, must be signed by a person described in Part VII.J.1., or by a duly authorized representative of that person. A person is a duly authorized representative only if:
- a. The authorization is made in writing by a person described in Part VII.J.1.
   or using the Duly Authorized Form, found on the DEC website; and
- b. The authorization specifies either an individual or a position having responsibility for the overall operation of the regulated facility or activity, position of equivalent responsibility, or an individual or position having overall responsibility for environmental matters for the company. (A duly authorized representative may thus be either a named individual or any individual occupying a named position); and
- c. The written authorization is submitted to NYSDEC
- 3. Changes to authorization. If an authorization under Part VII.J.2. is no longer accurate because a different individual or position has responsibility for the overall operation of the construction activity, a new authorization satisfying the requirements of Part VII.J.2. must be submitted to NYSDEC prior to or together with any reports, information, or applications to be signed by an authorized representative.
- Certification. Any person signing a document under Part VII.J.1. or 2. must make the following certification:

I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who

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manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

5. Electronic reporting. If documents described in Part VII.J.1. or 2. are submitted electronically by or on behalf of the *construction activity* with coverage under this permit, any person providing the electronic signature for such documents must meet all relevant requirements of this section, and must ensure that all of the relevant requirements of 40 CFR Part 3 (including, in all cases, subpart D to Part 3) (Cross-Media Electronic Reporting) and 40 CFR Part 127 (NPDES Electronic Reporting Requirements) are met for that submission.

## K. Inspection and Entry

The *owner or operator* must allow NYSDEC, the USEPA Regional Administrator, the applicable county health department, or any authorized representatives of those entities, or, in the case of a *construction site* which *discharges* through an *MS4*, an authorized representative of the *MS4* receiving the *discharge*, upon the presentation of credentials and other documents as may be required by law, to:

- enter upon the owner's or operator's premises where a regulated facility or activity is located or conducted or where records must be kept under the requirements of this permit; and
- have access to and copy at reasonable times, any records that must be kept under the requirements of this permit, including records required to be maintained for purposes of operation and maintenance; and
- inspect at reasonable times any facilities, equipment (including monitoring and control equipment), practices or operations regulated or required under this permit; and
- sample or monitor at reasonable times, for the purposes of assuring general SPDES permit compliance or as otherwise authorized by the CWA or ECL, any substances or parameters at any location; and
- 5. enter upon the property of any contributor to the regulated facility or activity under authority of the *owner or operator*.

Part VII.L.

## L. Confidentiality of Information

The following must not be held confidential: this permit, the fact sheet for this permit, the name and address of any owner or operator, effluent data, the Notice of Intent, and information regarding the need to obtain an individual permit or an alternative general SPDES permit. This includes information submitted on forms themselves and any attachments used to supply information required by the forms (except information submitted on usage of substances). Upon the request of the owner or operator, NYSDEC must make determinations of confidentiality in accordance with 6 NYCRR Part 616, except as set forth in the previous sentence. Any information accorded confidential status must be disclosed to the Regional Administrator upon his or her written request. Prior to disclosing such information to the Regional Administrator of the confidential status of such information.

## M. Other Permits May Be Required

Nothing in this permit relieves the owner or operator from a requirement to obtain any other permits required by law.

# N. NYSDEC Orders or Civil Decrees/Judgments

The issuance of this permit by the NYSDEC, and the coverage under this permit by the *owner or operator*, does not supersede, revoke, or rescind any existing order on consent or civil Decree/Judgment, or modification to any such documents or to any order issued by the Commissioner, or any of the terms, conditions, or requirements contained in such order or modification therefore, unless expressly noted.

### O. Property Rights

Coverage under this permit does not convey any property rights in either real or personal property, or any exclusive privileges, nor does it authorize any injury to private property or any invasion of personal rights, nor any infringement of Federal, State, or local laws or regulations, nor does it obviate the necessity of obtaining the assent of any other jurisdiction as required by law for the *discharge* authorized.

# P. Compliance with Interstate Standards

If the construction activity covered by this permit originates within the jurisdiction of an interstate water pollution control agency, then the construction activity must also comply with any applicable effluent standards or water quality standards promulgated by that interstate agency and as set forth in this permit for such construction activities.

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Part VII.Q.

# Q. Oil and Hazardous Substance Liability

Coverage under this permit does not affect the imposition of responsibilities upon, or the institution of any legal action against, the *owner or operator* under section 311 of the CWA, which must be in conformance with regulations promulgated pursuant to section 311 governing the applicability of section 311 of the CWA to *discharges* from facilities with NPDES permits, nor must such issuance preclude the institution of any legal action or relieve the *owner or operator* from any responsibilities, liabilities, or penalties to which the *owner or operator* from any be subject pursuant to the Comprehensive Environmental Response, Compensation and Liability Act of 1980, 42 U.S.C. section 9601 et seq. (CERCLA).

### R. Severability

The provisions of this permit are severable, and if any provision of this permit, or the application of any provision of this permit to any circumstance, is held invalid, the application of such provision to other circumstances, and the remainder of this permit, must not be affected thereby.

## S. NYSDEC Approved Forms

The owner or operator must provide all relevant information that is requested by NYSDEC, and required by this permit, on all NYSDEC approved forms.

# APPENDIX A – Abbreviations and Definitions

### **Abbreviations**

APO - Agency Preservation Officer

BB – New York State Standards and Specifications for Erosion and Sediment Control (Blue Book), dated November 2016

BMP – Best Management Practice

CPESC - Certified Professional in Erosion and Sediment Control

CPv - Channel Protection Volume

CWA - Clean Water Act (or the Federal Water Pollution Control Act, 33 U.S.C. §1251 et

DM - New York State Stormwater Management Design Manual (Design Manual), dated July 31, 2024

DOW - Division of Water

EAF - Environmental Assessment Form

ECL - chapter 43-B of the Consolidated Laws of the State of New York, entitled the

Environmental Conservation Law

EPA - U.S. Environmental Protection Agency

MS4 – Municipal Separate Storm Sewer System

HSG - Hydrologic Soil Group

NOT - Notice of Termination NOI - Notice of Intent

NPDES - National Pollutant Discharge Elimination System

NYC - The City of New York

NYCDEP - The City of New York Department of Environmental Protection

NYSDEC - The New York State Department of Environmental Conservation

OPRHP - Office of Parks, Recreation and Historic Places Qf – Extreme Flood

Qp - Overbank Flood

RRv - Runoff Reduction Volume RR - Runoff Reduction

RWE – Regional Water Engineer

SEQR - State Environmental Quality Review Act SHPA – State Historic Preservation Act

SMP - Post-Construction Stormwater Management Practice

SPDES - State Pollutant Discharge Elimination System

SWPPP - Stormwater Pollution Prevention Plan

FMDL - Total Maximum Daily Load

UPA – Uniform Procedures Act

USDA - United States Department of Agriculture WQv - Water Quality Volume

Definitions

Appendix A

All definitions in this section are solely for the purposes of this permit. If a word is not italicized in the permit, use its common definition.

structure designed, constructed or used, in whole or in part, for human habitation, as a implements, hay, grain, poultry, livestock or other horticultural products; excluding any place of employment where agricultural products are processed, treated or packaged, Agricultural Building – a structure designed and constructed to house farm or as a place used by the public. Agricultural Property – the land for construction of a barn, agricultural building, silo, stockyard, pen or other structural practices identified in Table II in the "Agricultural Best Management Practice Systems Catalogue" (dated June 2023). **Alter Hydrology from Pre- to Post-Development Conditions** – the post-development peak flow rate(s) has increased by more than 5% of the pre-developed condition for the design storm of interest (e.g. 10 yr and 100 yr).

Combined Sewer System – a sewer system which conveys sewage and stormwater through a single pipe system to a publicly owned treatment works.

material, and the initial installation of erosion and sediment control practices required in Commence (Commencement of) Construction Activities – the initial disturbance of soils associated with clearing, grading or excavation activities; or other construction related activities that disturb or expose soils such as demolition, stockpiling of fill the SWPPP. See definition for "Construction Activity(ies)" also.

markings, etc.) indicating *construction activities* may occur on a specific plot. A *common* (SEQR) environmental assessment form or other documents, zoning request, computer Common Plan of Development or Sale – a contiguous area where multiple separate advertisement, drawing, permit application, State Environmental Quality Review Act and distinct construction activities are occurring, or may occur, under one plan. The design, etc.) or physical demarcation (including boundary signs, lot stakes, surveyor "common plan" of development or sale is broadly defined as any announcement or piece of documentation (including a sign, public notice or hearing, marketing plan plan of development or sale is comprised of two or more phases.

1/4 mile apart provided any interconnecting road, pipeline or utility project that is part of construction activities that are occurring, or may occur, under one plan that are at least Common plan of development or sale does not include separate and distinct the same "common plan" is not concurrently being disturbed.

Construction Activity(ies) – identified within 40 CFR 122.26(b)(14)(x),

include, but are not limited to, mechanized logging equipment operation, the cutting and demolition or stockpiling activities that result in soil disturbance. Clearing activities can 122.26(b)(15)(i), and 122.26(b)(15)(ii), any clearing, grading, excavation, filling, skidding of trees, stump removal and/or brush root removal. *Construction activity* does <u>not</u> include routine maintenance that is performed to maintain excluded from the calculation of the soil disturbance for a project. Routine maintenance the original line and grade, hydraulic capacity, or original purpose of a facility, which is includes, but is not limited to:

- Re-grading of gravel roads or parking lots; and
- Cleaning and shaping of existing roadside ditches and culverts that maintains the approximate original line and grade, and maintains or improves the hydraulic capacity of the ditch; and
  - Replacement of existing culverts that maintains the approximate original line
- Replacement of existing bridges that maintains the approximate original line and grade, and maintains or improves the hydraulic capacity of a ditch; and and grade, and maintains or improves the hydraulic capacity beneath the
- Cleaning and shaping of existing roadside ditches that does not maintain the approximate original grade, hydraulic capacity and purpose of the ditch if the changes to the line and grade, hydraulic capacity or purpose of the ditch are installed to improve water quality and quantity controls (e.g. installing grass lined ditch); and bridges; and
- Placement of aggregate shoulder backing that stabilizes the transition between the road shoulder and the ditch or embankment; and
  - concrete pavement slabs, and similar work that does not expose soil or disturb Full depth milling and filling of existing asphalt pavements, replacement of the bottom six (6) inches of subbase material; and
    - Long-term use of equipment storage areas at or near highway maintenance Removal of sediment from the edge of the highway to restore a previously facilities; and
      - existing sheet-flow drainage connection from the highway surface to the highway ditch or embankment; and
- Existing use of Canal Corp owned upland disposal sites for the canal, and
  - Replacement of curbs, gutters, sidewalks and guide rail posts; and
    - Maintenance of ski trails including brush hog use and mowing; and
- Above ground snowmaking pipe replacement; and
  - Replacement of existing utility poles; etc.

Construction Site - the land area where construction activity(ies) will occur. See also the definitions for "Commence (Commencement of) Construction Activities" and "Common Plan of Development or Sale.

Appendix A

Dewatering – the act of draining rainwater and/or groundwater from building foundations, vaults or excavations/trenches. **Directly Discharge(s)(ing) (to a specific surface waterbody)** – runoff flows from a construction site by overland flow and the first point of discharge is the specific surface waterbody, or runoff flows from a construction site to a separate storm sewer system and the first point of discharge from the separate storm sewer system is the specific surface waterbody. Discharge(s)(d) - any addition of any pollutant to waters of the State through an outlet or point source

Embankment – an earthen or rock slope that supports a road/highway.

longevity, maintenance, and safety objectives of the technical standard and will provide Equivalent (Equivalence) – the practice or measure meets all the performance, an equal or greater degree of water quality protection. Final Stabilization – all soil disturbance activities have ceased and a uniform, perennial vegetative cover with a density of eighty (80) percent over the entire pervious surface has been established; or other *equivalent* stabilization measures, such as permanent disturbed areas that are not covered by permanent structures, concrete or pavement. landscape mulches, rock rip-rap or washed/crushed stone have been applied on all

State or National Registers of Historic Places or is determined to be eligible for listing on Historic Property – any building, structure, site, object or district that is listed on the the State or National Registers of Historic Places.

rainfall. This includes paved, concrete and compacted gravel surfaces (i.e. parking lots, Impervious Area (Cover) – all impermeable surfaces that cannot effectively infiltrate driveways, roads, runways and sidewalks); building rooftops and miscellaneous impermeable structures such as patios, pools, and sheds.

Infeasible - not technologically possible, or not economically practicable and achievable considering best industry practices.

measures (including best management practices) that are technologically available and Minimize(ing)(ation) – reduce and/or eliminate to the extent achievable using control economically practicable and achievable in light of best industry practices.

conveyances (including roads with drainage systems, municipal streets, catch basins, Municipal Separate Storm Sewer System (MS4) - a conveyance or system of curbs, gutters, ditches, man-made channels, or storm drains):

- owned or operated by a State, city, town, village, borough, county, parish, district, association, or other public body (created by or pursuant to State law) having jurisdiction over disposal of sewage, industrial wastes, stormwater, or other wastes, including special districts under State law such as a sewer district, flood control district or drainage district, or similar entity, or an Indian tribe or an authorized Indian tribal organization, or a designated and approved management agency under section 208 of the CWA, that discharges to surface waters of the State; and
- 2. designed or used for collecting or conveying stormwater; and
- 3. which is not a combined sewer system; and
- which is not part of a Publicly Owned Treatment Works (POTW) as defined at 40 CFR 122.2.

Natural Buffer(s) – an undisturbed area with natural cover running along a surface water (e.g. wetland, stream, river, lake, etc.).

**New Development** – any land disturbance that does not meet the definition of Redevelopment Activity included in this appendix.

New York State Erosion and Sediment Control Certificate Program – a certificate program that establishes and maintains a process to identify and recognize individuals who are capable of developing, designing, inspecting and maintaining erosion and sediment control plans on projects that disturb soils in New York State. The certificate program is administered by the New York State Conservation District Employees Association.

Nonpoint Source(s) – any source of water pollution or pollutants which is not a discrete conveyance or point source permitted pursuant to Title 7 or 8 of Article 17 of the Environmental Conservation Law (see ECL Section 17-1403).

Overbank – flow events that exceed the capacity of the stream channel and spill out into the adjacent floodplain.

Owner or Operator – the person, persons, or legal entity which owns or leases the property on which the construction activity is occurring; an entity that has operational control over the construction plans and specifications, including the ability to make modifications to the plans and specifications; and/or an entity that has day-to-day operational control of those activities at a project that are necessary to ensure compliance with the permit requirements.

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Appendix A

Performance Criteria – the six performance criteria for each group of SMPs in Chapters 5 and 6 of the technical standard, New York State Stormwater Management Design Manual (DM), dated July 31, 2024. These include feasibility, conveyance, pretreatment, treatment, landscaping, and maintenance. It does not include the Sizing Criteria (i.e. WQv, RRv, CPv, Qp and Qf) in Part I.C.2. of the permit.

Phase – a defined area in which construction activities are occurring or will occur separate from other defined area(s).

Point Source – any discernible, confined, and discrete conveyance, including but not limited to any pipe, ditch, channel, tunnel, conduit, well, discrete fissure, container, rolling stock, concentrated animal feeding operation, vessel or other floating craft, or landfill leachate collection system from which pollutants are or may be discharged.

Pollutant(s) – dredged spoil, filter backwash, solid waste, incinerator residue, sewage, garbage, sewage sludge, munitions, chemical wastes, biological materials, radioactive materials, heat, wrecked or discarded equipment, rock, sand and industrial, municipal, agricultural waste and ballast discharged into water; which may cause or might reasonably be expected to cause pollution of the waters of the state in contravention of the standards or guidance values adopted as provided in 6 NYCRR Parts 700 et seq.

Qualified Inspector – a person that is knowledgeable in the principles and practices of erosion and sediment control, such as a licensed Professional Engineer, Certified Professional in Erosion and Sediment Control (CPESC), Registered Landscape Architect, New York State Erosion and Sediment Control Certificate Program holder or other NYSDEC endorsed individual(s).

It can also mean someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided that person has training in the principles and practices of erosion and sediment control. Training in the principles and practices of erosion and sediment control means that the individual working under the direct supervision of the licensed Professional Engineer or Registered Landscape Architect has received four (4) hours of NYSDEC endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other NYSDEC endorsed entity. After receiving the initial training, the individual working under the direct supervision of the licensed Professional Engineer or Registered Landscape Architect shall receive four (4) hours of training every three (3) years.

It can also mean a person that meets the Qualified Professional qualifications in addition to the Qualified Inspector qualifications.

Note: Inspections of any SMPs that include structural components, such as a dam for an impoundment, shall be performed by a licensed Professional Engineer.

preparing SWPPPs that require the SMP component must have an understanding of the Qualified Professional - a person that is knowledgeable in the principles and practices (see Article 145), shall be prepared by, or under the direct supervision of, a professional SWPPP that involve the practice of engineering, as defined by the NYS Education Law control design, and, in many cases, the principles of hydraulics. All components of the of stormwater management and treatment, such as a licensed Professional Engineer, Registered Landscape Architect or other NYSDEC endorsed individual(s). Individuals principles of hydrology, water quality management practice design, water quantity engineer licensed to practice in the State of New York.

within five (5) years of preliminary project plan submission to the local government (i.e. impervious area, including impervious areas that were removed from a project site Redevelopment Activity(ies) – the disturbance and reconstruction of existing site plan, subdivision, etc.)

systems through use of the following technologies: solar thermal, photovoltaics, on land tidal energy, wave energy, ocean thermal, and fuel cells which do not utilize a fossil fuel and offshore wind, hydroelectric, geothermal electric, geothermal ground source heat, Renewable Energy - electricity or thermal energy generated by renewable energy resource in the process of generating electricity. Site Limitations - site conditions that prevent the use of an infiltration technique and or testing (i.e. test pits, soil borings, and infiltration test) or using information from the most infiltration of the total WQv. Typical site limitations include: seasonal high groundwater, The existence of site limitations shall be confirmed and documented using actual field shallow depth to bedrock, and soils with an infiltration rate less than 0.5 inches/hour. current United States Department of Agriculture (USDA) Soil Survey for the County where the project is located.

Sizing Criteria - the criteria included in Part I.C.2 of the permit that are used to size SMPs. The criteria include; Water Quality Volume (WQv), Runoff Reduction Volume (RRv), Channel Protection Volume (Cpv), Overbank Flood (Qp), and Extreme Flood

Agriculture (USDA) Soil Survey as Soil Slope Phase D, (provided the map unit name or description is inclusive of slopes greater than 25%), or Soil Slope Phase E or F, Steep Slope - land area designated on the current United States Department of (regardless of the map unit name), or a combination of the three designations.

surface features, which flows or will flow off the land by surface runoff to waters of the Stormwater - that portion of precipitation that, once having fallen to the ground, is in excess of the evaporative or infiltrative capacity of soils, or the retentive capacity of

Streambank – the terrain alongside the bed of a creek or stream. The bank consists of the sides of the channel, between which the flow is confined.

identifies the potential sources of pollution at the construction site; describes and shows Stormwater Pollution Prevention Plan (SWPPP) – a project specific report, including Part III of the permit for a complete description of the information that must be included construction drawings, that among other things: describes the construction activity(ies), sediment controls; for many projects, includes SMPs); and identifies procedures the owner or operator will implement to comply with the requirements of the permit. See the stormwater controls that will be used to control the pollutants (i.e. erosion and in the SWPPP

with natural surface waters), which are wholly or partially within or bordering the state or ponds, impounding reservoirs, springs, rivers, streams, creeks, estuaries, marshes, inlets, canals, the Atlantic ocean within the territorial seas of the state of New York and within its jurisdiction. Waters of the state are further defined in 6 NYCRR Parts 800 to public or private (except those private waters that do not combine or effect a junction all other bodies of surface water, natural or artificial, inland or coastal, fresh or salt, Surface Waters of the State - shall be construed to include lakes, bays, sounds,

Temporarily Ceased – an existing disturbed area will not be disturbed again within 14 calendar days of the previous soil disturbance.

but are not limited to, mulch, seed and mulch, and erosion control mats (e.g. jute twisted Temporary Stabilization – exposed soil has been covered with material(s) as set forth Sediment Control, to prevent the exposed soil from eroding. The materials can include, in the technical standard, New York Standards and Specifications for Erosion and yarn, excelsior wood fiber mats).

stipulates Waste Load Allocations (WLA) for point source discharges, Load Allocations quality standards, and an allocation of that amount to the pollutant's sources. A TMDL pollutant from all contributing point and nonpoint sources. It is a calculation of the maximum amount of a pollutant that a waterbody can receive and still meet water Total Maximum Daily Load (TMDL) - the sum of the allowable loads of a single (LA) for nonpoint sources, and a margin of safety (MOS).

General Permit For Stormwater Discharges from Municipal Separate Stormwater Sewer Systems (MS4s) or the City of New York's Individual SPDES Permit for their Municipal control authority that is authorized to discharge under New York State DEC's SPDES Traditional Land Use Control MS4 Operator - a city, town, or village with land use Separate Storm Sewer Systems (NY-0287890).

identified in Part III.A.7., that has received four (4) hours of NYSDEC endorsed training Trained Contractor – an employee from the contracting (construction) company,

District, or other NYSDEC endorsed entity. After receiving the initial training, the trained in proper erosion and sediment control principles from a Soil and Water Conservation contractor shall receive four (4) hours of training every three (3) years.

It can also mean an employee from the contracting (construction) company, identified in Engineer, Certified Professional in Erosion and Sediment Control (CPESC), Registered Part III.A.7., that meets the qualified inspector qualifications (e.g. licensed Professional erosion and sediment control principles from a Soil and Water Conservation District, or Program holder, or someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided they have received four (4) hours of NYSDEC endorsed training in proper Landscape Architect, New York State Erosion and Sediment Control Certificate other NYSDEC endorsed entity).

The trained contractor is responsible for the day-to-day implementation of the SWPPP.

Tree Clearing – construction activities limited to felling and removal of trees.

support from mechanized equipment, which is not considered construction activity Tree clearing does not include hand felling and leaving the trees in place with no requiring coverage under this permit. Water Quality Standard – such measures of purity or quality for any waters in relation to their reasonable and necessary use as promulgated in 6 NYCRR Part 700 et seq.

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Appendix B

# APPENDIX B - Required SWPPP Components by Project Type

## CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP THAT ONLY INCLUDES EROSION AND SEDIMENT CONTROLS

The following *construction activities* that involve soil disturbances of one (1) or more acres of

land, but less than five (5) acres:

- Single-family home <u>not</u> located in one of the watersheds listed in Appendix C and <u>not</u> directly discharging to one of the 303(d) segments listed in Appendix D
   Single-family residential subdivisions with 25% or less *impervious cover* at total site build-out and <u>not</u> located in one of the watersheds listed in Appendix C and <u>not</u> directly discharging to one of the <u>303</u>(d) segments listed in Appendix D
  - Construction of a barn or other agricultural building, silo, stock yard or pen.
- reconstruction of *impervious area* o<u>r</u> alter hydrology from pre- to post-development conditions. Structural agricultural conservation practices as identified in Table II in the "Agricultural Best Management Practice Systems Catalogue" (dated June 2023) that include construction or

The following construction activities that involve soil disturbances between five thousand (5000) square feet and one (1) acre of land: All construction activities located in the New York City Watershed located east of the Hudson River, see Appendix C Figure 1, that involve soil disturbances between five thousand (5,000) square feet and one (1) acre of land.

## Within the municipal boundaries of NYC:

Stand-alone road reconstruction, where the total soil disturbance from only that road construction, is less than one (1) acre of land.

## The following construction activities:

- Š as gas lines, fiber-optic cable, cable utilities; such electric, telephone, sewer mains, and water mains Installation of underground linear
  - Environmental enhancement projects, such as wetland mitigation, stormwater retrofits, stream
    - restoration, and resiliency projects that reconstruct shoreline areas to address sea level rise
- · Linear bike paths running through areas with vegetative cover, including bike paths surfaced with an impervious cover
  - parking lot (maximum 10 spaces total, sized for passenger cars) with 35 feet minimum preservation of undisturbed area downgradient from the parking lot · Cross-country ski trails, walking/hiking trails, and mountain biking trails, including a de minimis

    - Dam rehabilitation (the structure of the dam itself)
       Sidewalks, bike paths, or walking paths, surfaced with an impervious cover, that are not part of
- residential, commercial, or institutional development;
  Sidewalks, bike paths, or walking paths, surfaced with an *impervious cover*, that include incidental shoulder or curb work along an existing highway to support construction of the sidewalk, bike path.

Appendix B

## CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP THAT ONLY INCLUDES EROSION AND SEDIMENT CONTROLS Table 1 (Continued)

## The following construction activities:

- Slope flattening that changes the grade of the site, but does not significantly change the runoff
- Spoil areas that will be covered with vegetation
- · Vegetated open space (i.e. recreational parks, lawns, meadows, fields, downhill ski trails) that do
  - · Athletic fields (natural grass) that do not include the construction or reconstruction of impervious not after hydrology from pre- to post-development conditions
    - Demolition where vegetation will be established, and no redevelopment activity is planned<sup>1</sup> area and do not alter hydrology from pre- to post-development conditions
- Installation or replacement of either an overhead electric transmission line or a ski lift tower that does not include the construction of permanent access roads or parking areas surfaced with impervious cover
- not alter hydrology from pre- to post-development conditions, and address water quality volume and Solar array field areas that have tables elevated off the ground, spaced one table width apart, do runoff reduction volume by maintaining sheet flow on slopes less than 8%.
  - Management Practice Systems Catalogue" (dated June 2023) that do not include construction or Structural agricultural conservation practices as identified in Table II in the "Agricultural Best reconstruction of impervious area and do not alter hydrology from pre- to post-development
- areas that will be restored to pre-construction conditions once the construction activity is complete Temporary access roads, median crossovers, detour roads, lanes, or other temporary impervious (in this context, "temporary" means the impervious area will be in place for two years or less)
  - Other construction activities that do not include the construction or reconstruction of impervious
  - area, and do not alter hydrology from pre- to post-development conditions, and are not listed in Table 2.

1. If the site is redeveloped in the future, a new eNOI must be submitted.

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Appendix B

# CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP THAT INCLUDES POST-CONSTRUCTION STORMWATER MANAGEMENT PRACTICES (SMPS)

## The following construction activities:

- Single-family home located in one of the watersheds listed in Appendix C or directly discharging to
  - one of the 303(d) segments listed in Appendix D Single-family home that disturbs five (5) or more acres of land
- Single-family residential subdivisions located in one of the watersheds listed in Appendix C or directly discharging to one of the 303(d) segments listed in Appendix D directly discharging to one of the 303(d) segments listed in Appendix D Single-family residential subdivisions that involve soil disturbances of between one (1) and five (5) acres of land with greater than 25% impervious cover at total site build-out Single-family residential subdivisions that involve soil disturbances of between 20,000 square feet
  - and one (1) acre of land within the municipal boundaries of NYC with greater than 25% *impervious*
- cover at iotal site build-out Single-family residential subdivisions that involve soil disturbances of five (5) or more acres of land, and single-family residential subdivisions that involve soil disturbances of less than five (5) acres that are part of a common plan of development or sale that will ultimately disturb five (5) or more acres of land
  - Multi-family residential developments; includes duplexes, townhomes, condominiums, senior housing complexes, apartment complexes, and mobile home parks Creation of 5,000 square feet or more of *impervious area* in the municipal boundaries of NYC
- Airports
- Amusement parks .

Campgrounds

- Breweries, cideries, and wineries, including establishments constructed on agricultural land
- Cemeteries that include the construction or reconstruction of impervious area (>5% of disturbed area) or alter the hydrology from pre- to post-development conditions
  - Commercial developments
- Churches and other places of worship
- Construction of a barn or other agricultural building (e.g. silo) that involves soil disturbance greater than five acres.
- Structural agricultural conservation practices as identified in Table II in the "Agricultural Best Management Practice Systems Catalogue" (dated June 2023) that involves soil disturbance greater than five acres and include the construction or reconstruction of *impervious area* or *after hydrology* from pre- to post-development conditions.
  - Facility buildings, including ski lodges, restroom buildings, pumphouses, ski lift terminals, and
    - maintenance and groomer garages
- Institutional development; includes hospitals, prisons, schools and colleges Industrial facilities; includes industrial parks
- Landfills; including creation of landfills or capping landfills. Municipal facilities; includes highway garages, transfer stations, office buildings, POTWs, water treatment plants, and water storage tanks
- Office complexes

Appendix B

### Table 2 (Continued)

# CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP THAT INCLUDES POST-CONSTRUCTION STORMWATER MANAGEMENT PRACTICES (SMPS)

## The following construction activities:

- Permanent laydown yards and equipment storage lots
- Playgrounds that include the construction or reconstruction of impervious area
  - Sports complexes
- Racetracks; includes racetracks with earthen (dirt) surfaces
- Road construction or reconstruction, outside the municipal boundaries of NYC Road construction within the municipal boundaries of NYC
- Stand-alone road reconstruction, within the municipal boundaries of NYC where the total soil disturbance from that road reconstruction involves soil disturbance of one (1) acre or more of land Parking lot construction or reconstruction (as with all Table 2 bullets, this includes parking lots constructed as part of the construction activities listed in Table 1, unless a Table 1 bullet specifies
- Athletic fields (natural grass) that include the construction or reconstruction of impervious area (>5% of disturbed area) or alter the hydrology from pre- to post-development conditions
  - Athletic fields with artificial turf
- surfaced with impervious cover, and constructed as part of an overhead electric transmission line, wind-power, cell tower, oil or gas well drilling, sewer or water main, ski lift, or other linear utility Permanent access roads, parking areas, substations, compressor stations, and well drilling pads.
- Sidewalks, bike paths, or walking paths, surfaced with an impervious cover, that are part of a residential, commercial or institutional development
  - Sidewalks, bike paths, or walking paths, surfaced with an *impervious cover*, that are part of highway construction or reconstruction
- Solar array field areas on slopes greater than 8% that cannot maintain sheet flow using management practices identified in the BB or the DM
- Solar array field areas on slopes less than 8% that will alter the hydrology from pre- to post-
- Solar array field areas with tables that are not elevated high enough to achieve final stabilization development conditions
  - Traditional impervious areas associated with solar development (e.g. roads, buildings, beneath the tables
- Utility pads surfaced with *impervious cover*, including electric vehicle charging stations All other *construction activities* that include the construction or reconstruction of *impervious area* 
  - or alter the hydrology from pre- to post-development conditions, and are not listed in Table 1

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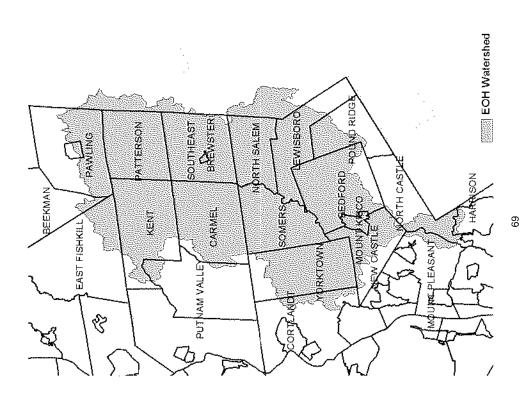
Appendix C

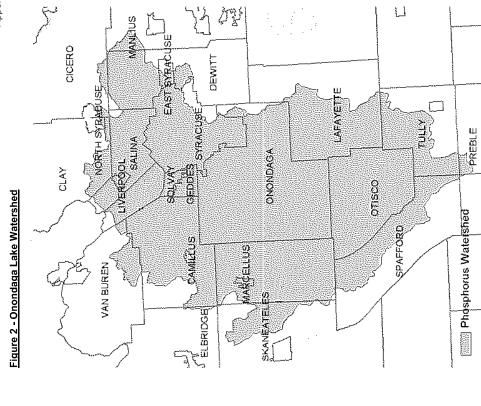
# APPENDIX C – Watersheds Requiring Enhanced Phosphorus Removal

conformance with the Enhanced Phosphorus Removal Standards included in the Table 2 of Appendix B must prepare a SWPPP that includes SMPs designed in Watersheds where owners or operators of construction activities identified in DM technical standard

- Entire New York City Watershed located east of the Hudson River Figure 1
  - Onondaga Lake Watershed Figure 2
- Greenwood Lake Watershed Figure 3
  Oscawana Lake Watershed Figure 4
  Kinderhook Lake Watershed Figure 5

Figure 1 - New York City Watershed East of the Hudson





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Appendix C

Figure 3 - Greenwood Lake Watershed

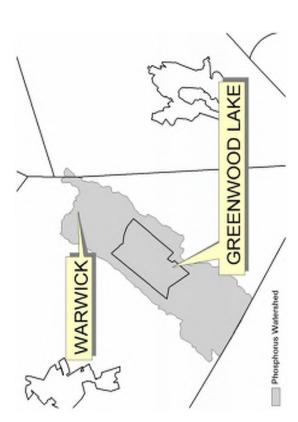
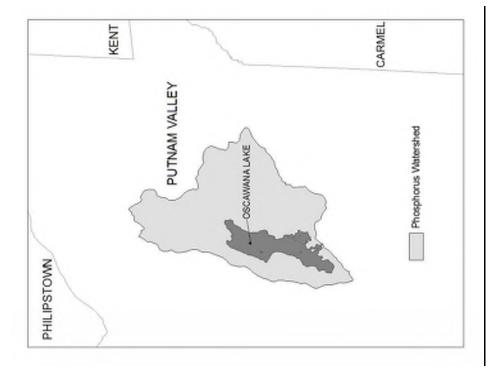
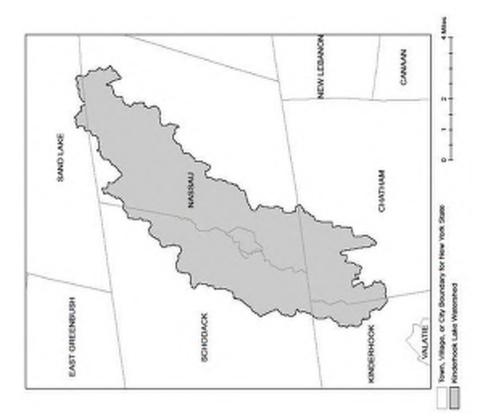


Figure 4 - Oscawana Lake Watershed



Appendix C

Figure 5 - Kinderhook Lake Watershed



# APPENDIX D - Impaired Waterbodies (by Construction Related Pollutants)

List of waterbodies impaired by pollutants related to construction activity, including turbidity, silt/sediment, and nutrients (e.g. nitrogen, phosphorus). This list is a subset of "The Final New York State 2018 Section 303(d) List of Impaired Waters Requiring a TMDL" dated June 2020.

Compty	Matatak	Dollutant
	Vaccious Vac	וסומנמונ
Albany	Ann Lee (Shakers) Pond, Stump Pond (1201-0096)	Phosphorus
Albany	Lawsons Lake (1301-0235)	Phosphorus
Allegany	Amity Lake, Saunders Pond (0403-0054)	Phosphorus
Allegany	Andover Pond (0403-0056)	Phosphorus
Bronx	Reservoir No.1/Lake Isle (1702-0075)	Phosphorus
Bronx	Van Cortlandt Lake (1702-0008)	Phosphorus
Broome	Blueberry, Laurel Lakes (1404-0033)	Phosphorus
Broome	Fly Pond, Deer Lake (1404-0038)	Phosphorus
Broome	Minor Tribs to Lower Susquehanna (0603-0044)	Phosphorus
Broome	Whitney Point Lake/Reservoir (0602-0004)	Phosphorus
Cattaraugus	Allegheny River/Reservoir (0201-0023)	Phosphorus
Cattaraugus	Beaver Lake/Alma Pond (0201-0073)	Phosphorus
Cattaraugus	Case Lake (0201-0020)	Phosphorus
Cattaraugus	Linlyco/Club Pond (0201-0035)	Phosphorus
Cayuga	Duck Lake (0704-0025)	Phosphorus
Cayuga	Owasco Inlet, Upper, and tribs (0706-0014)	Nutrients
Chautauqua	Chadakoin River and tribs (0202-0018)	Phosphorus
Chautauqua	Hulburt/Clymer Pond (0202-0079)	Phosphorus
Chautauqua	Middle Cassadaga Lake (0202-0002)	Phosphorus
Clinton	Great Chazy River, Lower, Main Stem (1002-0001)	Silt/Sediment
Columbia	Robinson Pond (1308-0003)	Phosphorus
Cortland	Dean Pond (0602-0077)	Phosphorus
Dutchess	Fallkill Creek (1301-0087)	Phosphorus
Dutchess	Hillside Lake (1304-0001)	Phosphorus
Dutchess	Wappingers Lake (1305-0001)	Phosphorus
Dutchess	Wappingers Lake (1305-0001)	Silt/Sediment
Erie	Beeman Creek and tribs (0102-0030)	Phosphorus
Erie	Delaware Park Pond (0101-0026)	Phosphorus
Erie	Ellicott Creek, Lower, and tribs (0102-0018)	Phosphorus
Erie	Ellicott Creek, Lower, and tribs (0102-0018)	Silt/Sediment
Erie	Green Lake (0101-0038)	Phosphorus
Erie	Little Sister Creek, Lower, and tribs (0104-0045)	Phosphorus
Erie	Murder Creek, Lower, and tribs (0102-0031)	Phosphorus

	Erie	Rush Creek and tribs (0104-0018)	Phosphorus
	Erie	Scajaquada Creek, Lower, and tribs (0101-0023)	Phosphorus
	Erie	Scajaquada Creek, Middle, and tribs (0101-0033)	Phosphorus
	Erie	Scajaquada Creek, Upper, and tribs (0101-0034)	Phosphorus
	Erie	South Branch Smoke Cr, Lower, and tribs (0101-0036)	Phosphorus
	Erie	South Branch Smoke Cr, Lower, and tribs (0101-0036)	Silt/Sediment
	Genesee	Bigelow Creek and tribs (0402-0016)	Phosphorus
	Genesee	Black Creek, Middle, and minor tribs (0402 0028)	Phosphorus
	Genesee	Black Creek, Upper, and minor tribs (0402-0048)	Phosphorus
	Genesee	Bowen Brook and tribs (0102-0036)	Phosphorus
	Genesee	LeRoy Reservoir (0402-0003)	Phosphorus
	Genesee	Mill Pond (0402-0050)	Phosphorus
0.0 July 1 July	Genesee	Oak Orchard Cr, Upper, and tribs (0301-0014)	Phosphorus
20 July 1	Genesee	Oatka Creek, Middle, and minor tribs (0402-0031)	Phosphorus
1	Genessee	Tonawanda Cr, Middle, Main Stem (0102-0002)	Phosphorus
	Greene	Schoharie Reservoir (1202-0012)	Silt/Sediment
	Greene	Sleepy Hollow Lake (1301-0059)	Silt/Sediment
יר איז איז איז איז איז איז איז איז איז איז	Herkimer	Steele Creek tribs (1201-0197)	Phosphorus
	Herkimer	Steele Creek tribs (1201-0197)	Silt/Sediment
1	Kings	Hendrix Creek (1701-0006) 18	Nitrogen
100 M	Kings	Prospect Park Lake (1701-0196)	Phosphorus
	Lewis	Mill Creek/South Branch, and tribs (0801-0200)	Nutrients
no no	Livingston	Christie Creek and tribs (0402-0060)	Phosphorus
	Livingston	Conesus Lake (0402-0004)	Phosphorus
	Livingston	Mill Creek and minor tribs (0404-0011)	Silt/Sediment
	Monroe	Black Creek, Lower, and minor tribs (0402-0033)	Phosphorus
	Monroe	Buck Pond (0301-0017)	Phosphorus
	Monroe	Cranberry Pond (0301-0016)	Phosphorus
	Monroe	Durand, Eastman Lakes (0302-0037)	Phosphorus
	Monroe	Lake Ontario Shoreline, Western (0301-0069) 9	Phosphorus
	Monroe	Long Pond (0301-0015)	Phosphorus
	Monroe	Mill Creek and tribs (0302-0025)	Phosphorus 2
	Monroe	Mill Creek/Blue Pond Outlet and tribs (0402-0049)	Phosphorus
	Monroe	Minor Tribs to Irondequoit Bay (0302-0038)	Phosphorus
	Monroe	Rochester Embayment - East (0302-0002) [9]	Phosphorus
	Monroe	Rochester Embayment - West (0301-0068) 9	Phosphorus
	Monroe	Shipbuilders Creek and tribs (0302-0026)	Phosphorus 2
	Monroe	Thomas Creek/White Brook and tribs (0302-0023)	Phosphorus

Nassau	Bannister Creek/Bay (1701-0380)	Nitrogen
Nassau	Beaver Lake (1702-0152)	Phosphorus
Nassau	Browswere Bay (1701-0383)	Nitrogen
Nassau	Camaans Pond (1701-0052)	Phosphorus
Nassau	East Meadow Brook, Upper, and tribs (1701-0211)	Silt/Sediment
Nassau	East Rockaway Channel (1701-0381)	Nitrogen
Nassau	Glen Cove Creek, Lower, and tribs (1702-0146)	Silt/Sediment
Nassau	Grant Park Pond (1701-0054)	Phosphorus
Nassau	Hempstead Bay, Broad Channel (1701-0032)	Nitrogen
Nassau	Hempstead Lake (1701-0015)	Phosphorus
Nassau	Hewlett Bay (1701-0382)	Nitrogen
Nassau	Hog Island Channel (1701-0220)	Nitrogen
Nassau	Massapequa Creek, Upper, and tribs (1701-0174)	Phosphorus
Nassau	Milburn/Parsonage Creeks, Upp, and tribs (1701-0212)	Phosphorus
Nassau	Reynolds Channel, East (1701-0215) [12]	Nitrogen
Nassau	Reynolds Channel, West (1701-0216) 12	Nitrogen
Nassau	Tidal Tribs to Hempstead Bay (1701-0218)	Nitrogen
Nassau	Tribs (fresh) to East Bay (1701-0204)	Silt/Sediment
Nassau	Tribs (fresh) to East Bay (1701-0204)	Phosphorus
Nassau	Tribs to Smith Pond/Halls Pond (1701-0221)	Phosphorus
Nassau	Woodmere Channel (1701-0219)	Nitrogen
New York	Harlem Meer (1702-0103)	Phosphorus
New York	The Lake in Central Park (1702-0105)	Phosphorus
Niagara	Bergholtz Creek and tribs (0101-0004)	Phosphorus
Niagara	Hyde Park Lake (0101-0030)	Phosphorus
Niagara	Lake Ontario Shoreline, Western (0301-0053) 9	Phosphorus
Niagara	Lake Ontario Shoreline, Western (0301-0072) 9	Phosphorus
Oneida	Ballou, Nail Creeks (1201-0203)	Phosphorus
Onondaga	Lev Creek and tribs (0702-0001) 10	Nutrients
0		(phosphorus)
Onondaga	Minor Tribs to Onondaga Lake (0702-0022) 10	Nutrients (phosphorus)
Onondaga	Minor Tribs to Onondaga Lake (0702-0022) 10	Nitrogen (NH3, NO2)
Onondaga	Onondaga Creek, Lower (0702-0023) 10	Nutrients (phosphorus)
Onondaga	Onondaga Creek, Lower, and tribs (0702-0023)	Turbidity
Onondaga	Onondaga Creek, Middle, and tribs (0702-0004)	Turbidity
Onondaga	Onondaga Creek, Upper, and tribs (0702-0024)	Turbidity
Ontario	Great Brook and minor tribs (0704-0034)	Phosphorus 2
Ontario	Great Brook and minor tribs (0704-0034)	Silt/Sediment

Hemlock Lake Outlet and minor tribs (0402-0013)	Phosphorus
Honeoye Lake (0402-0032)	Phosphorus
Brown Pond Reservoir (1303-0013)	Phosphorus
	Phosphorus
Minor Tribs to Middle Wallkill (1306-0061)	Phosphorus
306-0074)	Phosphorus
Orange Lake (1301-0008) [16]	Phosphorus
Quaker Creek and tribs (1306-0025)	Phosphorus
Wallkill River, Middle, Main Stem (1306-0038)	Phosphorus
Wallkill River, Upper, and Minor tribs (1306-0017)	Phosphorus
Glenvwood Lake (0301-0041)	Phosphorus
Lake Ontario Shoreline, Western (0301-0070) 9	Phosphorus
Lake Ontario Shoreline, Western (0301-0071) 9	Phosphorus
Lake Neatahwanta (0701-0018) (	Nutrients (phosphorus)
Pleasant Lake (0703-0047)	Phosphorus
Lost Lake, Putnam Lake (1302-0053)	Phosphorus
Minor Tribs to Croton Falls Reservoir (1302-0001)	Phosphorus
Bergen Basin (1701-0009) 18	Nitrogen
Jamaica Bay, Eastern, and tribs, Queens (1701- 0005) 18	Nitrogen
Kissena Lake (1702-0258)	Phosphorus
Meadow Lake (1702-0030)	Phosphorus
Shellbank Basin (1701-0001) 18	Nitrogen
Willow Lake (1702-0031)	Phosphorus
Nassau Lake (1310-0001)	Phosphorus
Snyders Lake (1301-0043)	Phosphorus
Grassmere Lake/Bradys Pond (1701-0357)	Phosphorus
Congers Lake, Swartout Lake (1501-0019)	Phosphorus
Rockland Lake (1501-0021)	Phosphorus
Ballston Lake (1101-0036)	Phosphorus
Dwaas Kill and tribs (1101-0007)	Phosphorus
Dwaas Kill and tribs (1101-0007)	Silt/Sediment
Lake Lonely (1101-0034)	Phosphorus
Round Lake (1101-0060)	Phosphorus
1-0001)	Phosphorus
	Phosphorus
Duane Lake (1311-0006)	Phosphorus
13)	Phosphorus
Cayuta Lake (0603-0005)	Phosphorus

Seneca	Reeder Creek and tribs (0705-0074)	Phosphorus
St.Lawrence	Black Lake Outlet, Black Lake (0906-0001)	Phosphorus
St.Lawrence	Fish Creek and minor tribs (0906-0026)	Phosphorus
Steuben	Smith Pond (0502-0012)	Phosphorus
Suffolk	Agawam Lake (1701-0117)	Phosphorus
Suffolk	Big/Little Fresh Ponds (1701-0125)	Phosphorus
Suffolk	Canaan Lake (1701-0018)	Phosphorus
Suffolk	Canaan Lake (1701-0018)	Silt/Sediment
Suffolk	Fresh Pond (1701-0241)	Phosphorus
Suffolk	Great South Bay, East (1701-0039)	Nitrogen
Suffolk	Great South Bay, Middle (1701-0040)	Nitrogen
Suffolk	Great South Bay, West (1701-0173)	Nitrogen
Suffolk	Lake Ronkoma (1701-0020)	Phosphorus
Suffolk	Mattituck/Marratooka Pond (1701-0129)	Phosphorus
Suffolk	Mill and Seven Ponds (1701-0113)	Phosphorus
Suffolk	Millers Pond (1702-0013)	Phosphorus
Suffolk	Moriches Bay, East (1701-0305)	Nitrogen
Suffolk	Moriches Bay, West (1701-0038)	Nitrogen
Suffolk	Quantuck Bay (1701-0042)	Nitrogen
Suffolk	Shinnecock Bay and Inlet (1701-0033)	Nitrogen
Suffolk	Tidal Tribs to West Moriches Bay (1701-0312)	Nitrogen
Sullivan	Bodine, Mongomery Lakes (1401-0091)	Phosphorus
Sullivan	Davies Lake (1402-0047)	Phosphorus
Sullivan	Evens Lake (1402-0004)	Phosphorus
Sullivan	Pleasure Lake (1402-0055)	Phosphorus
Sullivan	Swan Lake (1401-0063)	Phosphorus
Tompkins	Cayuga Lake, Southern End (0705-0040)	Phosphorus
Tompkins	Cayuga Lake, Southern End (0705-0040)	Silt/Sediment
Ulster	Ashokan Reservoir (1307-0004)	Silt/Sediment
Ulster	Esopus Creek, Lower, Main Stem (1307-0010) [17]	Turbidity
Ulster	Esopus Creek, Middle, Main Stem (1307-0003) 17	Turbidity
Ulster	Esopus Creek, Upper, and minor tribs (1307-0007)[3]	Silt/Sediment
Ulster	Wallkill River, Lower, Main Stem (1306-0027)	Phosphorus
Warren	Hague Brook and tribs (1006-0006)	Silt/Sediment
Warren	Huddle/Finkle Brooks and tribs (1006-0003)	Silt/Sediment
Warren	Indian Brook and tribs (1006-0002)	Silt/Sediment
Warren	Lake George (1006-0016) and tribs	Silt/Sediment
Warren	Tribs to Lake George, East Shore (1006-0020)	Silt/Sediment
Warren	Tribs to Lake George, Lk.George Village (1006-0008)	Silt/Sediment

Washington	Wood Cr/Champlain Canal and tribs (1005-0036)	Phosphorus
Westchester	Lake Katonah (1302-0136)	Phosphorus
Westchester	Lake LincoIndale (1302-0089)	Phosphorus
Westchester	Lake Meahagh (1301-0053)	Phosphorus
Westchester	Lake Mohegan (1301-0149)	Phosphorus
Westchester	Lake Shenorock (1302-0083)	Phosphorus
Westchester	Mamaroneck River, Lower (1702-0071)	Silt/Sediment
Westchester	Mamaroneck River, Upp, & minor tribs (1702-0123)	Silt/Sediment
Westchester	Saw Mill River (1301-0007)	Phosphorus
Westchester	Saw Mill River, Middle, and tribs (1301-0100)	Phosphorus
Westchester	Sheldrake River (1702-0069)	Phosphorus
Westchester	Sheldrake River (1702-0069)	Silt/Sedimnt
Westchester	Silver Lake (1702-0040)	Phosphorus
Westchester	Teatown Lake (1302-0150)	Phosphorus
Westchester	Truesdale Lake (1302-0054)	Phosphorus
Westchester	Wallace Pond (1301-0140)	Phosphorus

# APPENDIX E - List of NYSDEC Regional Offices

Region	COVERING THE FOLLOWING COUNTIES:	DIVISION OF ENVIRONMENTAL PERMITS (DEP) PERMIT ADMINISTRATORS	DIVISION OF WATER (DOW) WATER (SPDES) PROGRAM
1	NASSAU AND SUFFOLK	50 CIRCLE ROAD STONY BROOK, NY 11790 TEL. (631) 444-0365	50 CIRCLE ROAD STONY BROOK, NY 11790-3409 TEL. (631) 444-0405
2	BRONX, KINGS, NEW YORK, QUEENS AND RICHMOND	1 Hunters Point Plaza, 47-40 21st St. Long Bland City, NY 11101-5407 Tel. (718) 482-4997	1 Hunters Point Plaza, 47-40 Zist St. Long Island City, NY 11101-5407 Tel. (718) 482-4933
က	DUTCHESS, ORANGE, PUTNAM, ROCKLAND, SULLIVAN, ULSTER AND WESTCHESTER	21 SOUTH PUIT CORNERS ROAD NEW PALTZ, NY 12561-1696 TEL. (845) 256-3059	220 WHITE PLAINS ROAD, SUITE 110 TEL. (914) 428 - 2505
4	ALBANY, COLUMBIA, DELAWARE, GREENE, MONTGOMERY, OTSEGO, RENSSELAER, SCHENECTADY AND SCHOHARIE	1130 NORTH WESTCOTT ROAD SCHENECTADY, NY 12306-2014 TEL (518) 357-2069	1130 NORTH WESTCOTT ROAD SCHENECTADY, NY 12306-2014 TEL. (518) 357-2045
2	CLINTON, ESSEX, FRANKLIN, FULTON, HAMILTON, SARATOGA, WARREN AND WASHINGTON	1115 STATE ROUTE 86, PO BOX 296 RAY BROOK, NY 12977-0296 TEL. (518) 897-1234	232 GOLF COURSE ROAD WARRENSBURG, NY 12885-1172 TEL. (518) 623-1200
9	HERKIMER, JEFFERSON, LEWIS, ONEIDA AND ST. LAWRENCE	STATE OFFICE BUILDING 317 WASHINGTON STREET WATERTOWN, NY 13601-3787 TEL. (315) 785-2245	STATE OFFICE BUILDING 207 GENESEE STREET UTICA, NY 13501-2885 TEL. (315) 793-2554
7	BROOME, CAYUGA, CHENANGO, CORTLAND, MADISON, ONONDAGA, OSWEGO, TIOGA AND TOMPKINS	5786 WIDEWATERS PARKWAY SYRACUSE, NY 13214-1867 TEL. (315) 426-7438	5786 WIDEWATERS PARKWAY SYRACUSE, NY 13214-1867 TEL. (315) 426-7500
<sub>∞</sub>	CHEMUNG, GENESEE, LIVINGSTON, MONROE, NOTARIO, ORLEANS, SCHIVLER, SENECA, STEUBEN, WAYNE AND YATES	6274 EAST AVON-LIMA ROADAVON, NY 14414-9519 TEL. (585) 226-2466	6274 EAST AVON-LIMA RD. AVON, NY 14414-9519 TEL. (583) 226-2466
6	ALLEGANY, CATTARAUGUS, CHAUTAUQUA, ERE, NIAGARA AND WYOMING	700 DELAWARE AVENUE BUFFALO, NY 14209-2999 TEL. (716) 851-7165	700 DELAWARE AVENUE BUFFALO, NY 14209-2999 TEL. (716) 851-7070

# APPENDIX F - SWPPP Preparer Certification Form

The SWPPP Preparer Certification Form required by this permit begins on the following page.

YORK STATE Environmental Conservation

# **SWPPP Preparer Certification Form**

# SPDES General Permit for Stormwater Discharges from Construction Activity, GP-0-25-001 (CGP)

(In accordance with CGP Part I.D.2.b., the completed form must be attached to the eNOI and submitted to NYSDEC electronically.)

Project/Site Name:		
eNOI Submission ID:		
Owner/Operator Name:		
Certification Statement - SWPPP Preparer	/PPP Pre	parer
I hereby certify that the Stormwater P prepared in accordance with the requof law that the SWPPP and all attact supervision in accordance with a sysproperly gather and evaluate the infruency or persons who manage the significant of the information, the informand belief, true, accurate, and compliand belief.	ter Polluti requirement tachment system d information the syster ormation t implete. I	I hereby certify that the Stormwater Pollution Prevention Plan (SWPPP) has been prepared in accordance with the requirements of GP-0-25-001. I certify under penalty of law that the SWPPP and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.
SWPPP Preparer First Name	₹	SWPPP Preparer Last Name

81

Date

Signature

Revised: January 2025

# APPENDIX G - MS4 SWPPP Acceptance Form

The MS4 SWPPP Acceptance Form required by this permit begins on the following



# **MS4 SWPPP Acceptance Form**

for construction activities seeking authorization under the

# SPDES General Permit for Stormwater Discharges from Construction Activity, GP-0-25-001 (CGP)

(in accordance with CGP Part I.D.2.b., the completed form must be attached to the eNOI and submitted to NYSDEC electronically.)

Owner/Operator Information
Project (

- 1. Owner/Operator Name:
- 2. Contact Person:
- 3. Street Address:

### City/State/Zip:

## II. Project Site Information

5. Project/Site Name:

### 6. Street Address:

### 7. City/State/Zip:

# III. Stormwater Pollution Prevention Plan (SWPPP) Review and Acceptance Information

### 8, SWPPP Reviewed by:

## 9. Title/Position:

## 10. Date Final SWPPP Reviewed and Accepted:

IV. Regulated MS4 Information

## 11, Name of MS4 Operator:

12. MS4 SPDES Permit Identification Number: NYR20A

### 13. Street Address:

14. City/State/Zip:

### 15. Telephone Number:

Page 1 of 2

# NS4 SWPPP Acceptance Form - continued V. Certification Statement - MS4 Official (principal executive officer or ranking elected official) or buily Authorized Representative In the bety certify that the final Stomwader Pollution Prevention Plan (SWPPP) for the construction project identified in section II. of this form has been reviewed and meets the substantive requirements in the SPDES General Permit for Stomwader Discharges from Construction Activity, GP-0-25-001 (CGP). Where: The MS4 Operator, through the acceptance of the SWPPP. In addition, review and acceptance of the SWPPP by the MS4 Operator of fire SWPPP by the MS4 Operator does not relieve the owner/operator or their SWPPP preparer of responsibility or lability for errors or omissions in the plan. Titler/Position: Signature: Date: VI. Additional Information

(NYSDEC - MS4 SWPPP Acceptance Form - January 2025)

Page 2 of 2

# APPENDIX H - NYCDEP SWPPP Acceptance/Approval Form

The City of New York Department of Environmental Protection (NYCDEP) SWPPP Acceptance/Approval form required by this permit begins on the following page.

Printed name of the principal executive officer or ranking elected official for the MS4 Operator or their duly authorized representative in accordance with CGP Part VII.J.2.



## DEPARTMENT OF ENVIRONMENTAL PROTECTION Bureau of Environmental Planning and Analysis 59-17 Junction Bivd., 9th Floor; Flushing, NY 11373 THE CITY OF NEW YORK

## SWPPP Acceptance/Approval

Application Number:

I. Project Owner/Operator Information

1. Owner/Operator Name:
2. Contact Person:
3. Street Address:
4. City/State/Zip:
II. Project Site Information
5. Project/Site Name:
6. Street Address:
7. City/State/Zip:
III. Stormwater Pollution Prevention Plan (SWPPP) Review and Acceptance/Approval
8. SWPPP Reviewed by:
9. Title/Position: /
10. Date Final SWPPP Reviewed and Accepted:
11. Acceptance/Approval Expiration Date:
IV. Regulated MS4 Information for projects that require coverage under the NY State Pollution Discharge Elimination System General Permit for Stormwater Discharges from Construction Activity
12. Name of MS4: CITY OF NEW YORK
13. MS4 SPDES Permit Identification Number: NY-0287890
14. Contact Person:
15. Street Address: 59-17 Junction Blvd. 9th Floor
16. City/State/Zip: Flushing, NY 11373
17. Telephone Number:



Projects in the MS4 area must submit a copy of this SWPPP Acceptance with a Notice of Intent for coverage under the NY SPDES General Permit for Stormwater Discharges from Construction Activity to: NYS Department of Environmental Conservation, Division of Water; 625 Broadway, 4th Floor; Albany, New York 12233-3505.

Department of Environmental Conservation

Page 1 Of 2



# THE CITY OF NEW YORK DEPARTMENT OF ENVIRONMENTAL PROTECTION Bureau of Environmental Planning and Analysis 59-17 Junction Bivd., 9th Floor; Flushing, NY 11373

# V. Certification Statement - MS4 Official (principal executive officer or ranking elected official) or **Duly Authorized Representative**

I hereby certify that the final Stormwater Pollution Prevention Plan (SWPPP) for the construction project identified in question 5 has been reviewed and meets the substantive requirements in the SPDES General Permit For Stormwater Discharges from Municipal Separate Storm Sewer Systems (MS4s) Note: The MS4, through the acceptance of the SWPPP, assumes no responsibility for the accuracy and adequacy of relieve the owner/operator or their SWPPP preparer of responsibility or liability for errors or omissions in the plan. the design included in the SWPPP. In addition, review and acceptance of the SWPPP by the MS4 does not

Printed Name:

Title/Position:

Signature:

Date:

VI. Conditions of Acceptance/Approval and Additional Information



Projects in the MS4 area must submit a copy of this SWPPP Acceptance with a Notice of Intent for coverage under the NY SPDES General Permit for Stormwater Discharges from Construction Activity to: NYS Department of Environmental Conservation, Division of Water; 625 Broadway, 4th Floor; Albany, New York 12233-3505.

Page 2 Of 2

# APPENDIX I - MS4 No Jurisdiction Form

The MS4 No Jurisdiction Form required by this permit begins on the following page.

NEW Department of YORK Environmental Conservation

## MS4 No Jurisdiction Form

for construction activities seeking authorization under the

# SPDES General Permit for Stormwater Discharges from Construction Activity, GP-0-25-001 (CGP)

(In accordance with CGP Part I.D.2.b., the completed form must be attached to the eNOI and submitted to NYSDEC electronically.)

## I. Project Owner/Operator Information

- a. Owner/Operator Name:
- b. Contact Person:
- c. Street Address:
- d. City/State/Zip:

### Project Site Information

≓

- a. Project/Site Name:
- b. Street Address:
- c. City/State/Zip:
- d. eNOI Submission ID:

# Traditional Land Use Control MS4 Operator Information

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- a. Name of MS4 Operator:
- b. MS4 SPDES Permit ID Number: NYR20A
- c. Street Address:
- d. City/State/Zip:
- e. Telephone Number:

## IV. Certification Statement

In accordance with CGP Part I.D.2 bii 3, I hereby certify that the Traditional Land Use Control MS4 Operator identified in section III. of this form does not have review authority over the construction project identified in section II. of this form, which is owned/operated by the entity identified in section I. of this form, I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

- Printed name of the principal executive officer or ranking elected official for the MS4 Operator or their duly authorized representative in accordance with CGP Part VII.J.2.:
- b. Title/Position:
- c. Signature:
- d. Date:

# APPENDIX J - Owner/Operator Certification Form

The Owner/Operator Certification Form required by this permit begins on the following page.



# Owner/Operator Certification Form

# SPDES General Permit for Stormwater Discharges from Construction Activity, GP-0-25-001 (CGP)

(In accordance with CGP Part I.D.2.b. or Part I.F.2. and 3., the completed form must be attached to the eNOI or the Request to Continue Coverage, and submitted to NYSDEC electronically.

Project/Site Name:			
eNOI Submission ID:			Ī
eNOI Submitted by: Owner/Operator	ator	SWPPP Preparer	Other
Certification Statement - Owner/Operator	rator		
I hereby certify that I read, and will comply with, the GP-0-25-001 permit requirements. I understand that authorization to discharge under the permit for the project/site named above is dependent on receipt of a Letter of Authorization (LOA) or a Letter of Continued Coverage (LOCC) from the New York State Department of Environmental Conservation (NYSDEC) in accordance with CGP Part I.D.3.b. or Part I.F.4. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.	, the GP-0-; the project ntinued Cov YSDEC) in for submittii	25-001 permit requirements. Lusite named above is dependen erage (LOCC) from the New Yaccordance with CGP Part I.D. og false information, including t	nderstand that ton receipt of a ork State 3.b. or Part I.F.4. ne possibility of
	3		
Owner/Operator First Name	<b>=</b>	Owner/Operator Last Name	
Signature			
Date			
Revised: January 2025			



### APPENDIX J: NYSDEC SOLAR GUIDANCE



### NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION

Division of Water, Bureau of Water Compliance 625 Broadway, Albany, New York 12233-3506 P: (518) 402-8177 | F: (518) 402-8082 www.dec.ny.gov

October 23, 2024

Noah Ginsburg Executive Director New York State Energy Industries Association P.O. Box 1523 Long Island City, New York 11101

Dear Noah Ginsburg,

Thank you for your engagement efforts seeking to clarify New York State Department of Environmental Conservation (NYSDEC) requirements related to solar development. New York State Energy Industries Association (NYSEIA) provided correspondence and questions dated January 25, 2024 and June 26, 2024 seeking engagement on modernization of solar development techniques. In your correspondence, you raised scenarios and questions regarding common stormwater practices related to the NYSDEC Construction Stormwater General Permit (CGP), New York Standards and Specifications for Erosion and Sediment Control (Blue Book) dated November 2016, New York State Stormwater Management Design Manual (2024 White Book) dated July 2024, and use of common solar development methods. Your letter refers to a DEC memo of April 5, 2018 (Solar Memo) providing interpretation of the CGP in relation to solar installation. This letter responds to the correspondence and aids in the implementation of solar development consistent with clean energy goals, the CGP, and design standards while maintaining sound stormwater management related to these developments.

The following answers to these questions should be examined comprehensively as many factors together ultimately decide what post-construction treatment, if any, is required by the CGP and any specific project. When considering whether tables (solar panel arrays) should be modeled as pervious or impervious, NYSDEC recognizes that tables can be designed or constructed in such a way that runoff and infiltration can occur between and underneath tables. Variables such as table height, spacing between tables, site slope, soil conditions, vegetative cover, and hydrology analysis are studied together to determine whether runoff from tables can be considered disconnected for purposes of post-construction treatment practice selection or not.

The following are NYSEIA bullet points/questions and responses. For all responses, these are NYSDEC's interpretation of CGP, Blue Book, and 2024 White Book provisions which should be reviewed and considered for each site-specific application and govern any final determination for that application.



### **NYSDEC Scenario 1 Issues (Solar Memo):**

Item #2: The panels are spaced apart so that rain can flow off the down gradient side of the panel and continue to sheet flow across the ground surface.

### 1. NYSEIA Question:

Are the entirety of the solar rows and tables (made up of individual solar modules) considered one large impervious area? Typically, solar modules are installed to include a  $\sim$ 1" gap around all edges which will prevent one large leading drip edge and avoid significant drip edge erosion.

### **NYSDEC Response:**

While the actual solar table may be impervious, solar array field areas may be designed to be disconnected impervious area and qualify for impervious area reduction when computing water quality volume requirements. To clarify, NYSDEC has attached a figure (Diagram 1: NYSDEC Solar Plan View) representing common terms and practices. It is typical practice to consider the leading drip edge as the most useful scenario to model in considering impact on sheet flow from the table. Combined with site characteristics like vegetative cover, soil conditions, panel spacing and slope, design professionals can design sites to avoid erosion, promote sheet flow and optimize the perviousness of the ground surface.

In addition to site characteristics, table height can also influence infiltration capability. For solar panel projects where the panels are mounted directly to the ground with no space below panel to allow for the desired vegetative growth and infiltration of runoff, the panels would not meet the criteria for disconnection of impervious area. NYSDEC has not accepted or developed a specific table height requirement. A height requirement, like the assessment of slope and soil conditions, is a component of a designer's best professional judgement. However, the designer should keep in mind that the tables must be elevated enough to allow for permanent cover of 80% or more as required by the CGP definition for final stabilization. It is also noted that different seed mixes may factor into height requirements and become part of the design consideration for the designer and Landscape Architect team.

### 2. NYSEIA Question

Please explain what you mean by the panels need to be spaced apart.

### NYSDEC Response:

Panel spacing is consistent with the concept of disconnection of impervious area (Section 5.3.4 of the 2024 White Book) and is an important component to consider when avoiding erosive conditions. Disconnection can be achieved through adequate spacing of panel rows over areas of well-established vegetative cover (see Diagram 2: NYSDEC Solar Profile View: Inter-Row Space). For the designer to meet the intent of disconnection, a design should consider the largest solar panel width, considering all solar panel rotations/positions

and provide that width as the minimum spacing downgradient of every row of solar panels in the solar array field area. If inter-row space requirements are insufficient, infiltration is less likely, resulting in panels that are more likely to create concentrated flow paths and perform as impervious. For solar array field areas that have insufficient spacing for infiltration, the disconnection credit for runoff reduction would not be applicable. As a result, the SWPPP would need to address post-construction stormwater management control practices.

### Item #3: For solar panels constructed on slopes, the individual rows of solar panels are generally installed along the contour so rainwater sheet flows down slope.

### 3. NYSEIA Question:

If panels aren't "generally installed along the contour" does that automatically mean the area of solar panels should be considered impervious?

### **NYSDEC Response:**

No. NYSDEC recognizes that rows of solar tables are frequently constructed to track the sun's path, not along the contour. The main intent of considering placement along the contour is to ensure that precipitation coming off the panels consists of sheet flow across the site. As explained in the previous section, practices that maintain sheet flow and promote infiltration are necessary to meet CGP sizing criteria for Water Quality Volume (WQv) and Runoff Reduction Volume (RRv).

### 4. NYSEIA Question:

If panels aren't "generally installed along the contour" does that mean the area of solar panels wouldn't be allowed to function as a filter strip?

### **NYSDEC Response:**

Through proper design and installation of panels even if not along the contour, the site could still be designed to allow the area to function as a pervious filter strip.

The concept behind the Sheet Flow to Filter Strip practice is outlined in Section 5.3.2 of the 2024 White Book and is important when considering stormwater management needs on solar array field areas. Precipitation falls to a well-vegetated surface with suitable characteristics between the panels and then across the site, which allows infiltration. This meets the WQv and RRv sizing criteria in the CGP for this portion of the solar array field area. Since the panels are not treated as connected impervious area, the 10 ft. pretreatment requirement is not necessary for each row of tables.

In accordance with Table 5.7 of the 2024 White Book, it is noted that there are slope limitations for the filter strip. They can only be used on slopes less than or equal to 8%. This is to ensure that sheet flow can be maintained. If slopes are greater than 8%, then there are two main alternatives:

- a) Design and implement permanent engineered practices from the Blue Book and White Book or use the NYSDEC Gravel Diaphragm detail (Diagram 3) along the slope/contour, to reestablish and maintain sheet flow, which will meet RRv and WQv requirements. It is worth noting that the use of the gravel diaphragm has been effectively implemented in many scenarios. OR
- b) Where sheet flow cannot be maintained, consider the panels in that area to be connected impervious and design post-construction stormwater management control practices to provide water quality and quantity controls for this portion of the solar array field area consistent with the CGP and White Book. Some acceptable design elements used on other sites for RRv and WQv include gravel diaphragms (see Diagram 3: NYSDEC Gravel Diaphragm Along the Contour), level spreaders and flow diffusers. Practices such as water bars have generally been shown to not be effective for large areas as they are diversion practices designed to convey stormwater to a stable outlet. Water bars as a practice to maintain sheet flow have posed their own unique set of issues on several sites including unintended ponding, traffic concerns, and poorly stabilized outlets.

### Item #4: The ground surface below the panels shall consist of a well-established vegetative cover (see "Final Stabilization" definition in Appendix A of the CGP).

### 5. NYSEIA Question:

Please confirm that in addition to the space under the panel, the ground surface between the rows needs to have well established vegetation.

### **NYSDEC Response:**

This is correct. For the ground surface under the panel to aid in creating a pervious area, a well-established vegetative cover is required for the final stabilization of a site post construction. Please see the definition of "final stabilization" in the CGP.

### Item #5: The project does not include the construction of any traditional impervious areas (i.e., buildings, substation pads, gravel access roads or parking areas, etc.)

### 6. NYSEIA Question:

All solar facilities will require some impervious areas in the form of concrete or gravel equipment areas and access roads that are fire code compliant. These areas are typically treated with stormwater controls (with stone areas or adjacent grass filter strips). Please explain the rationale behind Item # 5 and how developers should address impervious areas within the proposed solar facility.

### NYSDEC Response:

The CGP provides sizing criteria in Part I.C and Part III.C. to address impervious areas. When RRv and WQv are not met for the ancillary areas and/or tables, designers must implement post-construction stormwater practices in accordance with the White Book for those areas. If designers are looking for relief from designing post-construction practices, there are some approved alternatives such as limited use pervious access roads and alternative stormwater management practices for substations to achieve sizing criteria and reduce post-construction practices.

### 7. NYSEIA Question:

Does DEC require pervious access roads into the solar facility? If so, is there a typical you can provide for developers to reference?

### NYSDEC Response:

No, pervious access roads are not required by the CGP and are not listed in the 2024 White Book and therefore are not required on renewable energy sites. It is worth noting that on many solar sites, the pertinent design requirements for the pervious access road detail cannot be met. For example, there are slope limitations. Cross slopes cannot exceed 6% and the longitudinal slope should not exceed 15%. Additionally, these roads should not be utilized during construction as it may impact soil or other conditions and require testing and re-construction to be effective as a pervious feature. Many sites do not have enough available area to construct both temporary and permanent roads in separate footprints. Finally, the limited use pervious access road cannot be constructed or used until all areas subject to runoff onto the road have achieved final stabilization. Attached, please find a copy of a typical limited use pervious access road detail that has been accepted in SWPPP review for your reference.

One alternative that has been accepted on other sites is the revegetated gravel road scarification detail for upland roads and work areas. This can be used in areas that are not wetlands and are not agricultural areas where restoration is required. This road is meant for minimal intermittent use and does not have the load-bearing capacity to serve as emergency access for the site.

Maryland DE's "Stormwater Design Guidance- Solar Panel Installations," Page 1, 2<sup>nd</sup> Paragraph: "Commonly used with smaller or narrower impervious areas like driveways or open roads, the Disconnection of Non-Rooftop Runoff technique (see pp. 5.61 to 5.65 of the 2000 Maryland Stormwater Design Manual) is a low-cost alternative for treating runoff in situations like rows of solar panels."

### 8. NYSEIA Question:

Should the Maryland Department of the Environment (MDE) Stormwater Design Guidance be applicable in NYS? We feel that the guidance is outdated and doesn't represent typical field conditions in New York.

### NYSDEC Response:

While the Maryland Department of the Environment's (MDE) Stormwater Guidance aided in NYSDEC's initial understanding of solar guidance standard development, designs should meet New York State standards. NYSDEC did not propose the MDE document as guidance or intend it to be adopted it in its entirety. Primarily, NYSDEC intended to reference use of the diagrams as suitable alternative practices. To eliminate confusion, changes have been proposed in the draft version of the CGP.

Maryland DE's "Stormwater Design Guidance-Solar Panel Installations," Page 1,  $3^{rd}$  Bullet: "Disconnections should be located on gradual slopes ( $\leq 5\%$ ) to maintain sheet flow. Level spreaders, terraces, or berms may be used to maintain sheet flow conditions if the average slope is steeper than 5%. However, installations on slopes greater than 10% will require an engineered plan that ensures adequate treatment and the safe and non-erosive conveyance of runoff to the property line or downstream stormwater management practice."

### 9. NYSEIA Question:

If panels are placed on slopes greater than 10% does that automatically make them impervious?

**NYSDEC Response:** 

No. Please refer to Question 4.

### 10. NYSEIA Question:

Please clarify the requirement for an "engineered plan."

### NYSDEC Response:

With regard to the Maryland guidance, NYSDEC's original intent was to reference diagrams and not the document as a whole. The draft CGP provides clarification as to the need for post-construction controls for solar array field areas. The need for an "engineered plan" references the use of suitable design practices from the White Book for post-construction controls for RRv, WQv, and quantity based on site conditions and solar table layout.

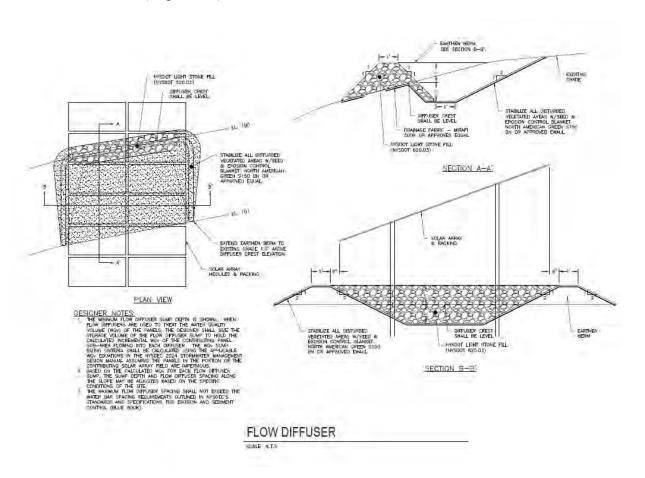
### 11. NYSEIA Question:

If the panels are placed on slopes greater than 10%, what is acceptable to demonstrate developers are designing a site with effective stormwater management practices? In our experience we have proposed the following accepted solutions:

To date, the majority of our solutions have been gravel level spreaders/diaphragms on slopes above 10%, generally spaced throughout the solar panel area site using the NYSDEC blue book spacing for like water bars.

In steep panel areas (e.g. in excess of  $\sim 20\%$  slopes), we have had instances where we provided additional attenuation (reducing runoff rates further than what is required by the Stormwater Design Manual) in order to satisfy Town engineers.

An alternative "engineered" solution pioneered by Marathon Engineering have been flow diffusers which he has used at staggered locations primarily beneath the panels to catch water leaving the panels and slow it down while avoiding most of the drive aisles for construction (snip below).



### **NYSDEC Response:**

NYSDEC acknowledges that well-designed projects on slopes steeper than 10 percent have successfully implemented some of the alternative practices mentioned such as flow diffusers and stone diaphragms. The flow diffuser detail (above) from Marathon Engineering has been applied on several sites. If there are instances that trigger 60 day review, please note that it will aid in review if you can provide examples of previous projects that this practice has been approved on.

It has come to NYSDEC's attention that there are a variety of practices that may be useful in managing stormwater on solar sites that have been approved in other states besides New York. NYSDEC will be seeking to include examples of these practices in future training. It is important that when submitting alternative practices for review, that as much documentation is provided to satisfy the Criteria for Practice addition in Section 3.3.2 in the White Book including an 80% reduction in total suspended solids (TSS) and 40% reduction in total phosphorous (TP).

The CGP, White Book, and Blue Book do not exclude solar development on steep slopes or establish specific practices for solar development. For these challenging projects, the designer will need to carefully consider practices and controls during design and provide sufficient information in the SWPPP to facilitate evaluation by NYSDEC staff, and manage erosion and sedimentation during construction, and post-construction stormwater management for the project. For solar projects on steep slopes, it is encouraged that NYSDEC, project developer, designer and contractors work together to ensure that New York's goals for renewable energy are achieved in an environmentally responsible manner and that balance water quality and climate goals.

### **Additional Question #1**

### 12. NYSEIA Question:

Are solar trackers (solar tables/rows which move with the sun) considered impervious? Do the current guidelines apply to them? The examples in the 2018 Solar Guidance Memo only show fixed tilt. How can a developer ensure these systems follow DEC guidelines?

### NYSDEC Response:

Solar trackers are not considered disconnected impervious area unless they meet the criteria discussed above. The SWPPP must demonstrate that the site conditions and placement of the solar panels will allow for disconnection and achieve the runoff reduction objectives of the CGP in order to apply the area reduction credit for the panels in all positions.

### **Additional Question #2**

### 13. NYSEIA Question:

On slopes under 8%, can the meadow grass beneath the modules be counted as part of a filter strip to satisfy water quality and RRv? We have typically started our filter strips outside of the system area due to Town Designated Engineer preference and understanding of the "Filter Strip" practice, but we feel the area under the solar panels should suffice as a filter strip.

### **NYSDEC Response:**

The area between the panels counts toward the filter strip. Since the panels are not treated as connected impervious area, the 10 ft. pretreatment requirement is not necessary for each row of tables. Please refer to Question 4.

We hope this information has been of assistance. Although currently solar development falls under Appendix B Table 2 of the current 2020 version of the CGP as "all other construction activities that include the construction or reconstruction of impervious area or alter the hydrology from pre to post development conditions and are not listed in Table 1" the 2024 White Book and the CGP have provisions that allow disconnection of impervious areas. Use of the clarifications provided in this letter will allow design and development practices under the requirements of the CGP and 2024 White Book to be applied for disconnection and should be utilized in lieu of the 2018 Solar Memo. Additionally, provisions for solar development are proposed in the 2024 draft of the new CGP. We hope you were able to review the draft and provide feedback during the public comment period which ended October 1, 2024.

Sincerely,

Edward Hampston, P.E.

Edward Hampston

Director, Bureau of Water Compliance

### Attachments:

- 1. Diagram 1: NYSDEC Solar Plan View
- 2. Diagram 2: NYSDEC Solar Profile View: Inter-Row Space
- 3. Diagram 3: NYSDEC Gravel Diaphragm Along the Contour
- 4. Limited use pervious access road detail
- 5. Gravel road scarification detail for upland roads and work areas
- 6. NYSEIA Letter dated June 26<sup>th</sup>, 2024

cc: C. Lamb-Lafay, PE, NYSDEC

A. Smith, NYSDEC

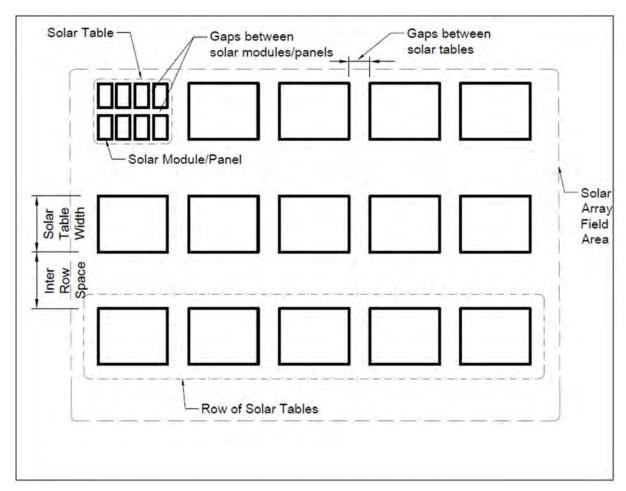
J. Melançon, CPESC. NYSDEC

C. Warner, NYSERDA



### 2024 Solar Memo - Terminology

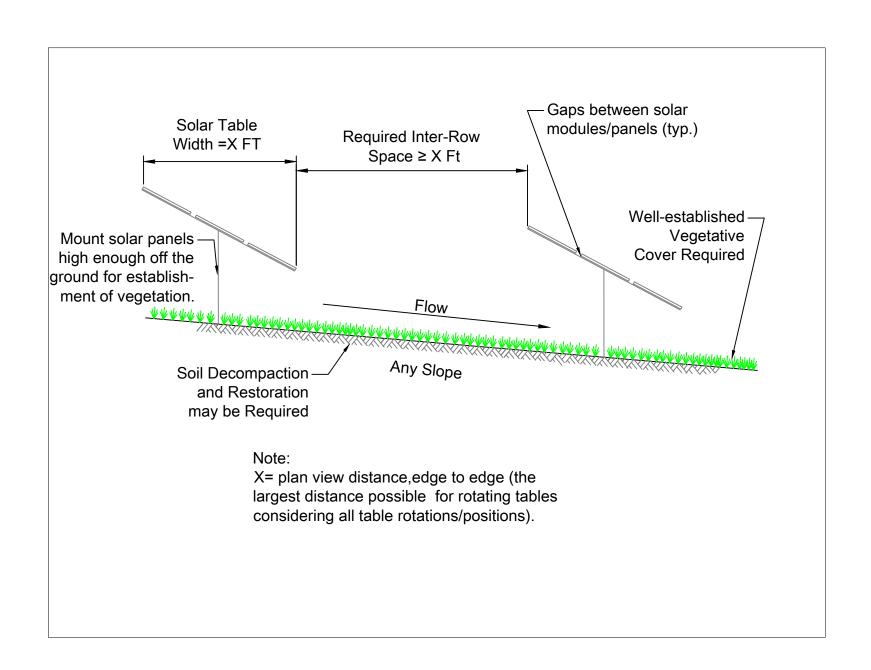
**Diagram 1: NYSDEC Solar Plan View** N.T.S.

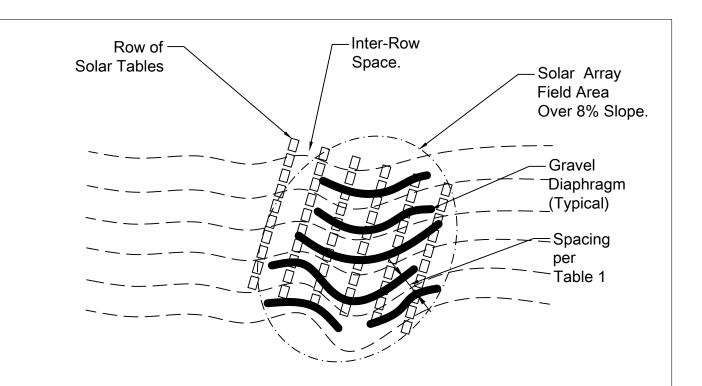


### \*Definitions:

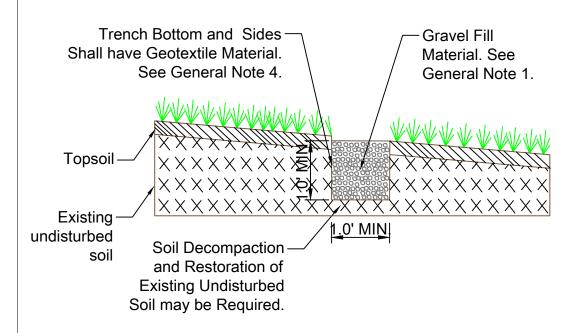
\*NYSDEC provides the above diagram and the following definitions for solar terminology for the specific purpose of demonstrating how to interpret and apply the NYSDEC solar guidance. The following definitions are intended for that limited context only and are not meant to be used for any other purpose.

- Gap between solar table: the distance between two solar tables within a row of solar tables. This gap varies depending on the layout and design.
- Gaps between solar modules/panels: the distance(s) between solar modules/panels within a solar table. These gaps vary depending on the layout and design.
- Inter-row space: the plan view distance between two solar table rows. NYSDEC solar guidance sets the minimum inter-row space requirements; see Diagram 2: NYSDEC Solar Profile View: Inter-Row Space Requirements. This requirement is based on the solar table upgradient of the inter-row space.
- Solar array field area: the area of a solar site that includes the rows of solar tables, their inter-row spaces, and the surrounding buffer area. This area does not include access roads and other traditional impervious areas, even if those are located within the solar array field area.
- Solar module/panel: a unit comprised of photovoltaic cells.
- Solar table: one or more modules/panels wired and framed together. Solar tables may be a single module/panel, a single row of modules/panels, or stacked rows of modules/panels. The number of modules/panels framed and stacked together varies.
- Solar table width: the plan view distance, edge to edge, of a solar table. It is the largest distance possible for rotating tables considering all table rotations/positions.





### SOLAR ARRAY FIELD AREA WITH GRAVEL DIAPHRAGMS PLAN VIEW



**GRAVEL DIAPHRAGM CROSS SECTION** 

### **GENERAL NOTES:**

- 1. USE OF THIS DETAIL/CRITERION IS LIMITED TO ACCESS ROADS USED ON AN OCCASIONAL BASIS ONLY (I.E. PROVIDE ACCESS FOR MOVING, EQUIPMENT REPAIR OR MAINTENANCE, ETC.).
- 2. LIMITED USE PERVIOUS ACCESS ROAD IS LIMITED TO LOW IMPACT IRREGULAR MAINTENANCE ACCESS ASSOCIATED WITH RENEWABLE ENERGY PROJECTS IN NEW YORK STATE.
- 3. REMOVE STUMPS, ROCKS AND DEBRIS AS NECESSARY. FILL VOIDS TO MATCH EXISTING NATIVE SOILS AND COMPACTION LEVEL.
- 4. REMOVED TOPSOIL MAY BE SPREAD IN ADJACENT AREAS A DIRECTED BY THE PROJECT ENGINEER. DO NOT PLACE IN AN AREA THAT IMPEDES STORMWATER DRAINAGE.
- 5. GRADE ROADWAY, WHERE NECESSARY, TO NATIVE SOIL AND DESIRED ELEVATION. MINOR GRADING FOR CROSS SLOPE CUT AND FILL MAY BE REQUIRED.
- 6. REMOVE REFUSE SOILS AS DIRECTED BY THE PROJECT ENGINEER. DO NOT PLACE IN AN AREA THAT IMPEDES STORMWATER DRAINAGE.
- 7. ROADWAY WIDTH TO BE DETERMINED BY CLIENT.
- 8. THE LIMITED USE PERVIOUS ACCESS ROAD CROSS SLOPE SHALL BE 2% IN MOST CASES AND SHOULD NOT EXCEED 6%. THE LONGITUDINAL SLOPE OF THE ACCESS DRIVE SHOULD NOT EXCEED 15%
- 9. LIMITED USE PERVIOUS ACCESS ROAD IS NOT TO BE UTILIZED FOR CONSTRUCTION WHICH MAY SUBJECT THE ACCESS TO SEDIMENT TRACKING. THIS SPECIFICATION IS TO BE DEVELOPED FOR POST—CONSTRUCTION USE. SOIL RESTORATION PRACTICES MAY BE APPLICABLE TO RESTORE CONSTRUCTION RELATED COMPACTION TO PRE—EXISTING CONDITIONS AND SHOULD BE VERIFIED BY SOIL PENETROMETER READINGS. THE PENETROMETER READINGS SHALL BE COMPARED TO THE RESPECTIVE RECORDED READINGS TAKEN PRIOR TO CONSTRUCTION, EVERY 100 LINEAR FEET ALONG THE PROPOSED ROADWAY.
- 10. TO ENSURE THAT SOIL IS NOT TRACKED ONTO THE LIMITED USE PERVIOUS ACCESS ROAD, IT SHALL NOT BE USED BY CONSTRUCTION VEHICLES TRANSPORTING SOIL, FILL MATERIAL, ETC. IF ACCESS IS COMPLETED DURING THE INITIAL PHASES OF CONSTRUCTION, A STABILIZED CONSTRUCTION ACCESS/ENTRANCE IS REQUIRED TO REMOVE SEDIMENT FROM CONSTRUCTION VEHICLES AND EQUIPMENT PRIOR TO ENTERING THE LIMITED USE PERVIOUS ACCESS ROAD. MAINTENANCE OF THE PERVIOUS ACCESS ROAD WILL BE REQUIRED IF SEDIMENT IS OBSERVED WITHIN THE CLEAN STONE.
- 11. THE LIMITED USE PERVIOUS ACCESS ROAD SHALL NOT BE CONSTRUCTED OR USED UNTIL ALL AREAS SUBJECT TO RUNOFF ONTO THE PERVIOUS ACCESS HAVE ACHIEVED FINAL STARILIZATION
- 12. PROJECTS SHOULD AVOID INSTALLATION OF THE LIMITED USE PERVIOUS ACCESS ROAD IN POORLY DRAINED AREAS, HOWEVER IF NO ALTERNATIVE LOCATION IS AVAILABLE, THE PROJECT SHALL UTILIZE WOVEN GEOTEXTILE MATERIAL AS DETAILED IN FOLLOWING NOTES.
- 13. THE DRAINAGE DITCH IS OFFERED IN THE DETAIL FOR CIRCUMSTANCES WHEN CONCENTRATING FLOW COULD NOT BE AVOIDED. THE INTENTION OF THIS DESIGN IS TO MINIMIZE ALTERATIONS TO HYDROLOGY, HOWEVER WHEN DEALING WITH 5%—15% GRADES NOT PARALLEL TO THE CONTOUR, A ROADSIDE DITCH MAY BE REQUIRED. THE NYS STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROLS FOR GRASSED WATERWAYS AND VEGETATED WATERWAYS ARE APPLICABLE FOR SIZING AND STABILIZATION. DIMENSIONS FOR THE GRASSED WATERWAY SPECIFICATION WOULD BE DESIGNED FOR PROJECT SPECIFIC HYDROLOGIC RUNOFF CALCULATIONS, AND A SEPARATE DETAIL FOR THE SPECIFIC GRASSED WATERWAY WOULD BE INCLUDED IN THIS PRACTICE. RUNOFF DISCHARGES WILL BE SUBJECT TO THE OUTLET REQUIREMENTS OF THE REFERENCED STANDARD. INCREASED POST—DEVELOPMENT RUNOFF FROM THE ASSOCIATED ROADSIDE DITCH MAY REQUIRE ADDITIONAL PRACTICES TO ATTENUATE RUNOFF TO PRE—DEVELOPMENT CONDITIONS.
- 14. IF A ROADSIDE DITCH IS NOT UTILIZED TO CAPTURE RUNOFF FROM THE ACCESS ROAD, THE PERVIOUS ACCESS ROAD WILL HAVE A WELL-ESTABLISHED PERENNIAL VEGETATIVE COVER, WHICH SHALL CONSIST OF UNIFORM VEGETATION, 20 FEET PARALLEL TO THE DOWN GRADIENT SIDE OF THE ACCESS ROAD. POST-CONSTRUCTION OPERATION AND MAINTENANCE PRACTICES WILL MAINTAIN THIS VEGETATIVE COVER TO ENSURE FINAL STABILIZATION FOR THE LIFE OF THE ACCESS ROAD.
- 15. THE DESIGN PROFESSIONAL MUST ACCOUNT FOR THE LIMITED USE PERVIOUS ACCESS ROAD IN THEIR SITE ASSESSMENT/HYDROLOGY ANALYSIS. IF THE HYDROLOGY ANALYSIS SHOWS THAT THE HYDROLOGY HAS BEEN ALTERED FROM PRE— TO POST—DEVELOPMENT CONDITIONS (SEE APPENDIX A OF GP—0—15—002 FOR THE DEFINITION OF "ALTER THE HYDROLOGY..."), THE DESIGN MUST INCLUDE THE NECESSARY DETENTION/RETENTION PRACTICES TO ATTENUATE THE RATES (10 AND 100 YEAR EVENTS) TO PRE—DEVELOPMENT CONDITIONS.

### GEOGRID MATERIAL NOTES:

- 1. THE GEOGRID, OR COMPARABLE PRODUCT, IS INTENDED FOR USE FOR ALL CONDITIONS, IN ORDER TO ASSIST IN MATERIAL SEPARATION FROM NATIVE SOILS AND PRESERVE ACCESS LOADS.
- 2. GRAVEL FILL MATERIAL SHALL CONSIST OF 1-4" CLEAN, DURABLE, SHARP-ANGLED CRUSHED STONE OF UNIFORM QUALITY, MEETING THE SPECIFICATIONS OF NYSDOT ITEM 703-02, SIZE DESIGNATION 3-5 OF TABLE 703-4. STONE MAY BE PLACED IN FRONT OF, AND SPREAD WITH, A TRACKED VEHICLE. GRAVEL SHALL NOT BE COMPACTED.
- 3. GEOGRID SHALL BE MIRAFI BXG110 OR APPROVED EQUAL. GEOGRID SHALL BE DESIGNED BASED ON EXISTING SOIL CONDITIONS AND PROPOSED HAUL ROAD SLOPES.
- 4. IF MORE THAN ONE ROLL WIDTH IS REQUIRED, ROLLS SHOULD OVERLAP A MINIMUM OF SIX
- 5. REFER TO MANUFACTURER'S SPECIFICATION FOR PROPER TYING AND CONNECTIONS.
- 6. LIMITED USE PERVIOUS ACCESS ROAD SHALL BE TOP DRESSED AS REQUIRED WITH ONLY 1-4" CRUSHED STONE MEETING NYSDOT ITEM 703-02 SPECIFICATIONS.
- BASIS OF DESIGN: TENCATE MIRAFI BXG110 GEOGRIDS; 365 SOUTH HOLLAND DRIVE, PENDERGRASS, GA;800-685-9990 OR 706-693-2226; WWW.MIRAFI.COM

### GEOWEB MATERIAL NOTES:

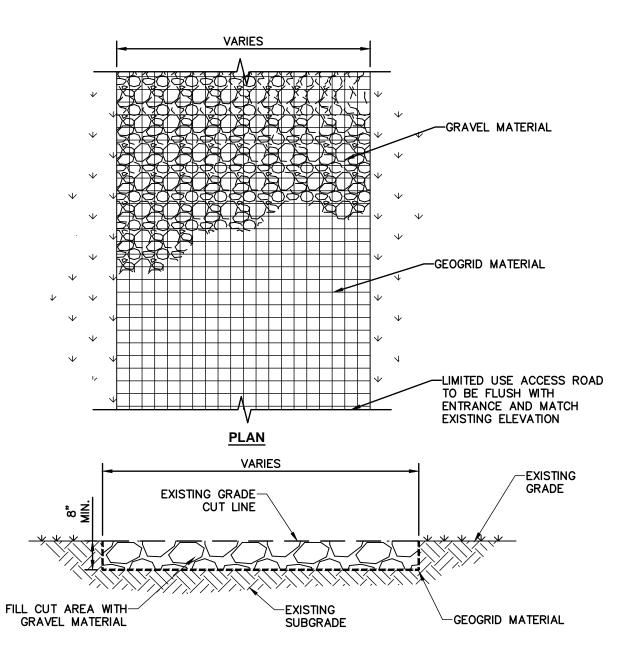
- 1. THE GEOWEB, OR COMPARABLE PRODUCT, IS SUGGESTED FOR USE ON ROAD PROFILES EXCEEDING 5%. THE GEOWEB PRODUCT IS INTENDED TO LIMIT SHIFTING STONE MATERIAL
- 2. INSTALLATION TO BE COMPLETED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.
- 3. WHERE REQUIRED, A NATIVE SOIL WEDGE SHALL BE PLACED TO ACCOMMODATE ROAD CROSS SLOPE OF 2%. NATIVE SOIL SHALL BE COMPACTED TO MATCH EXISTING SOIL CONDITIONS.
- 4. GRAVEL FILL MATERIAL SHALL CONSIST OF 1-4" CLEAN, DURABLE, SHARP-ANGLED CRUSHED STONE OF UNIFORM QUALITY, MEETING THE SPECIFICATIONS OF NYSDOT ITEM 703-02, SIZE DESIGNATION 3-5 OF TABLE 703-4. STONE MAY BE PLACED IN FRONT OF, AND SPREAD WITH, A TRACKED VEHICLE. GRAVEL SHALL NOT BE COMPACTED.
- 5. GEOWEB SYSTEM SHALL BE PRESTO GEOSYSTEM GEOWEB OR APPROVED EQUAL. GEOWEB SHALL BE DESIGNED BASED ON EXISTING SOIL CONDITIONS AND PROPOSED HAUL ROAD SLOPES.
- 6. LIMITED USE PERVIOUS ACCESS ROAD SHALL BE TOP DRESSED AS REQUIRED WITH ONLY 1-4" CRUSHED STONE MEETING NYSDOT ITEM 703-02 SPECIFICATIONS.
- 7. THE TOP EDGES OF ADJACENT CELL WALLS SHALL BE FLUSH WHEN CONNECTING. ALIGN THE I—SLOTS FOR INTERLEAF AND END TO END CONNECTIONS. THE GEOWEB PANELS SHALL BE CONNECTED WITH ATRA KEYS AT EACH INTERLEAD AND END TO END CONNECTIONS. REFER TO MANUFACTURER'S SPECIFICATION FOR PROPER INSTALLATION, TYING AND CONNECTIONS.
- BASIS OF DESIGN: PRESTO GEOSYSTEMS GEOWEB; 670 NORTH PERKINS STREET, APPLETON, WI; 800-548-3424 OR 920-738-1222; INFO@PRESTOGEO.COM;

### **WOVEN GEOTEXTILE MATERIAL NOTES:**

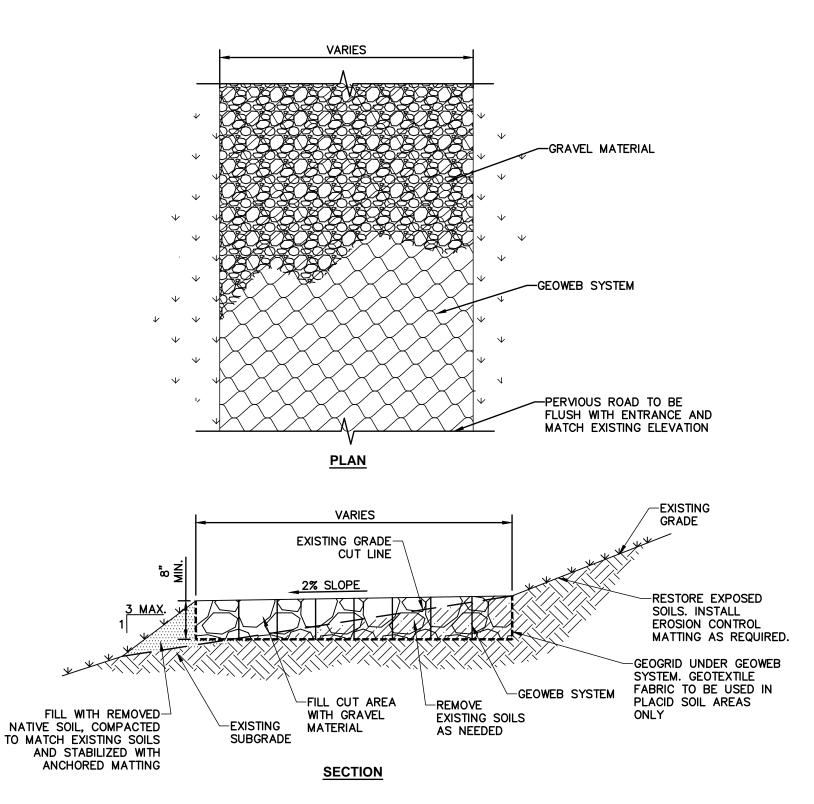
WWW.MIRAFI.COM

- 1. SPECIFIED GEOTEXTILE WILL ONLY BE UTILIZED IN PLACID SOILS. PLACID SOILS CONSIST OF POORLY DRAINED SOILS COMPOSED OF FINELY TEXTURED PARTICLES AND ARE PRONE TO RUTTING. PLACID SOILS ARE TYPICALLY PRESENT IN LOW-LYING AREAS WITH HYDROLOGIC SOILS GROUP (HSG) OF C OR D, OR AS SPCIFIED FROM AN ENVIRONMENTAL SCIENTIST, SOIL SCIENTIST, OR GEOTECHNICAL DATA.
- 2. THE CONCERN FOR POTENTIAL REDUCTION OF NATIVE INFILTRATION RATES DUE TO THE GEOTEXTILE MATERIAL WOULD NOT BE A SIGNIFICANT CONCERN IN POORLY DRAINED SOILS WHERE SEGREGATION OF PERVIOUS STONE AND NATIVE MATERIALS IS CRUCIAL FOR LONG TERM OPERATION AND MAINTENANCE.

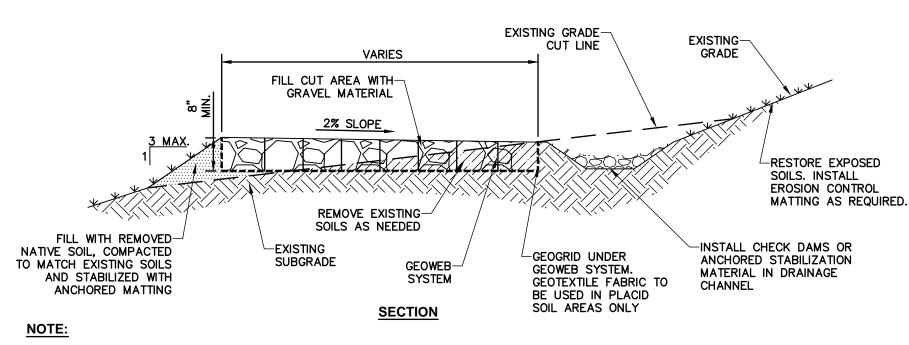
BASIS OF DESIGN: TENCATE MIRAFI RSI-SERIES WOVEN GEOSYNTHETICS; 365 SOUTH HOLLAND DRIVE, PENDERGRASS, GA; 800-685-9990 OR 706-693-2226;



1 LIMITED USE PERVIOUS ACCESS ROAD - 0% TO 5% SLOPES SCALE: N.T.S.

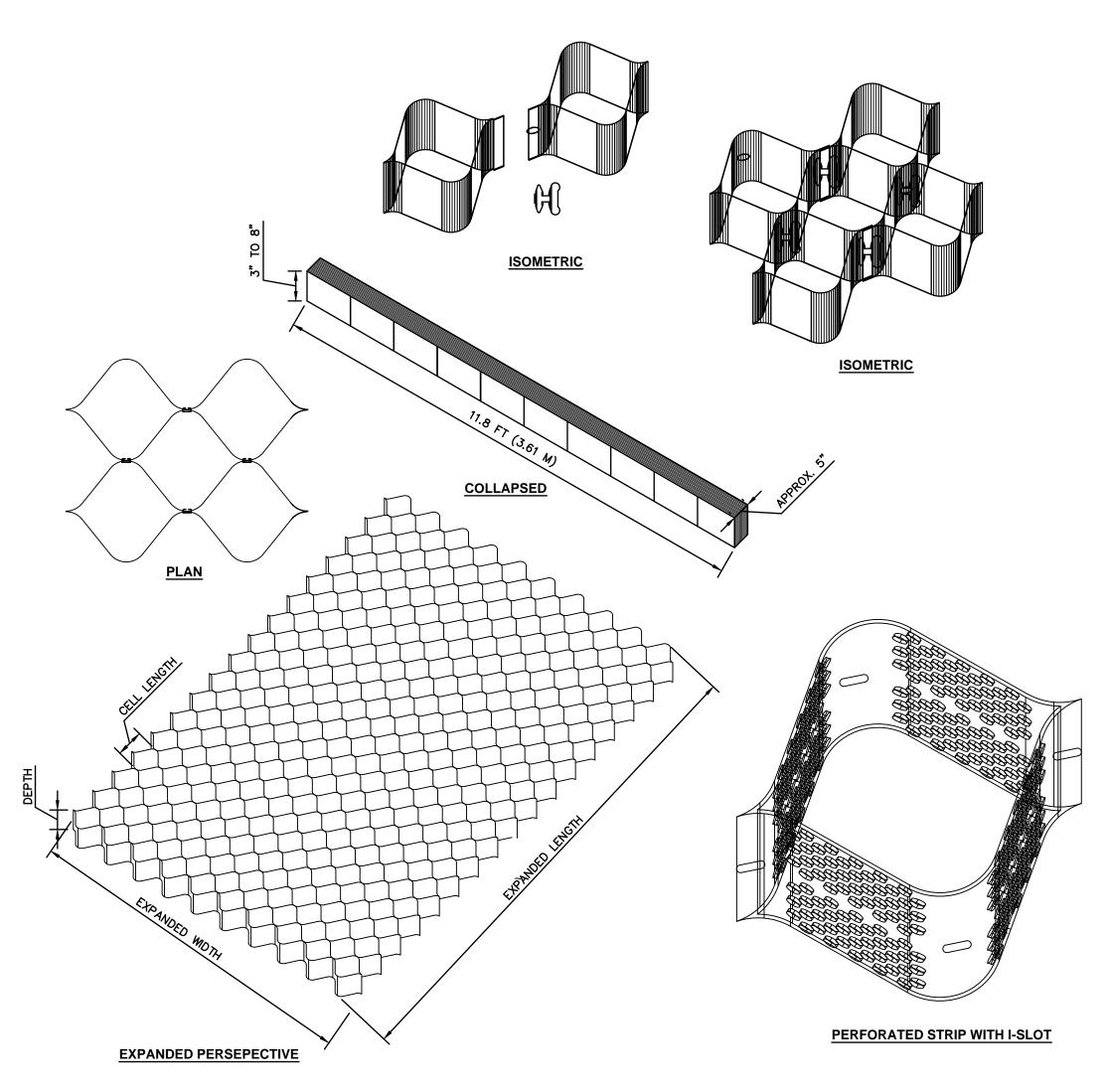


LIMITED USE PERVIOUS ACCESS ROAD - 5% TO 15% SLOPES



1. THE ROADSIDE DITCH SHALL BE DESIGNED IN ACCORDANCE WITH THE NEW YORK STATE STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROLS FOR GRASSED AND VEGETATED WATERWAYS. ADDITIONAL DETAILS WILL BE PROVIDED SPECIFIC TO THE SITE DESIGN.

LIMITED USE PERVIOUS ACCESS ROAD - 5% TO 15% SLOPES WITH DITCH SCALE: N.T.S.



GEOWEB SYSTEM SCALE: N.T.S.

1 ISSUED AS FINAL 10/30/2018

NO. REVISION DATE

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DRAWING TITLE:

LIMITED USE PERVIOUS ACCESS ROAD DETAIL

SCALE: N.T.S.

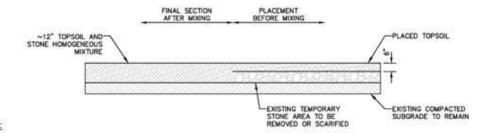
DATE: 10/16/2018

DRAWN BY: CAK

CHECKED BY: SML

PROJECT:

DRAWING NO.:



### NOTES:

- 1, REFER TO THE PLAN DRAWINGS FOR LOCATIONS OF STONE WORK PADS AND ACCESS ROADS TO BE SCARIFIED.
- 2. FINISHED ROAD AND PAD ELEVATIONS SHALL REMAIN AFTER CONSTRUCTION UNLESS OTHERWISE INDICATED ON THE PLANS.
- 3 IN GENERAL, STONE MATERIAL SHALL REMAIN AFTER CONSTRUCTION AND BE MIXED WITH 6 INCHES OF TOPSOIL, HOWEVER IN SOME CASES THE STONE MAY BE ENTIRELY REMOVED FROM THE SITE REFER TO THE PLAN DRAWINGS FOR SPECIFIC LOCATIONS.
- 4. STONED AREAS SHALL BE SCARIFIED OR OTHERWISE LOOSENED TO A MINIMUM DEPTH OF 12 INCHES TO PERMIT BONDING OF THE TOPSOIL TOPSOIL SHOULD BE UNIFORMLY DISTRIBUTED ACROSS THE AREA TO BE RESTORED TO A DEPTH OF 6 INCHES (MINIMUM OF 2 INCHES ON FILL OUTSLOPES) AND INCORPORATED OR TILLED INTO SCARIFIED STONE AREA.
- SPREADING SHALL BE DONE IN A MANNER SUCH THAT SEEDING CAN BE COMPLETED WITH A MINIMUM OF ADDITIONAL TILLAGE.
  IRREGULARITIES OF THE SURFACE RESULTING FROM PLACEMENT SHALL BE CORRECTED IN ORDER TO PREVENT FORMATION OF
  DEPRESSIONS.
- 6. PRE-PLANTING FERTILIZERS AND PH ADJUSTING AGENTS MAY BE APPLIED PRIOR TO INCORPORATING.
- RESTORATION AREA TO BE SEEDED IN ACCORDANCE WITH RECOMMENDATIONS OF LOCAL COUNTY EXTENSION SERVICE. SEED
  SHALL BE APPLIED VIA DRILL SEEDER, HYDROSEEDER OR BROADCAST SPREADER ALONG WITH A CARRIER. RAKE SEEDED AREA
  TO ENSURE PROPER SOIL-SEED CONTACT AND APPLY STRAW MULCH AT 3 TONS/ACRE TO PRESERVE MOISTURE.
- 8. ESTABLISHMENT OF VEGETATION ON THE RESTORED ACCESS ROAD SHALL BE MAINTAINED BY THE CONTRACTOR INCLUDING BUT NOT LIMITED TO SUPPLEMENTAL WATERING, FERTILIZATION, SEEDING AND OTHER TECHNIQUES, UNTIL AN ACCEPTABLE STAND OF VEGETATION HAS BEEN ESTABLISHED. ACCEPTABLE STAND IS GENERALLY CONSIDERED 80% VEGETATION COVERAGE OF ALL SURFACES.
- 9. UPON FINAL RESTORATION, THE SCARIFICATION AREAS SHALL BE REVIEWED BY THE DESIGN PROFESSIONAL TO CONFIRM THAT SITE HYDROLOGY HAS NOT BEEN ALTERED PER APPENDIX C OF THE CONSTRUCTION GENERAL PERMIT IF IT IS DETERMINED THAT THE HYDROLOGY HAS BEEN ALTERED, APPROPRIATE NECESSARY/RETENTION PRACTICES WILL NEED TO BE DESIGNED AND IMPLEMENTED TO RESTORE PRE-CONSTRUCTION HYDROLOGIC CONDITIONS.

REVEGETATED GRAVEL ROAD SCARIFICATION
DETAIL FOR UPLAND ROADS AND WORK AREAS

### Initial Questions/Bullet Points for DEC regarding the Solar Panel Construction Stormwater Permitting/SWPPP Guidance

A number of solar developers have reached out to us with concerns about the DEC's 2018 Solar Panel Construction Stormwater Permitting/SWPPP Guidance memo (attached). The concerns include inconsistencies in the Memo's interpretation and enforcement. The developers expressed concern about their ability to move their projects forward with upcoming interconnection costs, permit applications, etc.

To ensure we meet NYS climate goals, it is important that well designed projects move forward in a timely manner. Please provide clarification on the following items to ensure that developers and reviewers understand and follow the stormwater guidelines as they relate to solar facilities.

### Scenario 1 from 2018 NYSDEC Stormwater and Solar Memo:

Item # 2. The panels are spaced apart so that rain can flow off the down gradient side of the panel and continue to sheet flow across the ground surface.

Questions: Please explain what you mean by the panels need to be spaced apart? Are the entirety
of the solar rows and tables (made up of individual solar modules) considered one large
impervious area? Typically, solar modules are installed to include a ~1" gap around all edges which
will prevent one large leading drip edge and avoid significant drip edge erosion.

### Item # 3. For solar panels constructed on slopes, the individual rows of solar panels are generally installed along the contour so rainwater sheet flows down slope.

- <u>Question</u>: If panels aren't "generally installed along the contour" does that automatically mean the area of solar panels should be considered impervious?
- <u>Question:</u> If panels aren't "generally installed along the contour" does that mean the area of solar panels wouldn't be allowed to function as a filter strip?
- Questions: If the solar facility does not align with the contours, could the developer propose effective engineering designs to ensure sheet flow where the panels can't align with the contours? Are there any designs that you would recommend to meet the intent of this item?

### Item # 4. The ground surface below the panels shall consist of a well-established vegetative cover (see "Final Stabilization" definition in Appendix A of the CGP).

- Question: Please confirm that in addition to the space under the panel, the ground surface between the rows needs to have well established vegetation.

### Item # 5. The project does not include the construction of any traditional impervious areas (i.e., buildings, substation pads, gravel access roads or parking areas, etc.)

- Question: All solar facilities will require some impervious areas in the form of concrete or gravel equipment areas and access roads that are fire code-compliant. These areas are typically treated with stormwater controls (with stone areas or adjacent grass filter strips). Please explain the rationale behind Item # 5 and how developers should address impervious areas within the proposed solar facility.
- <u>Question</u>: Does DEC require pervious access roads into the solar facility? If so, is there a typical you can provide for developers to reference?

### Maryland DE's "Stormwater Design Guidance- Solar Panel Installations"

Page 1. 2nd Paragraph. "Commonly used with smaller or narrower impervious areas like driveways or open roads, the Disconnection of Non-Rooftop Runoff technique (see pp. 5.61 to 5.65 of the 2000 Maryland Stormwater Design Manual) is a low-cost alternative for treating runoff in situations like rows of solar panels."

- Question: Should the Maryland Department of the Environment (MDE) Stormwater Design Guidance be applicable in NYS? We feel that the guidance is outdated and doesn't represent typical field conditions in New York.

Page 1. Bullet 3. "Disconnections should be located on gradual slopes (≤ 5%) to maintain sheetflow. Level spreaders, terraces, or berms may be used to maintain sheetflow conditions if the average slope is steeper than 5%. However, installations on slopes greater than 10% will require an engineered plan that ensures adequate treatment and the safe and non-erosive conveyance of runoff to the property line or downstream stormwater management practice."

Questions: If panels are placed on slopes greater than 10% does that automatically make them impervious? Please clarify the requirement for an "engineered plan". If the panels are placed on slopes greater than 10%, what is acceptable to demonstrate developers are designing a site with effective stormwater management practices? In our experience we have proposed the following accepted solutions:

To date, the majority of our solutions have been gravel level spreaders /diaphragms on slopes above 10%, generally spaced throughout the solar panel area site using the NYSDEC blue book spacing for like waterbars (see snip below).

### STANDARD AND SPECIFICATIONS FOR WATER BAR



### **Definition & Scope**

A permanent or temporary ridge, ridge and channel, a structural channel, or flow deflector, constructed diagonally across a sloping road or utility right-of-way that is subject to erosion to limit the accumulation of erosive velocity of water by diverting surface runoff at pre-designed intervals.

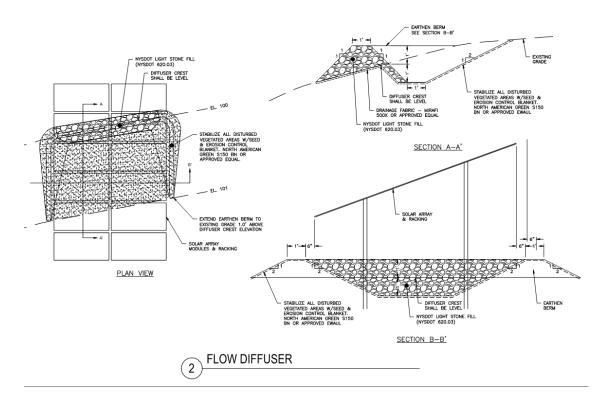
Slope (%)	Spacing (ft.)
<5	125
5 TO 10	100
10 TO 20	75
20 TO 35	50
>35	25

- The positive grade of the water bar shall not exceed 2%. A crossing angle of approximately 60 degrees is preferred.
- Once diverted, water must be conveyed to a stable system (i.e. vegetated swale or storm sewer system). Water bars should have stable, unrestricted outlets, either natural or constructed.

See Figure 3.22 on page 3.53 for details.

In steep panel areas (e.g. in excess of ~20% slopes), we have had instances where we provided additional attenuation (reducing runoff rates further than what is required by the Stormwater Design Manual) in order to satisfy Town engineers.

An alternative "engineered" solution pioneered by Adam Fishel from Marathon have been flow diffusers which he has used at staggered locations primarily beneath the panels to catch water leaving the panels and slow it down while avoiding most of the drive aisles for construction (snip below)



### **Additional Questions:**

- Are solar trackers (solar tables/rows which move with the sun) considered impervious? Do the current guidelines apply to them? The examples in the 2018 Memo only show fixed tilt. How can a developer ensure these systems follow DEC guidelines?
- On slopes under 8%, can the meadow grass beneath the modules be counted as part of a filter strip to satisfy water quality and RRv? We have typically started our filter strips outside of the system area due to Town Designated Engineer preference and understanding of the "Filter Strip" practice, but we feel the area under the solar panels should suffice as a filter strip.

### NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION

Division of Water, Bureau of Water Permits 625 Broadway, Albany, New York 12233-3505 P: (518) 402-8111 | F: (518) 402-9029 www.dec.ny.gov

### MEMORANDUM

TO:

Robert Wither, Chief, South Permit Section FROM:

SUBJECT: Solar Panel Construction Stormwater Permitting/SWPPP Guidance

DATE: January 17, 2020

### Issue

The Department is seeing an increase in the number of solar panel construction projects across New York State. This has resulted in an increase in the number of questions on Construction General Permit (CGP) and Stormwater Pollution Prevention Plan (SWPPP) requirements from design professionals because the current CGP (GP-0-15-002) does not include a specific reference to the SWPPP requirements for solar panel projects in Tables 1 and 2 of Appendix B. To address this issue, the Division of Water (DOW) has developed the following guidance on CGP/SWPPP requirements for the different types of solar panel projects.

### Scenario 1

The DOW considers solar panel projects designed and constructed in accordance with the following criteria to be a "Land clearing and grading for the purposes of creating vegetated open space (i.e. recreational parks, lawns, meadows, fields)" type project as listed in Table 1, Appendix B of the CGP. Therefore, the SWPPP for this type of project will typically just need to address erosion and sediment controls.

- 1. Solar panels are constructed on post or rack systems and elevated off the ground surface,
- 2. The panels are spaced apart so that rain water can flow off the down gradient side of the panel and continue as sheet flow across the ground surface\*,
- 3. For solar panels constructed on slopes, the individual rows of solar panels are generally installed along the contour so rain water sheet flows down slope\*,
- 4. The ground surface below the panels consist of a well-established vegetative cover (see "Final Stabilization" definition in Appendix A of the CGP),
- 5. The project does not include the construction of any traditional impervious areas (i.e. buildings, substation pads, gravel access roads or parking areas, etc.),
- 6. Construction of the solar panels will not alter the hydrology from pre-to post development conditions (see Appendix A of the CGP, for definition of "Alter the hydrology..."). Note: The design professional shall perform the necessary site assessment/hydrology analysis to make this determination.



\*Refer to Maryland's "Stormwater Design Guidance- Solar Panel Installations" attached for guidance on panel installation.

\*\*See notes below for additional criteria.

### Scenario 2

If the design and construction of the solar panels meets all the criteria above, except for item 6, the project will fall under the "All other construction activities that include the construction or reconstruction of impervious area or alter the hydrology from pre-to post development conditions, and are not listed in Table 1" project type as listed in Table 2, Appendix B of the CGP. Therefore, the SWPPP for this type of project must address post-construction stormwater practices designed in accordance with the sizing criteria in Chapter 4 of the NYS Stormwater Management Design Manual, dated January 2015 (Note: Chapter 10 for projects in NYC EOH Watershed). The Water Quality Volume (WQv)/Runoff Reduction Volume (RRv) sizing criteria can be addressed by designing and constructing the solar panels in accordance with the criteria in items 1 – 4 above, however, the quantity control sizing criteria (Cpv, Qp and Qf) from Chapter 4 (or 10) of the Design Manual must still be addressed, unless one of the waiver criteria from Chapter 4 can be applied. \*\*See notes below for additional criteria.

### \*\* Notes

- Item 1: For solar panel projects where the panels are mounted directly to the ground (i.e. no space below panel to allow for infiltration of runoff), the SWPPP must address post-construction stormwater management controls designed in accordance with the sizing criteria in Chapter 4 of the NYS Stormwater Management Design Manual, dated January 2015 (Note: Chapter 10 for projects in NYC EOH Watershed).
- Item 5: For solar panel projects that include the construction of traditional impervious areas (i.e. buildings, substation pads, gravel access roads or parking areas, etc.), the SWPPP must address post-construction stormwater management controls for those areas of the project. This applies to both Scenario 1 and 2 above.

cc: Carol Lamb-Lafay, BWP Dave Gasper, BWP



### **Maryland Department of the Environment**

### **Stormwater Design Guidance – Solar Panel Installations**

Revisions to Maryland's stormwater management regulations in 2010 require that environmental site design (ESD) be used to the maximum extent practicable (MEP) to mimic natural hydrology, reduce runoff to reflect forested wooded conditions, and minimize the impact of land development on water resources. This applies to any residential, commercial, industrial, or institutional development where more than 5,000 square feet of land area is disturbed. Consequently, stormwater management must be addressed even when permeable features like solar panel installations exceed 5,000 square feet of land disturbance.

Depending on local soil conditions and proposed imperviousness, the amount of rainfall that stormwater requirements are based on varies from 1.0 to 2.6 inches. However, addressing stormwater management does not mean that structural or micro-scale practices must be constructed to capture and treat large volumes of runoff. Using nonstructural techniques like disconnecting impervious cover reduces runoff by promoting overland filtering and infiltration. Commonly used with smaller or narrower impervious areas like driveways or open roads, the Disconnection of Non-Rooftop Runoff technique (see pp. 5.61 to 5.65 of the **2000 Maryland Stormwater Design Manual**<sup>1</sup>) is a low cost alternative for treating runoff in situations like rows of solar panels.

When non-rooftop disconnection is used to treat runoff, the following factors should be considered:

- The vegetated area receiving runoff must be equal to or greater in length than the disconnected surface (e.g., width of the row of solar panels)
- Runoff must sheet flow onto and across vegetated areas to maintain the disconnection
- Disconnections should be located on gradual slopes (≤ 5%) to maintain sheetflow. Level spreaders, terraces, or berms may be used to maintain sheetflow conditions if the average slope is steeper than 5%. However, installations on slopes greater than 10% will require an engineered plan that ensures adequate treatment and the safe and non-erosive conveyance of runoff to the property line or downstream stormwater management practice.
- Disconnecting impervious surfaces works best in undisturbed soils. To minimize disturbance and compaction, construction vehicles and equipment should avoid areas used for disconnection during installation of the solar panels.
- Groundcover vegetation must be maintained in good condition in those areas receiving disconnected runoff. Typically this maintenance is no different than other lawn or landscaped areas. However, areas receiving runoff should be protected (e.g., planting shrubs or trees along the perimeter) from future compaction.

Depending on the layout and number of panels installed, the disconnection of non-rooftop runoff technique may address some or all of the stormwater management requirements for an individual project. Where the imperviousness is high or there is other infrastructure (e.g., access roads, transformers), additional runoff may need to be treated. In these situations, other ESD techniques or micro-scale practices may be needed to provide stormwater management for these features.

### Example 1 – Using Non-Rooftop Disconnection Where the Average Slope ≤ 5%

Several rows of solar panels will be installed in an existing meadow. The soils within the meadow are hydrologic soil group (HSG) B and the average slope does not exceed 5%. Each row of panels is 10 feet wide and the distance between rows is 20 feet. The rows of solar panels will be installed according to Figure 1 below. In this scenario, the disconnection length is the same as the distance between rows (20 feet) and is greater than the width of each row (10 feet). Therefore, each row of panels is adequately disconnected and the runoff from 1.0 inch of rainfall is treated.

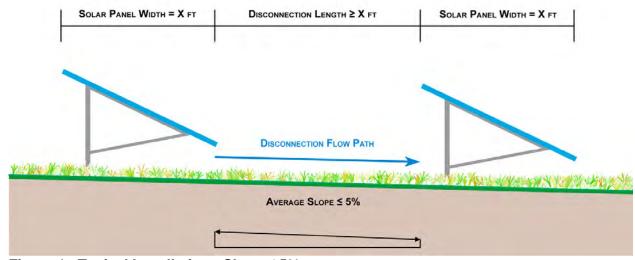


Figure 1. Typical Installation - Slope ≤ 5%

### Example 2 – Using Non-Rooftop Disconnection Where the Average Slope ≥ 5% but ≤ 10%

Several rows of solar panels will be installed in an existing meadow. The soils within the meadow are hydrologic soil group (HSG) B and the average slope is greater than 5% but less than 10%. Each row of panels is 10 feet wide and the distance between rows is 20 feet. The rows of solar panels will be installed as shown in Figure 2 below. The disconnection length is the same as the distance between rows (20 feet) and is greater than the width of each row (10 feet). However, in this example, a level spreader (typically 1 to 2-foot wide and 1 foot deep) has been located at the drip edge of each row of panels to dissipate energy and maintain sheetflow.

### **Discussion**

To meet State and local stormwater management requirements, ESD must be used to the MEP to reduce runoff to reflect forested conditions. While all reasonable options for implementing ESD must be investigated, minimally, the runoff from 1 inch of rainfall must be treated. In each of the examples above, there may be additional opportunities to implement ESD techniques or practices and reduce runoff that should be explored. However, simply disconnecting the runoff from the solar panel arrays captures and treats the runoff from 1.0 inch of rainfall. Where imperviousness is low and soil conditions less optimal (e.g., HSG C or D), this may be sufficient to completely address stormwater management requirements. In more dense applications or in sandy soils, additional stormwater management may be required.

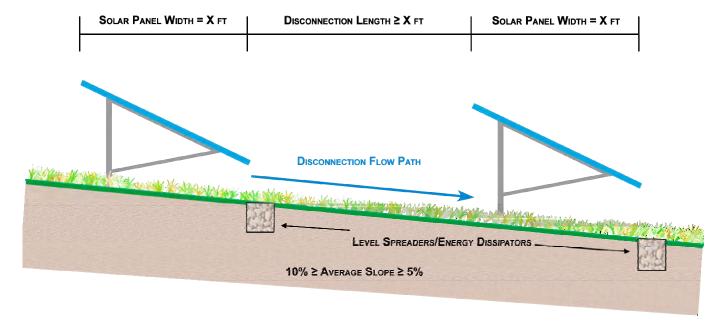


Figure 2. Typical Installation – Slope ≥ 5% but ≤ 10%

### Conclusion

The primary purpose of Maryland's stormwater management program is to mimic natural hydrologic runoff characteristics and minimize the impact of land development on water resources. Any land development project that exceeds 5,000 square feet of disturbance, including solar panel projects, must address stormwater management. However, for solar panels, stormwater management may be provided in a cost-effective manner by disconnecting each row of panels and directing runoff over the vegetated areas between the individual rows.

### Resources

<sup>1</sup> 2000 Maryland Stormwater Design Manual, Volumes I and II, MDE, October 2000 (http://www.mde.state.md.us/programs/Water/StormwaterManagementProgram/MarylandStormwaterDesignManual/Pages/Programs/WaterPrograms/SedimentandStormwater/stormwater\_design/index.aspx)